



Test Pit TP-4 - Photo 3



Test Pit TP-4 - Photo 4



Test Pit TP-4 - Photo 5



Test Pit TP-4 - Photo 6



Test Pit TP-5 - Photo 1



Test Pit TP-5 - Photo 2



Test Pit TP-5 - Photo 3



Test Pit TP-5 - Photo 4



Test Pit TP-5 - Photo 5



Test Pit TP-5 - Photo 6



Test Pit TP-5 - Photo 7



Test Pit TP-5 - Photo 8



Test Pit TP-6 - Photo 1



Test Pit TP-6 - Photo 2



Test Pit TP-6 - Photo 3



Test Pit TP-6 - Photo 4



Test Pit TP-6 - Photo 5



Test Pit TP-6 - Photo 6



Test Pit TP-7 - Photo 1



Test Pit TP-7 - Photo 2



Test Pit TP-7 - Photo 3



Test Pit TP-7 - Photo 4



Test Pit TP-7 - Photo 5



Test Pit TP-8 - Photo 1



Test Pit TP-8 - Photo 2



Test Pit TP-8 - Photo 3



Test Pit TP-8 - Photo 4



Test Pit TP-8 - Photo 5

APPENDIX C

Langan Test Pit Logs – East Campus Maintenance Area

LOG OF TEST PIT TP-301

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/4/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 263 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8.5 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Timothy Light

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+263.0	Dark brown silty fine SAND, trace fine gravel, trace clay, some roots, trace organics (moist)[TOPSOIL]	0			
	+262.0	Brown silty fine SAND, trace coarse-fine gravel, trace cobbles, trace roots (moist)	1			
			2	S-1	GRAB	S-1 from 1.5ft to 2.0ft. Infiltration test IT-301 performed at 2.0ft.
	+260.5	Gray coarse-fine SAND, some coarse-fine gravel, some silt (moist)[TILL]	3			
			4			
			5	S-2	GRAB	S-2 from 4.0ft to 5.5ft.
			6			
	+256.0	Gray coarse-fine SAND with coarse-fine gravel, some silt, some cobbles, trace boulders (moist)[WEATHERED ROCK]	7			Difficult excavation starting at about 7.0ft.
			8	S-3	GRAB	S-3 from 7.5ft to 8.5ft.
			9			
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-302

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/4/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 262 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 6 ft	WATER LEVEL - First N/E  WATER LEVEL - Completion N/A 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	LANGAN PERSONNEL Timothy Light

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+262.0	Dark brown silty fine SAND, some clay, trace fine gravel, some roots, trace organics (moist)[TOPSOIL]	0			
	+261.0	Brown silty coarse-fine SAND, some coarse-fine gravel, trace cobbles, trace clay, trace organics, trace roots (moist)	1			
	+259.3	Gray coarse-fine SAND with coarse-fine gravel, some silt, trace cobbles (moist)[TILL]	2	S-1	GRAB	S-1 from 2.0ft to 2.5ft. Infiltration test IT-302 performed at 2.5ft.
			3			
			4			
			5	S-2	GRAB	S-2 from 4.0ft to 5.5ft.
	+256.0	Bottom of test pit	6			Refusal on bedrock or boulders at 6.0ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			7			
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

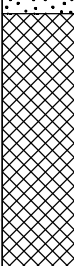
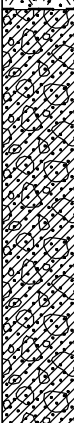
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LOG OF TEST PIT TP-303

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/4/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 260.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Timothy Light

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+260.5	Dark brown silty fine SAND, trace fine gravel, trace clay, trace roots, trace organics (moist)[TOPSOIL]	0			
	+259.5	Grayish brown coarse-fine SAND with coarse-fine gravel, some silt, trace roots (moist)[FILL]	1			
			2	S-1	GRAB	S-1 from 2.0ft to 3.0ft.
			3			
	+257.0	Brown silty fine SAND, some fine gravel, some roots (moist)[BURIED TOPSOIL]	4			
	+256.5	Gray coarse-fine SAND with coarse-fine gravel, some silt (moist)[TILL]	5			
			6			
			7	S-2	GRAB	S-2 from 6.5ft to 7.5ft.
			8			
	+252.5	Bottom of test pit	9			Maximum depth for equipment at 8.0ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-304

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/4/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 257 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Timothy Light

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+257.0	Light brown to brown silty coarse-fine SAND, some coarse-fine gravel, some concrete debris, some brick debris, trace glass debris, trace wire debris, trace fabric fragments, trace roots (moist)[FILL]	0			Fill at ground surface.
	+256.5		1	S-1	GRAB	S-1 from 0.5ft to 1.5ft.
	+255.5	Dark brown fine sandy SILT, some fine gravel, some clay, trace roots, trace concrete debris (moist)[TOPSOIL]	2			
		Brown to tan fine sandy SILT, trace coarse-fine gravel, trace cobbles (moist)	3	S-2	GRAB	S-2 from 2.5ft to 3.0ft. Infiltration test IT-304 performed at 3.0ft.
	+253.3	Gray coarse-fine SAND with coarse-fine gravel, some silt, some cobbles, trace boulders (moist)[TILL]	4			
			5	S-3	GRAB	S-3 from 4.5ft to 6.5ft.
			6			
	+249.5	Gray coarse-fine SAND with coarse-fine gravel, trace silt (moist)[WEATHERED ROCK]	7			
	+249.0		8	S-4	GRAB	S-4 from 7.7ft to 8.0ft.
		Bottom of test pit	9			Maximum depth for equipment at 8.0ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-305

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/5/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 263.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 5.5 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+263.5	Dark brown silty medium-fine SAND, trace coarse sand, trace fine gravel, trace roots	0			
	+262.8	(moist)[TOPSOIL]				
		Brown coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles (moist)	1			
			2	S-1	GRAB	S-1 from 1.75ft to 2.25ft. Grab sample of three 5-gallon buckets from 1.75ft to 2.25ft.
	+261.3	Gray coarse-fine SAND, some silt, some coarse-fine gravel, trace cobbles, trace boulders, trace weathered rock fragments (moist)[TILL]	3	S-2	GRAB	Infiltration test IT-305A performed at 2.25ft. S-2 from 2.5ft to 3.0ft. Infiltration test IT-305B performed at 3.0ft.
			4			Roots to 4.0ft.
			5			
	+258.0	Bottom of test pit	6			Refusal on bedrock or boulders at 5.5ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			7			
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-306

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/5/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 272 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 2 ft	WATER LEVEL - First N/E  WATER LEVEL - Completion N/A 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+272.0	Dark brown silty medium-fine SAND, trace fine gravel, some roots (moist)[TOPSOIL]	0			
	+271.2	Brown coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles, trace roots (moist)	1			
	+270.0	Bottom of test pit	2			Roots to 1.5ft.
			3			Refusal on bedrock at 2.0ft. Bedrock sloping from 0.16ft at the north side of the test pit to 2.0ft at the south south side. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			4			
			5			
			6			
			7			
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-307

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/5/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 256 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 4 ft	WATER LEVEL - First N/E  WATER LEVEL - Completion N/A 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+256.0	Dark brown silty medium-fine SAND, trace coarse sand, some roots	0			
	+255.3	(moist)[TOPSOIL]				
		Brown to light brown medium-fine SAND, trace coarse sand, trace coarse-fine gravel, trace cobbles, trace roots (moist)	1			
			2	S-1	GRAB	S-1 from 1.5ft to 2.0ft.
	+253.8	Gray coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles (moist)[TILL]	3			Roots to 2.5ft.
	+252.0	Bottom of test pit	4			Refusal on bedrock at 4.0ft. Bedrock at 4.0ft on west side (higher adjacent ground surface) and at 3.0ft on east side (lower adjacent ground surface). Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			5			
			6			
			7			
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

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LOG OF TEST PIT TP-308

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/5/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 262.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 7 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+262.5	Dark brown silty medium-fine SAND, trace coarse sand, trace roots (moist)[TOPSOIL]	0			
	+261.7	Brown to light brown silty medium-fine SAND, some coarse sand, trace coarse-fine gravel, trace roots (moist)	1			
			2			
	+259.5	Gray coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles, trace roots (moist)[TILL]	3			
			4			Roots to 4.0ft.
			5	S-1	GRAB	S-1 from 4.5ft to 5.0ft.
	+257.0	Gray coarse-fine SAND, some silt, some coarse-fine gravel, trace cobbles, some weathered rock fragments (moist)[TILL]	6			
			7	S-2	GRAB	S-2 from 6.5ft to 7.0ft.
	+255.5	Bottom of test pit	7			Test pit terminated at 7.0ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

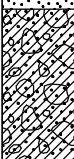
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LOG OF TEST PIT TP-309

Sheet 1 of 1

PROJECT NAME Belmont Hill School - East Campus Maintenance Area	PROJECT NUMBER 151014301	DATE 1/5/2022
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 264 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 5 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 303.5E Excavator	FOREMAN Steve Blackburn	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+264.0	Dark brown silty medium-fine SAND, trace coarse sand, trace roots (moist)[TOPSOIL]	0	S-1	GRAB	S-1 from 0.0ft to 0.5ft.
	+263.0	Brown to tan fine-coarse SAND, some silt, trace coarse-fine gravel, trace cobbles, trace boulders, trace roots (moist)	1			
			2	S-2	GRAB	S-2 from 2.0ft to 2.5ft.
			3			Roots to 3.0ft.
	+260.5	Gray coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles, trace weathered rock fragments (moist)[TILL]	4			
	+259.0	Bottom of test pit	5			Refusal on bedrock or boulder at 5.0ft. Test pit backfilled to grade with excavated material tamped with the excavator bucket.
			6			
			7			
			8			
			9			
			10			
			11			
			12			
			13			
			14			
			15			

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APPENDIX D

Langan Test Pit Photographs – East Campus Maintenance Area



Test Pit TP-301 - Photo 1



Test Pit TP-301 - Photo 2



Test Pit TP-301 - Photo 3



Test Pit TP-301 - Photo 4



Test Pit TP-301 - Photo 5



Test Pit TP-301 - Photo 6



Test Pit TP-301 - Photo 7



Test Pit TP-302 - Photo 1



Test Pit TP-302 - Photo 2



Test Pit TP-302 - Photo 3



Test Pit TP-302 - Photo 4



Test Pit TP-302 - Photo 5



Test Pit TP-302 - Photo 6



Test Pit TP-302 - Photo 7



Test Pit TP-303 - Photo 1



Test Pit TP-303 - Photo 2



Test Pit TP-303 - Photo 3



Test Pit TP-303 - Photo 4



Test Pit TP-303 - Photo 5



Test Pit TP-303 - Photo 6



Test Pit TP-304 - Photo 1



Test Pit TP-304 - Photo 2



Test Pit TP-304 - Photo 3



Test Pit TP-304 - Photo 4



Test Pit TP-304 - Photo 5



Test Pit TP-305 - Photo 1



Test Pit TP-305 - Photo 2



Test Pit TP-305 - Photo 3



Test Pit TP-305 - Photo 4



Test Pit TP-305 - Photo 5



Test Pit TP-305 - Photo 6



Test Pit TP-305 - Photo 7



Test Pit TP-305 - Photo 8



Test Pit TP-306 - Photo 1



Test Pit TP-306 - Photo 2



Test Pit TP-306 - Photo 3



Test Pit TP-306 - Photo 4



Test Pit TP-306 - Photo 5



Test Pit TP-306 - Photo 6



Test Pit TP-306 - Photo 7



Test Pit TP-306 - Photo 8



Test Pit TP-307 - Photo 1



Test Pit TP-307 - Photo 2



Test Pit TP-307 - Photo 3



Test Pit TP-307 - Photo 4



Test Pit TP-307 - Photo 5



Test Pit TP-307 - Photo 6



Test Pit TP-307 - Photo 7



Test Pit TP-307 - Photo 8



Test Pit TP-308 - Photo 1



Test Pit TP-308 - Photo 2



Test Pit TP-308 - Photo 3



Test Pit TP-308 - Photo 4



Test Pit TP-308 - Photo 5



Test Pit TP-308 - Photo 6



Test Pit TP-308 - Photo 7



Test Pit TP-308 - Photo 8



Test Pit TP-308 - Photo 9



Test Pit TP-308 - Photo 10



Test Pit TP-308 - Photo 11



Test Pit TP-309 - Photo 1



Test Pit TP-309 - Photo 2



Test Pit TP-309 - Photo 3



Test Pit TP-309 - Photo 4



Test Pit TP-309 - Photo 5



Test Pit TP-309 - Photo 6



Test Pit TP-309 - Photo 7



Test Pit TP-309 - Photo 8



Test Pit TP-309 - Photo 9



Test Pit TP-309 - Photo 10

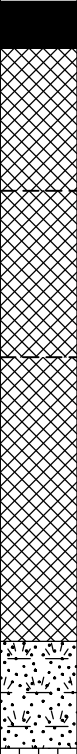
APPENDIX E

Langan Test Pit Logs – JAC Parking Lot

LOG OF TEST PIT TP-201

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/17/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 250.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8.5 ft	WATER LEVEL - First 8 ft ▽
EQUIPMENT CAT 306 Excavator	FOREMAN Scott Perachi	WATER LEVEL - Completion N/A ▼
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+250.5	Black ASPHALT	0			
	+250.2	(moist)[ASPHALT]				
		Grayish brown coarse to fine SAND, some coarse to fine gravel, trace silt, trace asphalt fragments, trace brick fragments (moist)[FILL]	1	S-1	GRAB	S-1 from 0.5ft to 1.0ft Dense graded base material
	+249.2	Light brown coarse to fine SAND, trace medium to fine gravel (moist)[FILL]	2	S-2	GRAB	S-2 from 1.5ft to 2.0ft
	+248.0	Brown coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders, trace brick fragments, trace asphalt fragments (moist)[FILL]	3	S-3	GRAB	S-3 from 3.0ft to 3.5ft
			4			
	+246.0	Dark brown SILT, some clay, some organics, trace medium to fine sand, trace roots, trace wood fragments (moist)[TOPSOIL]	5	S-4/6	GRAB	S-4 and S-6 from 4.5ft to 5.0ft S-5 from 4.75ft to 5.25ft
	+245.3	Light brownish SILT, trace clay, trace fine sand, trace roots (moist)	6	S-7	GRAB	Guelph Permeameter infiltration test at 5.25ft S-7 from 5.25ft to 6.25ft
			7			
		Brown SILT, trace clay, trace coarse to fine sand, trace fine gravel (moist)	8	S-8	GRAB	S-8 from 7.5ft to 8.0ft
			8			Groundwater at 8.0ft
	+242.0	Bottom of test pit	9			Bottom of test pit at 8.5ft. Test pit backfilled to 4.0ft with lifts of excavated material that was tamped with the excavator bucket. Test pit backfilled to 1.0ft with lifts of excavated material that was compacted with a plate compactor. Test pit backfilled to grade with dense graded material that was compacted with a plate compactor
			10			
			11			

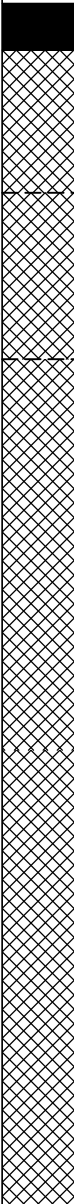
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LOG OF TEST PIT TP-202

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/16/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 249.8 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8.5 ft	WATER LEVEL - First N/E  WATER LEVEL - Completion N/A 
EQUIPMENT CAT 306 Excavator	FOREMAN Scott Perachi	LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+249.8	Black ASPHALT	0			
	+249.5	(moist)[ASPHALT]				
		Grayish brown coarse to fine SAND, some coarse to fine gravel, trace silt (moist)[FILL]	1			Dense graded base material
	+248.5	Brown silty coarse to fine SAND, some coarse to fine gravel, trace cobbles, trace boulders, trace asphalt fragments, trace brick fragments (moist)[FILL]	2			
	+247.3	Light brown to brown coarse to fine SAND, some coarse gravel, some silt, trace cobbles, trace boulders, trace brick fragments, trace wire fragments, trace concrete fragments, trace asphalt fragments, trace tile fragments (moist)[FILL]	3			
			4	S-1	GRAB	S-1 from 4.0ft to 4.5ft USDA textural classification performed Guelph Permeameter infiltration test at 4.5ft
			5			
		Light brown to dark brown coarse to fine SAND, some coarse to fine gravel, some silt, trace clay, trace cobbles, trace boulders, trace concrete fragments, trace glass fragments, trace asphalt fragments, trace brick fragments (moist)[FILL]	6			
			7	S-2	GRAB	S-2 from 7.0ft to 7.5ft
			8			
	+241.3	Bottom of test pit	9			Bottom of test pit at 8.5ft. Test pit backfilled to 4.0ft with lifts of excavated material that was tamped with the excavator bucket. Test pit backfilled to 1.0ft with lifts of excavated material that was compacted with a plate compactor. Test pit backfilled to grade with dense graded material that was compacted with a plate compactor
			10			
			11			

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LOG OF TEST PIT TP-203

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/16/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 250.4 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR FE French	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Scott Perachi	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+250.4	Black ASPHALT	0			
	+250.1	(moist)[ASPHALT]				
		Grayish brown coarse to fine SAND, some coarse to fine gravel, trace silt	1			Dense graded base material
		(moist)[FILL]				
			2			
	+248.4	Brown silty coarse to fine SAND, some coarse to fine gravel, trace cobbles, trace boulders, trace asphalt fragments, trace brick fragments, trace wood fragments, trace roots				
		(moist)[FILL]		S-1	GRAB	S-1 from 2.5ft to 3.0ft
			3			
			4	S-2	GRAB	S-2 from 4.0ft to 4.5ft. Roots to 4.0ft
			5			
	+245.4	Tan to white medium to fine SAND, some silt, trace fine gravel, some ash, trace coal fragments		S-3	GRAB	S-3 from 5.0ft to 5.25ft
	+245.1	(moist)[FILL]				
		Brown coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace brick fragments, trace ceramic fragments, trace asphalt fragments.	6			
		(moist)[FILL]				
	+243.9	Dark brown SILT, some clay, trace medium to fine sand				
		(moist)[TOPSOIL]	7			
	+243.4	Light brown SILT, trace coarse sand, trace fine gravel		S-4	GRAB	S-4 from 7.0ft to 7.5ft
		(moist)				
			8			
	+242.4	Bottom of test pit				Bottom of test pit at 8.0ft. Test pit backfilled to 4.0ft with lifts of excavated material that was tamped with the excavator bucket.
			9			Test pit backfilled to 1.0ft with lifts of excavated material that was compacted with a plate compactor. Test pit backfilled to grade with dense graded material that was compacted with a plate compactor
			10			
			11			

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LOG OF TEST PIT TP-204

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/16/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 253.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Scott Perachi	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+253.5	Dark brown silty medium to fine SAND, some roots (moist)[TOPSOIL]	0			
	+253.3	Light brown silty coarse to fine SAND, trace medium to fine gravel, trace asphalt fragments, trace plastic fragments, trace roots (moist)[FILL]	1			
	+251.5	Grayish brown coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders (moist)	2			
			3			
			4			
			5	S-1	GRAB	S-1 from 5.0ft to 5.5ft
			6			Guelph Permeameter infiltration test at 5.5ft
			7			Roots to 6.0ft
			8			Bottom of test pit at 8.0ft. Test pit backfilled to grade with lifts of excavated material that was tamped with the excavator bucket
			9			
			10			
			11			

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LOG OF TEST PIT TP-205

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/16/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 251.4 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 5.5 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Scott Perachi	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+251.4	Dark brown medium to fine SAND, some silt, trace coarse sand, some roots (moist)[TOPSOIL]	0			
	+251.2					
		Brown medium to fine SAND, some silt, trace medium to fine gravel, trace metal fragments, trace plastic pipe fragments (moist)[FILL]	1			
	+249.4	Gray coarse to fine SAND, some coarse to fine gravel, some silt, trace cobbles, trace boulders (moist)	2	S-1	GRAB	S-1 from 2.0ft to 2.5ft
						Guelph Permeameter infiltration test at 2.5ft
			3			
			4	S-2	GRAB	S-2 from 4.0ft to 4.5ft . Roots to 4.0ft
			5			
	+245.9	Bottom of test pit	6			Refusal on large boulder at 5.5ft. Test pit backfilled to grade with lifts of excavated material that was tamped with the excavator bucket.
			7			
			8			
			9			
			10			
			11			

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LOG OF TEST PIT TP-206

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/17/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 251.9 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Matt Ragone	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+251.9	Dark brown SILT, some coarse to fine sand, trace fine gravel, some roots (moist)[TOPSOIL]	0			
	+251.7	Brown silty coarse to fine SAND, some coarse to fine gravel, trace asphalt fragments, trace roots (moist)[FILL]	1			
	+250.4	Light brown coarse to fine sandy SILT, trace coarse to fine gravel, trace roots (moist)	2	S-1	GRAB	S-1 from 1.5ft to 2.0ft. USDA textural classification performed Guelph Permeameter infiltration test at 2.0ft
	+248.4	Gray coarse to fine SAND, some coarse to fine gravel, trace silt, some cobbles, trace boulders (moist)	4			Roots to 4.0ft
	+243.9	Bottom of test pit	8			Bottom of test pit at 8.0ft. Test pit backfilled to grade with lifts of excavated material that was tamped with the excavator bucket
			9			
			10			
			11			

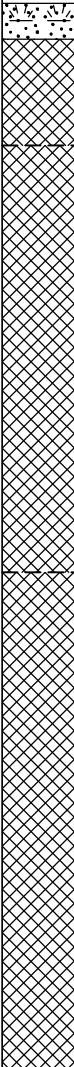
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LOG OF TEST PIT TP-207

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/17/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 256.5 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 7.5 ft	WATER LEVEL - First N/E  WATER LEVEL - Completion N/A 
EQUIPMENT CAT 306 Excavator	FOREMAN Matt Ragone	LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+256.5	Dark brown medium to fine sandy SILT, trace clay, some roots (moist)[TOPSOIL]	0			
	+256.3					
		Light brown to light gray SILT, some clay, trace medium to fine sand, trace roots (moist)[FILL]	1	S-1	GRAB	S-1 from 0.5ft to 1.0ft
	+255.5					
		Brown coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders, trace ceramic fragments, trace plastic fragments, trace roots (moist)[FILL]	2	S-2	GRAB	S-2 from 1.0ft to 1.5ft USDA textural classification performed
			3			Guelph Permeameter infiltration test at 1.5ft
			4			
	+252.5	Gray coarse to fine SAND, some coarse to fine gravel, trace silt, trace cobbles, trace boulders, trace wood fragments, trace ceramic fragments (moist)[FILL]	5			Roots to 5.0ft
			6			
			7			
	+249.0	Bottom of test pit	8			Bottom of test pit at 7.5ft. Test pit backfilled to grade with lifts of excavated material that was tamped with the excavator bucket
			9			
			10			
			11			

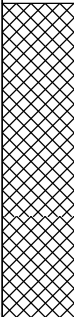
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LOG OF TEST PIT TP-208

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/17/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 259.4 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Matt Ragone	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+259.4	Brown coarse to fine SAND, some coarse to fine gravel, trace silt, trace cobbles, trace asphalt fragments, trace plastic fragments (moist)[FILL]	0			
			1			
		Light brown coarse to fine SAND, some silt, trace medium to fine gravel, trace brick fragments (moist)[FILL]	2			
	+257.2	Dark brown silty coarse to fine SAND		S-1	GRAB	S-1 from 2.25 to 2.5ft
	+256.9	(moist)[TOPSOIL]				
	+256.7	Black ASPHALT				
		(moist)[ASPHALT]	3	S-2	GRAB	S-2 from 3.0ft to 3.5ft
		Light brown coarse to fine SAND, some silt, some fine gravel (moist)				Guelph Permeameter infiltration test at 3.5ft
	+255.4	Gray coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders, trace roots (moist)	4	S-3	GRAB	S-3 from 4.0ft to 4.5ft
			5			
			6			Roots to 6.0ft
			7			
	+251.4	Bottom of test pit	8			Bottom of test pit at 8.0ft. Test pit backfilled to grade with lifts of excavated material that was tamped with the excavator bucket
			9			
			10			
			11			

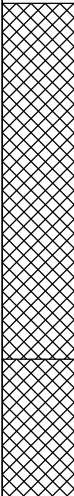
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LOG OF TEST PIT TP-209

Sheet 1 of 1

PROJECT NAME Belmont Hill School - Jordan Athletic Center	PROJECT NUMBER 151014301	DATE 10/17/2021
LOCATION 350 Prospect Street, Belmont, MA	ELEVATION Approx. 262 ft (Town of Belmont Datum)	
EXCAVATION CONTRACTOR F.E. French Construction, Inc.	DEPTH 8 ft	WATER LEVEL - First N/E 
EQUIPMENT CAT 306 Excavator	FOREMAN Matt Ragone	WATER LEVEL - Completion N/A 
		LANGAN PERSONNEL Alexander Macon

Symbol	ELEV (feet)	DESCRIPTION	Depth Scale	SAMPLE		REMARKS
				Number	Type	
	+262.0	Brown to dark brown WOOD CHIPS, trace silt (moist)[FILL]	0			
			1			
			2	S-1	GRAB	S-1 from 1.5ft to 2.0ft
	+259.5	Reddish brown medium to fine SAND, some silt, trace medium to fine gravel, trace wood fragments, trace glass fragments, trace brick fragments (moist)[FILL]	3	S-2	GRAB	S-2 from 2.75ft to 3.25ft
	+258.5	Dark brown SILT, trace medium to fine sand, trace roots (moist)[TOPSOIL]	4	S-3	GRAB	S-3 from 3.5ft to 4.0ft
	+258.0	Light brown sandy SILT, trace coarse to fine gravel, trace roots (moist)	5	S-4	GRAB	S-4 from 4.5ft to 5.0ft
			6			Guelph Permeameter infiltration test at 5.0ft
	+256.5	Gray silty coarse to fine SAND, some coarse to fine gravel, trace clay, trace cobbles, trace boulders (moist)	7	S-5	GRAB	S-5 from 6.5ft to 7.0ft
			8			Roots to 7.0ft
	+254.0	Bottom of test pit	9			
			10			
			11			

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APPENDIX F

Langan Test Pit Photographs – JAC Parking Lot



Test Pit TP-201 - Photo 1



Test Pit TP-201 - Photo 2



Test Pit TP-201 - Photo 3



Test Pit TP-201 - Photo 4



Test Pit TP-201 - Photo 5



Test Pit TP-201 - Photo 6



Test Pit TP-201 - Photo 7



Test Pit TP-202 - Photo 1



Test Pit TP-202 - Photo 2



Test Pit TP-202 - Photo 3



Test Pit TP-202 - Photo 4



Test Pit TP-202 - Photo 5



Test Pit TP-202 - Photo 6



Test Pit TP-202 - Photo 7



Test Pit TP-203 - Photo 1



Test Pit TP-203 - Photo 2



Test Pit TP-203 - Photo 3



Test Pit TP-203 - Photo 4



Test Pit TP-203 - Photo 5



Test Pit TP-203 - Photo 6



Test Pit TP-204 - Photo 1



Test Pit TP-204 - Photo 2



Test Pit TP-204 - Photo 3



Test Pit TP-204 - Photo 4



Test Pit TP-204 - Photo 5



Test Pit TP-205 - Photo 1



Test Pit TP-205 - Photo 2



Test Pit TP-205 - Photo 3



Test Pit TP-205 - Photo 4



Test Pit TP-205 - Photo 5



Test Pit TP-206 - Photo 1



Test Pit TP-206 - Photo 2



Test Pit TP-206 - Photo 3



Test Pit TP-206 - Photo 4



Test Pit TP-206 - Photo 5



Test Pit TP-206 - Photo 6



Test Pit TP-207 - Photo 1



Test Pit TP-207 - Photo 2



Test Pit TP-207 - Photo 3



Test Pit TP-207 - Photo 4



Test Pit TP-207 - Photo 5



Test Pit TP-207 - Photo 6



Test Pit TP-208 - Photo 1



Test Pit TP-208 - Photo 2



Test Pit TP-208 - Photo 3



Test Pit TP-208 - Photo 4



Test Pit TP-208 - Photo 5



Test Pit TP-208 - Photo 6



Test Pit TP-209 - Photo 1



Test Pit TP-209 - Photo 2



Test Pit TP-209 - Photo 3



Test Pit TP-209 - Photo 4



Test Pit TP-209 - Photo 5



Test Pit TP-209 - Photo 6



Test Pit TP-209 - Photo 7

APPENDIX G

Langan Boring Logs – East Campus Maintenance Area

Project Belmont Hill School - East Campus Maintenance Area				Project No. 151014301							
Location 350 Prospect Street, Belmont, MA				Elevation and Datum Approx. 255.5 ft (Town of Belmont Datum)							
Drilling Company Geologic Earth Exploration Inc.				Date Started 01/06/2022		Date Finished 01/06/2022					
Drilling Equipment Acker Scout				Completion Depth 10.5 ft		Rock Depth 7.5 ft					
Size and Type of Bit 3-7/8in Tricone Roller Bit				Number of Samples 4		Undisturbed N/A		Core 1			
Casing Diameter (in) 4			Casing Depth (ft) 7.5			Water Level (ft.) First ▽ N/E		Completion ▴ N/A		24 HR. ▴ N/A	
Casing Hammer Donut		Weight (lbs) 300		Drop (in) 30		Drilling Foreman Dave Sheldon					
Sampler 2-inch-diameter split spoon						Field Engineer Alexander Macon					
Sampler Hammer Donut		Weight (lbs) 140		Drop (in) 30							

MATERIAL SYMBOL	Elev. (ft)	Sample Description	Casing blws/ft Coring (min)	Depth Scale	Sample Data						Remarks (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.)			
					Number	Type	Recov. (in)	Penetr. resist. BLU6in	N-Value (Blows/ft)					
	255.5			0						10	20	30	40	
	255.0	Dark brown silty medium-fine SAND, trace coarse sand, trace roots (moist)[TOPSOIL]		1	S-1A	SS	10	2	11					S-1 at 0ft.
		Brown medium-fine SAND, some silt, some coarse sand, trace fine gravel, trace roots (moist)	Spin		S-1B	SS		21	11					
	253.5	Gray coarse-fine SAND, some silt, trace fine gravel (moist)[TILL]		2										
			90	3	S-2	SS	11	17	25					Attempted to sample and the sampler deflected laterally. Drill to 2.0ft. Moderate drilling. Some rig chatter from 1.0ft to 1.5ft. Inferred cobble from 1.0ft to 1.5ft. S-2 at 2ft.
		Gray coarse-fine SAND, some silt, trace fine gravel (moist)[TILL]		4					50					Switch to 3-inch-diameter casing. Switch to 2 3/4-inch roller bit. Spin casing to 3.0ft. Drive casing to 4.0ft. Drill to 4.0ft. Moderate drilling. Some rig chatter from 3.5ft to 4.0ft. Gray wash.
			115	5	S-3	SS	8	59	54					S-3 at 4ft.
		Gray coarse-fine SAND, some silt, some fine gravel (moist)[TILL]		6					50/2					Drill to 6.0ft. Moderate to hard drilling. Some rig chatter from 5.5ft to 6.0ft. Gray wash. Drive casing to 6.0ft. Clean out casing.
				7	S-4	SS	4	44	39					S-4 at 6ft.
	248.0	Brown GRANODIORITE [BOULDER]							50/1					Drill to 7.5ft. Moderate to hard drilling to 7.0ft. Very hard drilling from 7.0ft to 7.5ft. Gray wash. Spin casing to 7.5ft. Clean out casing.
	246.5	Inferred gray coarse-fine SAND, some silt, some fine gravel (moist)[TILL]	3:00	8										C-1 at 7.5ft Advanced through hard material at 9.0ft.
			1:36	9	C-1	NX Core	15							
	245.0	Bottom of boring	2:01	10										Boring terminated at 10.5ft. Boring backfilled to grade with soil cuttings and #2 sand.
				11										
				12										
				13										
				14										

Project Belmont Hill School - East Campus Maintenance Area				Project No. 151014301				
Location 350 Prospect Street, Belmont, MA				Elevation and Datum Approx. 261 ft (Town of Belmont Datum)				
Drilling Company Geologic Earth Exploration Inc.				Date Started 01/06/2022		Date Finished 01/06/2022		
Drilling Equipment Acker Scout				Completion Depth 16 ft		Rock Depth N/E		
Size and Type of Bit 3-7/8in Tricone Roller Bit				Number of Samples	Disturbed 6	Undisturbed N/A	Core 1	
Casing Diameter (in) 4			Casing Depth (ft) 13	Water Level (ft.)	First ▽ N/E	Completion ▽ N/A	24 HR. ▽ N/A	
Casing Hammer Donut		Weight (lbs) 300		Drop (in) 30	Drilling Foreman Dave Sheldon			
Sampler 2-inch-diameter split spoon								
Sampler Hammer Donut		Weight (lbs) 140		Drop (in) 30	Field Engineer Alexander Macon			

MATERIAL SYMBOL	Elev. (ft)	Sample Description	Casing blws/ft Coring (min)	Depth Scale	Sample Data						Remarks (Drilling Fluid, Depth of Casing, Fluid Loss, Drilling Resistance, etc.)
					Number	Type	Recov. (in)	Penetr. resist. (psi)	N-Value (Blows/ft)		
	261.0			0	S-1A	SS	4	8	10	20	S-1 at 0ft.
	260.6	Dark brown silty medium-fine SAND, trace wood chips, trace roots (moist)[TOPSOIL]		1	S-1B	SS	8	10	18		
		Dark brown silty medium-fine SAND, some coarse sand, trace fine gravel, trace roots (moist)[FILL]		2	S-2A	SS	7	7	16		S-2 at 2ft.
	258.8	Grayish brown coarse-fine SAND, some silt (moist)[FILL]		3	S-2B	SS	6	9	13		
		Dark brown silty medium-fine SAND, some coarse sand, trace fine gravel, trace wood chips (moist)[FILL]		4	S-3	SS	5	10	28		
		Gray coarse-fine SAND, some silt, trace fine gravel (moist)[FILL]		5	S-4A	SS	12	7	12		Spin casing to 4.0ft. Drill to 4.0ft. Easy drilling. Hard drilling from 3.5ft to 4.0ft. Some rig chatter from 3.5ft to 4.0ft. Dark brown wash. Wash color changed to grayish brown at 3.5ft. Inferred cobble from 3.5ft to 4.0ft. S-3 at 4ft. S-4 at 6.0ft. No recovery. Switch to 3-inch sampler with 300lb hammer. Sample from 4.0ft to 8.0ft. Blows: 4-4-6-6. Recovery: 12-inches
	255.0	Brown medium-fine SAND, trace silt (moist)	61	6	S-4B	SS	7	30	109		Drive casing to 8.0ft. Drill to 8.0ft. Moderate drilling. Light rig chatter. Gray to light brown wash. S-5 at 8ft.
	254.0	Gray coarse-fine SAND, some silt, trace fine gravel (moist)[TILL]		7	S-5	SS	7	35	10/5		Drive casing to 9.0ft. Drill to 10.0ft. Moderate drilling. Light rig chatter. Gray wash. S-6 at 10ft.
		Gray coarse-fine SAND, some silt, trace fine gravel (moist)[TILL]	42	8	S-6	SS	5	74			Drill to 13.0ft. Hard drilling. Some rig chatter. Very hard drilling from 11.5ft to 13.0ft. Inferred bedrock at 11.5ft Gray wash. Switch to 3-inch-diameter casing. Spin casing to 13.0ft. Switch to 2 3/4-inch roller bit. Clean out casing. C-1 at 13ft.
		Gray coarse-fine SAND, some silt, trace fine gravel (moist)[TILL]		9				50/3			
				10				10/5			
	249.5	Brown GRANODIORITE; medium to coarse grained; moderately weathered; close fracture spacing; fractures near horizontal; poor rock quality [BEDROCK]	3:14	13	C-1	NX Core	REC=97%	RQD=44%			
			3:19	14							
				15							

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APPENDIX H

Langan Boring Groundwater Observation Well Log – East Campus Maintenance Area

WELL CONSTRUCTION SUMMARY Well No. LB-02(OW)				
PROJECT Belmont Hill School		PROJECT NO. 151014301		
LOCATION 350 Prospect Street, Belmont, MA		ELEVATION AND DATUM Approx. 261 Town of Belmont Datum		
DRILLING AGENCY Geologic Earth Exploration, Inc		DATE STARTED 01/06/2022 DATE FINISHED 01/06/2022		
DRILLING EQUIPMENT CME Truck-Mounted Drill Rig		DRILLER Dave Sheldon		
SIZE AND TYPE OF BIT 3-7/8" Tri-Cone Roller Bit		INSPECTOR Alexander Macon		
METHOD OF INSTALLATION Boring LB-2(OW) was advanced to a depth of about 16ft with mud rotary drilling and rock coring techniques. The screen and riser for the well were placed into the borehole and set to the bottom of the hole. #2 sand was poured around the pipe to 1ft. above the screen. A 1-foot-thick seal of 3/8" Bentonite Chips was placed above the sand. The rest of the borehole was backfilled with #2 sand.				
METHOD OF WELL DEVELOPMENT N/A				
TYPE OF CASING PVC DIAMETER 2in.		TYPE OF BACKFILL MATERIAL #2 Sand		
TYPE OF SCREEN PVC DIAMETER 2in.		TYPE OF SEAL MATERIAL 3/8" Bentonite Chips		
BOREHOLE DIAMETER 4-1/4"		TYPE OF FILTER MATERIAL #2 sand		
TOP OF CASING ELEVATION DEPTH (ft) el. 261 0		WELL DETAILS 		SUMMARY SOIL CLASSIFICATION Topsoil Fill Sand Glacial Till Bedrock
TOP OF BACKFILL ELEVATION DEPTH (ft) el. 261 0				0.5
TOP OF SEAL ELEVATION DEPTH (ft) el. 257 4				6.0
TOP OF FILTER ELEVATION DEPTH (ft) el. 256 5				7.0
TOP OF SCREEN ELEVATION DEPTH (ft) el. 255 6				11.5
BOTTOM OF BORING ELEVATION DEPTH (ft) el. 245 16				16.0
SCREEN LENGTH 10ft.				
SLOT SIZE .1in.				
GROUNDWATER ELEVATIONS				
DATE	ELEVATION	DEPTH TO WATER (ft)	NOTE(s)	
NOTES:				
LANGAN MA, Inc.				

APPENDIX I
Laboratory Test Results

Client:	Langan Engineering	Project No:	GTX-314519
Project:	Belmont High School	Tested By:	ckg
Location:	Belmont, MA	Checked By:	bfs
Boring ID:	TP-5	Sample Type:	bag
Sample ID:	S-3	Test Date:	10/28/21
Depth :	4.5-5.0 ft	Test Id:	636906
Test Comment:	---		
Visual Description:	Moist, gray silty sand with gravel		
Sample Comment:	---		

USDA Textural Classification

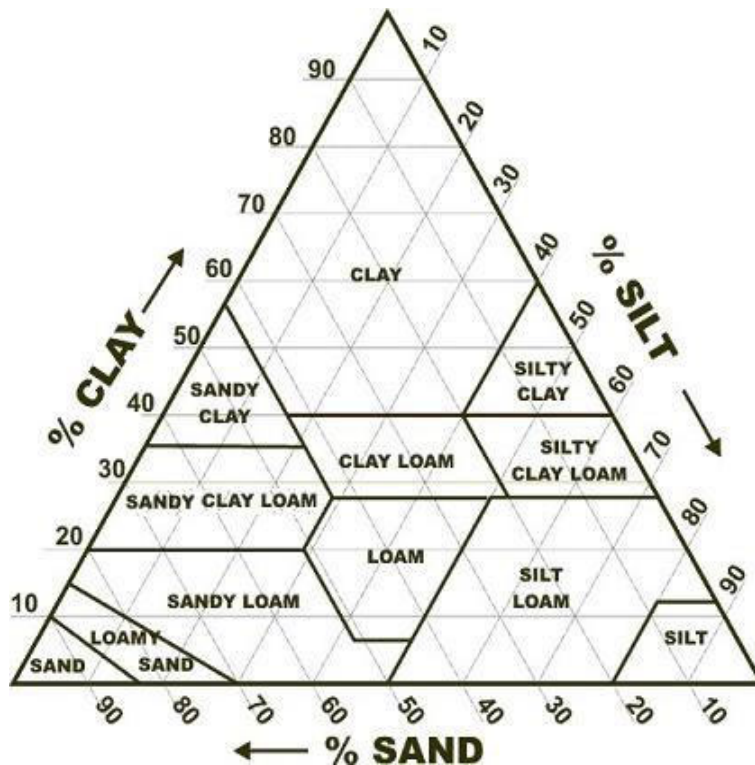
Boring ID	Sample ID	Depth	Sand, %	Silt, %	Clay, %	Classification
TP-5	S-3	4.5-5.0 ft	66	33	1	Sandy Loam

Classifications based only on material passing the #10 sieve

Sand: material passing 2.0 mm and retained on 0.05 mm diameter

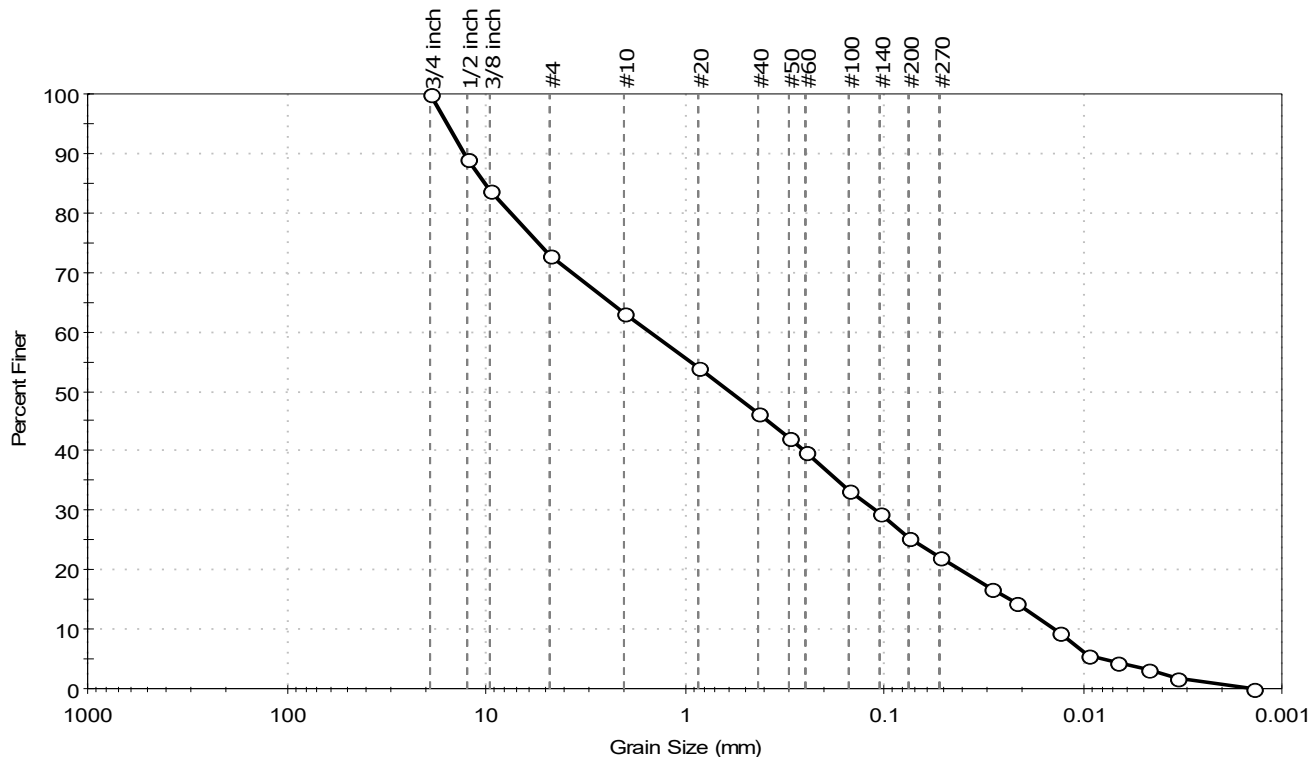
Silt: material passing 0.05 mm and retained on 0.002 mm diameter

Clay: material passing 0.002 mm diameter



Client: Langan Engineering	Project No: GTX-314519
Project: Belmont High School	
Location: Belmont, MA	
Boring ID: TP-5	Sample Type: bag
Sample ID: S-3	Tested By: ckg
Depth: 4.5-5.0 ft	Test Date: 10/28/21
	Checked By: bfs
	Test Id: 636902
Test Comment: ---	
Visual Description: Moist, gray silty sand with gravel	
Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	27.2	47.4	25.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
3/4 inch	19.00	100		
1/2 inch	12.50	89		
3/8 inch	9.50	84		
#4	4.75	73		
#10	2.00	63		
#20	0.85	54		
#40	0.42	46		
#50	0.30	42		
#60	0.25	40		
#100	0.15	33		
#140	0.11	30		
#200	0.075	25		
#270	0.053	22		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0290	17		
---	0.0216	14		
---	0.0131	9		
---	0.0093	6		
---	0.0067	4		
---	0.0048	3		
---	0.0034	2		
---	0.0014	0		

Coefficients

$D_{85} = 10.1511 \text{ mm}$ $D_{30} = 0.1104 \text{ mm}$
 $D_{60} = 1.4787 \text{ mm}$ $D_{15} = 0.0232 \text{ mm}$
 $D_{50} = 0.5900 \text{ mm}$ $D_{10} = 0.0140 \text{ mm}$
 $C_u = 105.621$ $C_c = 0.589$

Classification

ASTM N/A

AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #270 Sieve

Client:	Langan Engineering	Project No:	GTX-314519
Project:	Belmont High School		
Location:	Belmont, MA		
Boring ID:	TP-202	Sample Type:	bag
Sample ID:	S-1	Test Date:	10/28/21
Depth :	4.0-4.5 ft	Test Id:	636907
Test Comment:	---		
Visual Description:	Moist, dark brown silty sand with gravel		
Sample Comment:	Removed one unrepresentative 2 inch rock		

USDA Textural Classification

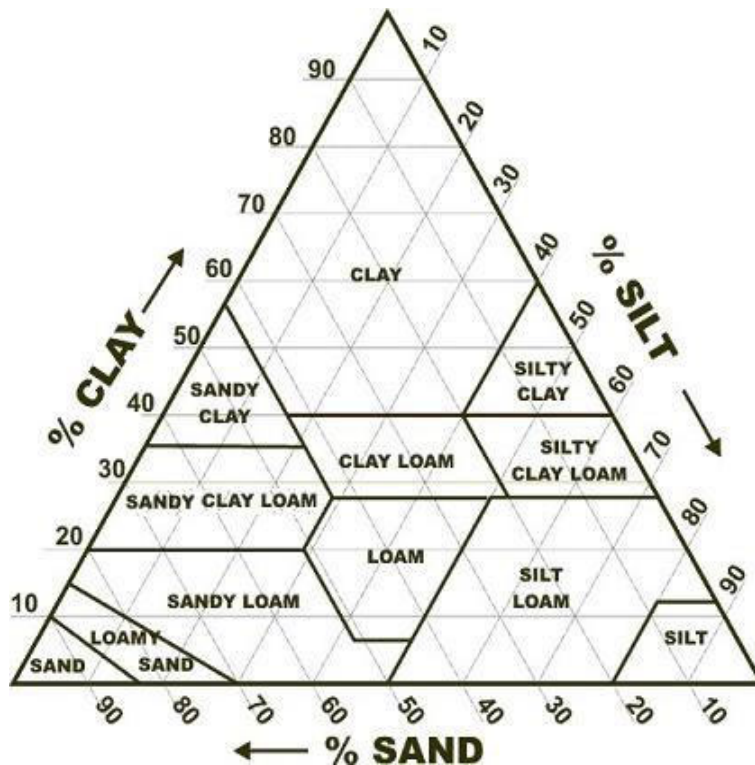
Boring ID	Sample ID	Depth	Sand, %	Silt, %	Clay, %	Classification
TP-202	S-1	4.0-4.5 ft	75	25	0	Loamy Sand

Classifications based only on material passing the #10 sieve

Sand: material passing 2.0 mm and retained on 0.05 mm diameter

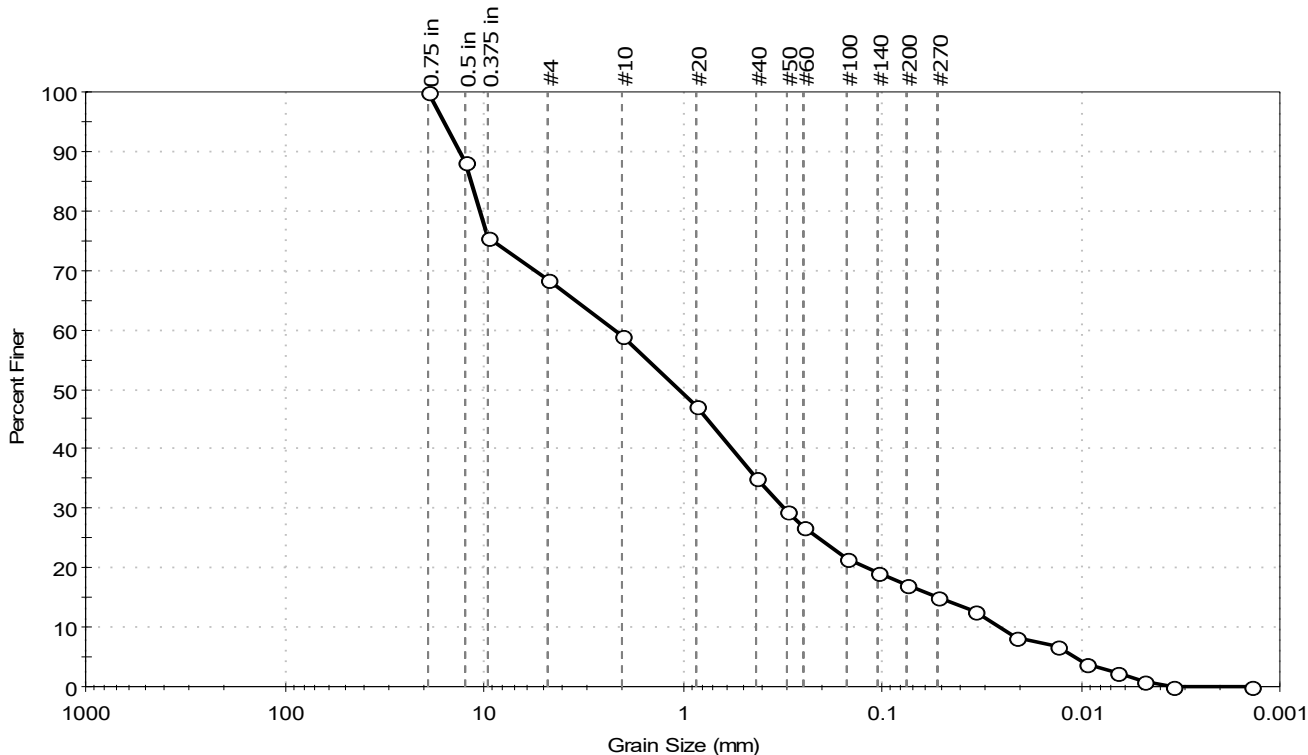
Silt: material passing 0.05 mm and retained on 0.002 mm diameter

Clay: material passing 0.002 mm diameter



Client: Langan Engineering	Project No: GTX-314519
Project: Belmont High School	
Location: Belmont, MA	
Boring ID: TP-202	Sample Type: bag
Sample ID: S-1	Tested By: ckg
Depth: 4.0-4.5 ft	Test Date: 10/28/21
	Checked By: bfs
	Test Id: 636903
Test Comment: ---	
Visual Description: Moist, dark brown silty sand with gravel	
Sample Comment: Removed one unrepresentative 2 inch rock	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	31.5	51.4	17.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	88		
0.375 in	9.50	76		
#4	4.75	68		
#10	2.00	59		
#20	0.85	47		
#40	0.42	35		
#50	0.30	30		
#60	0.25	27		
#100	0.15	22		
#140	0.11	19		
#200	0.075	17		
#270	0.053	15		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0343	13		
---	0.0211	8		
---	0.0131	7		
---	0.0094	4		
---	0.0066	2		
---	0.0048	1		
---	0.0034	0		
---	0.0014	0		

Coefficients

$D_{85} = 11.6728 \text{ mm}$ $D_{30} = 0.3078 \text{ mm}$
 $D_{60} = 2.1889 \text{ mm}$ $D_{15} = 0.0515 \text{ mm}$
 $D_{50} = 1.0430 \text{ mm}$ $D_{10} = 0.0257 \text{ mm}$
 $C_u = 85.171$ $C_c = 1.684$

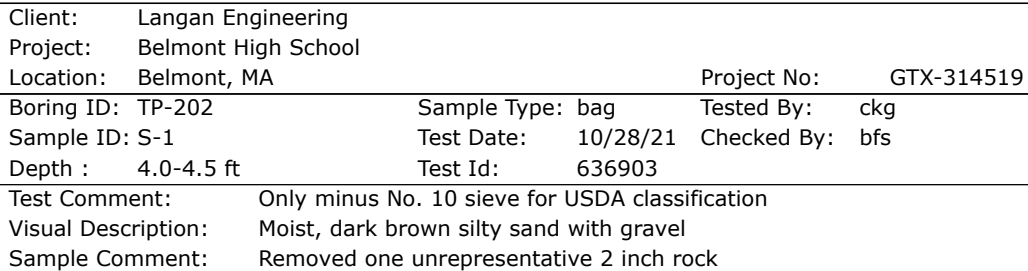
Classification

ASTM N/A

AASHTO Stone Fragments, Gravel and Sand (A-1-b (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #270 Sieve



Grain size distribution curve for a sand sample. The Y-axis represents Percent Finer (0 to 100), and the X-axis represents Grain Size (mm) on a logarithmic scale (1000 to 0.001). The curve shows that approximately 100% of the sample is finer than 4.75 mm, and about 0% is finer than 0.075 mm. Key sieve sizes are marked at the top: 0.75 in, 0.5 in, 0.375 in, #4, #10, #20, #40, #50, #60, #100, #140, #200, and #270.

Grain Size (mm)	Percent Finer (%)
4.75	100
2.0	80
0.85	60
0.6	50
0.425	46
0.3	37
0.25	33
0.2	29
0.15	26
0.106	21
0.075	14
0.05	11
0.0375	6
0.025	4
0.015	2
0.0075	0
0.00425	0

% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	71.1	28.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
# 10	2.00	100		
# 20	0.85	80		
# 40	0.42	60		
# 50	0.30	50		
# 60	0.25	46		
# 100	0.15	37		
# 140	0.11	32		
# 200	0.075	29		
# 270	0.053	26		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0343	21		
---	0.0211	14		
---	0.0131	11		
---	0.0094	6		
---	0.0066	4		
---	0.0048	1		
---	0.0034	0		
---	0.0014	0		

D ₈₅ = 1.0551 mm	D ₃₀ = 0.0834 mm
D ₆₀ = 0.4303 mm	D ₁₅ = 0.0231 mm
D ₅₀ = 0.2983 mm	D ₁₀ = 0.0121 mm
C _u = 35.562	C _c = 1.336

ASTM N/A

AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/ Test Description	Result
Sand/Gravel Particle Shape : ANGULAR	

Sand/Gravel Hardness : HARD

Dispersion Device : Apparatus A - Mech Mixer

Dispersion Period : 1 minute

Est. Specific Gravity : 2.65

Separation of Sample: #270 Sieve

Client:	Langan Engineering	Project No:	GTX-314519
Project:	Belmont High School		
Location:	Belmont, MA		
Boring ID:	TP-206	Sample Type:	bag
Sample ID:	S-1	Test Date:	10/28/21
Depth :	1.5-2.0 ft	Test Id:	636908
Test Comment:	---		
Visual Description:	Moist, dark yellowish brown sandy silt		
Sample Comment:	---		

USDA Textural Classification

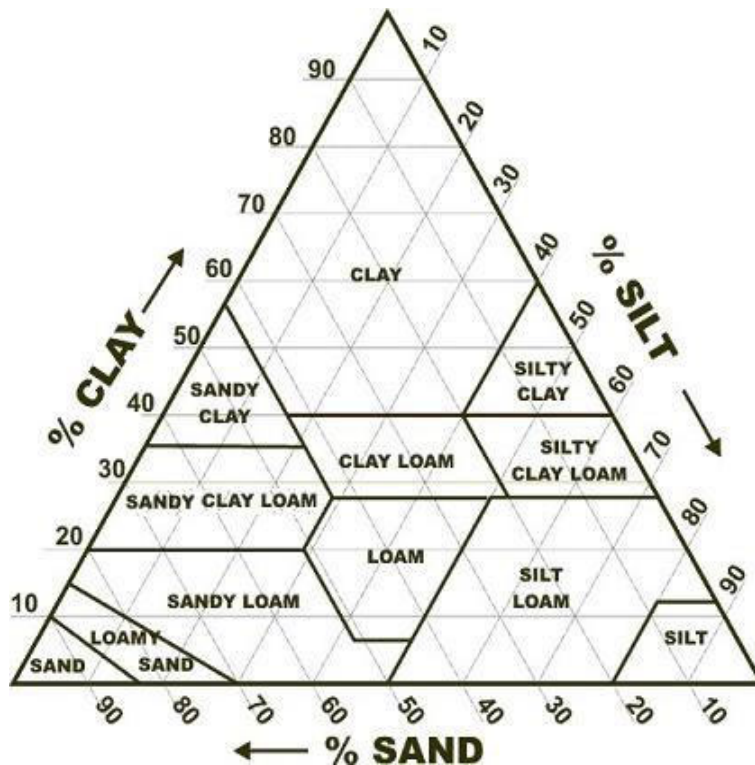
Boring ID	Sample ID	Depth	Sand, %	Silt, %	Clay, %	Classification
TP-206	S-1	1.5-2.0 ft	36	64	0	Silt Loam

Classifications based only on material passing the #10 sieve

Sand: material passing 2.0 mm and retained on 0.05 mm diameter

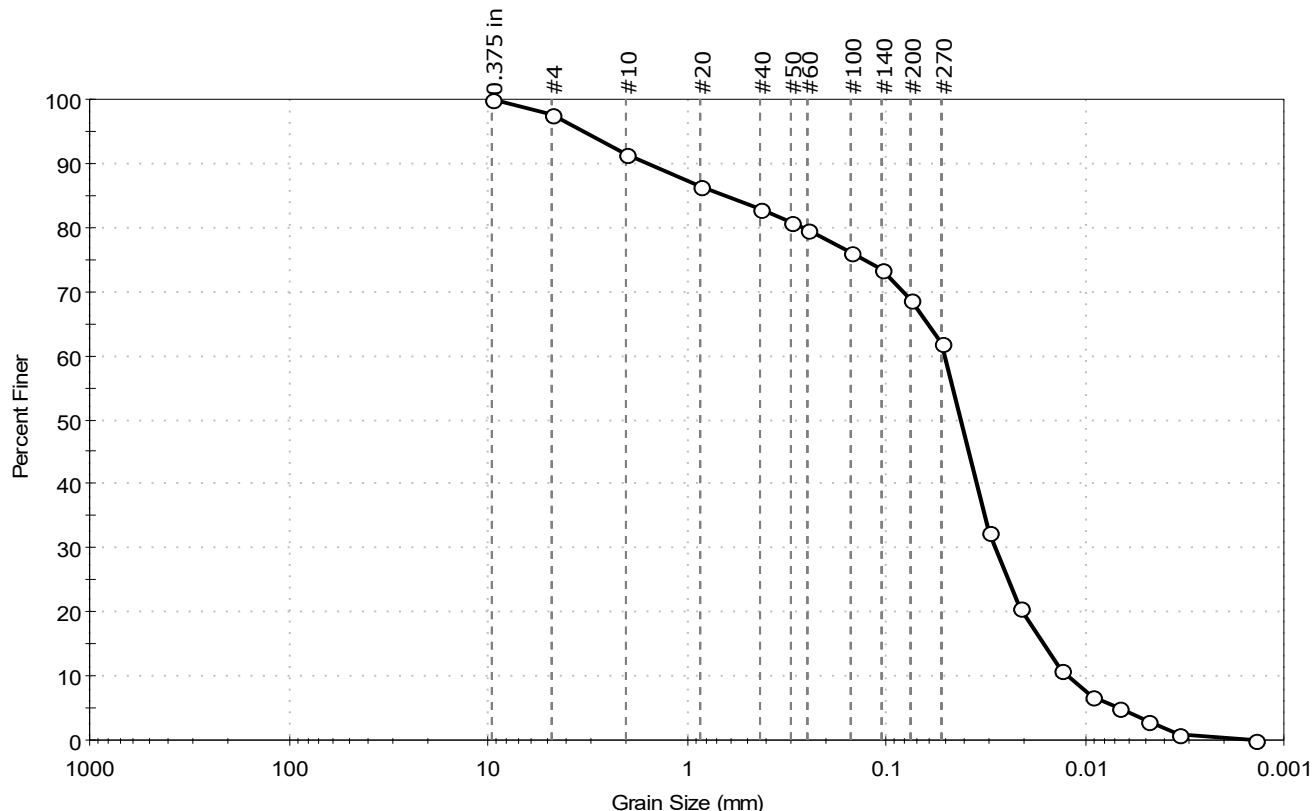
Silt: material passing 0.05 mm and retained on 0.002 mm diameter

Clay: material passing 0.002 mm diameter



Client: Langan Engineering	Project No: GTX-314519
Project: Belmont High School	
Location: Belmont, MA	
Boring ID: TP-206	Sample Type: bag
Sample ID: S-1	Tested By: ckg
Depth: 1.5-2.0 ft	Test Date: 10/28/21
	Checked By: bfs
Test Id: 636904	
Test Comment: ---	
Visual Description: Moist, dark yellowish brown sandy silt	
Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	2.5	28.8	68.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	98		
#10	2.00	91		
#20	0.85	87		
#40	0.42	83		
#50	0.30	81		
#60	0.25	80		
#100	0.15	76		
#140	0.11	73		
#200	0.075	69		
#270	0.053	62		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0306	32		
---	0.0214	21		
---	0.0132	11		
---	0.0092	7		
---	0.0067	5		
---	0.0048	3		
---	0.0034	1		
---	0.0014	0		

Coefficients

D₈₅ = 0.6288 mm D₃₀ = 0.0285 mm
 D₆₀ = 0.0511 mm D₁₅ = 0.0163 mm
 D₅₀ = 0.0425 mm D₁₀ = 0.0123 mm
 C_u = 4.154 C_c = 1.292

Classification

ASTM N/A

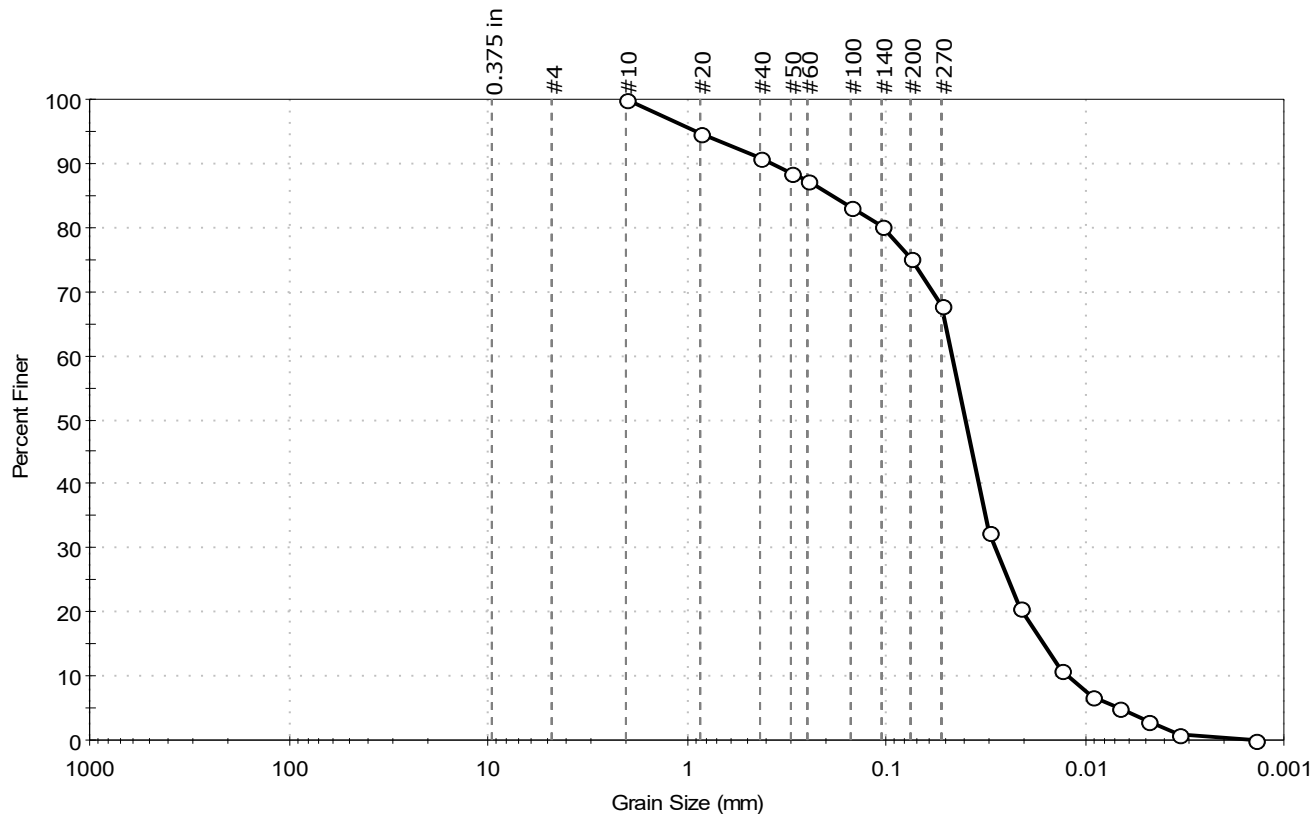
AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #270 Sieve

Client: Langan Engineering	Project No: GTX-314519
Project: Belmont High School	
Location: Belmont, MA	
Boring ID: TP-206	Sample Type: bag
Sample ID: S-1	Test Date: 10/28/21
Depth: 1.5-2.0 ft	Test Id: 636904
Test Comment: Only minus No. 10 sieve for USDA classification	Tested By: ckg
Visual Description: Moist, dark yellowish brown sandy silt	Checked By: bfs
Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	24.8	75.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#10	2.00	100		
#20	0.85	95		
#40	0.42	91		
#50	0.30	89		
#60	0.25	87		
#100	0.15	83		
#140	0.11	80		
#200	0.075	75		
#270	0.053	68		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0306	32		
---	0.0214	21		
---	0.0132	11		
---	0.0092	7		
---	0.0067	5		
---	0.0048	3		
---	0.0034	1		
---	0.0014	0		

Coefficients

$D_{85} = 0.1881$ mm $D_{30} = 0.0285$ mm
 $D_{60} = 0.0470$ mm $D_{15} = 0.0163$ mm
 $D_{50} = 0.0402$ mm $D_{10} = 0.0123$ mm
 $C_u = 3.821$ $C_c = 1.405$

Classification

ASTM N/A

AASHTO Silty Soils (A-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape : ANGULAR
 Sand/Gravel Hardness : HARD
 Dispersion Device : Apparatus A - Mech Mixer
 Dispersion Period : 1 minute
 Est. Specific Gravity : 2.65
 Separation of Sample: #270 Sieve

Client:	Langan Engineering	Project No:	GTX-314519
Project:	Belmont High School		
Location:	Belmont, MA		
Boring ID:	TP-207	Sample Type:	bag
Sample ID:	S-2	Test Date:	10/28/21
Depth :	1.0-1.5 ft	Test Id:	636909
Test Comment:	---		
Visual Description:	Moist, dark brown silty sand with gravel		
Sample Comment:	---		

USDA Textural Classification

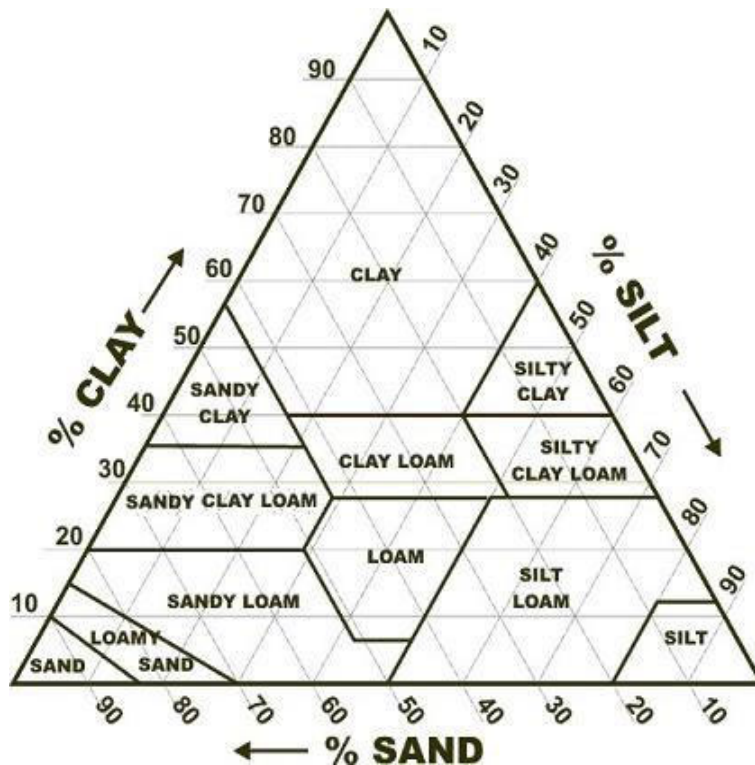
Boring ID	Sample ID	Depth	Sand, %	Silt, %	Clay, %	Classification
TP-207	S-2	1.0-1.5 ft	75	24	1	Silt Loam

Classifications based only on material passing the #10 sieve

Sand: material passing 2.0 mm and retained on 0.05 mm diameter

Silt: material passing 0.05 mm and retained on 0.002 mm diameter

Clay: material passing 0.002 mm diameter



Client: Langan Engineering
Project: Belmont High School
Location: Belmont, MA

Project No: GTX-314519

Boring ID: TP-207

Sample Type: bag

Tested By: ckg

Sample ID: S-2

Test Date: 10/28/21

Checked By: bfs

Depth: 1.0-1.5 ft

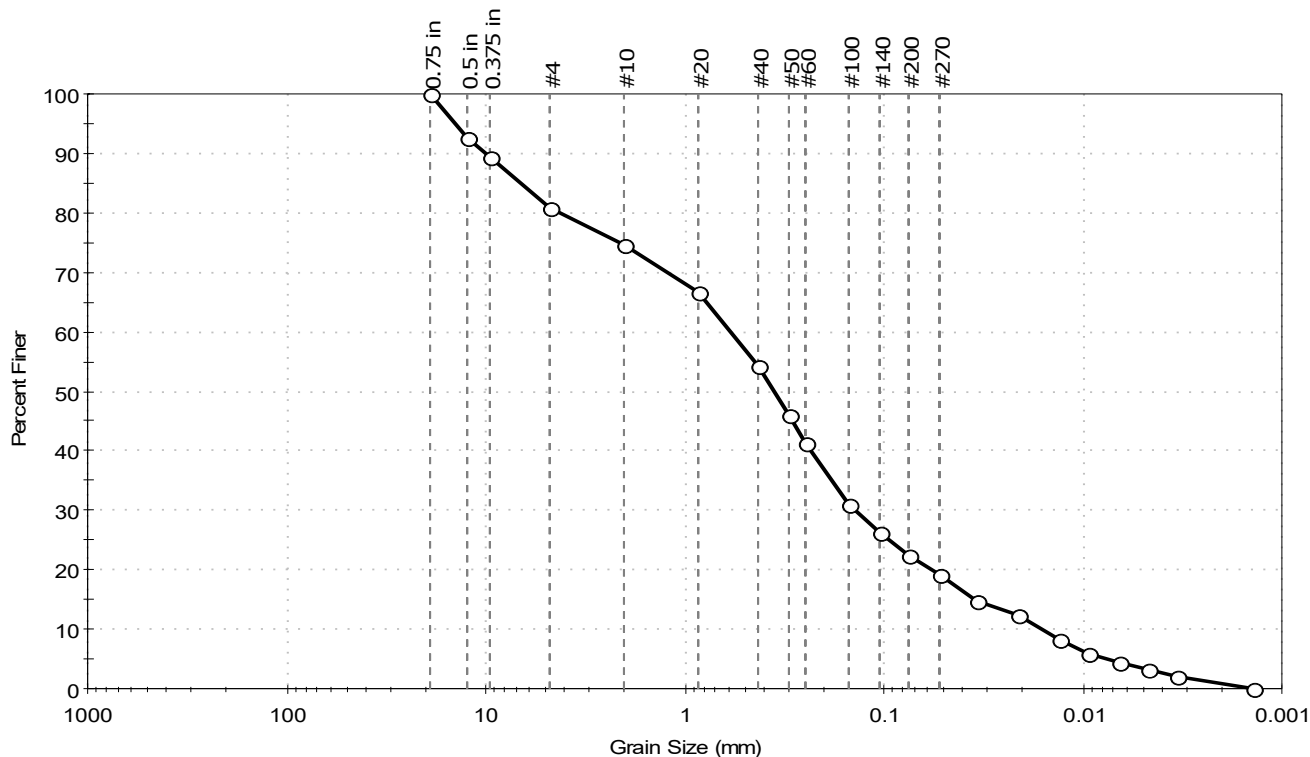
Test Id: 636905

Test Comment: ---

Visual Description: Moist, dark brown silty sand with gravel

Sample Comment: ---

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	19.1	58.6	22.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	93		
0.375 in	9.50	89		
#4	4.75	81		
#10	2.00	75		
#20	0.85	67		
#40	0.42	54		
#50	0.30	46		
#60	0.25	41		
#100	0.15	31		
#140	0.11	26		
#200	0.075	22		
#270	0.053	19		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0343	15		
---	0.0210	12		
---	0.0132	8		
---	0.0095	6		
---	0.0066	5		
---	0.0047	3		
---	0.0034	2		
---	0.0014	0		

Coefficients

$D_{85} = 6.6167 \text{ mm}$ $D_{30} = 0.1396 \text{ mm}$
 $D_{60} = 0.5893 \text{ mm}$ $D_{15} = 0.0349 \text{ mm}$
 $D_{50} = 0.3565 \text{ mm}$ $D_{10} = 0.0160 \text{ mm}$
 $C_u = 36.831$ $C_c = 2.067$

Classification

ASTM N/A

AASHTO Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description

Sand/Gravel Particle Shape: ---

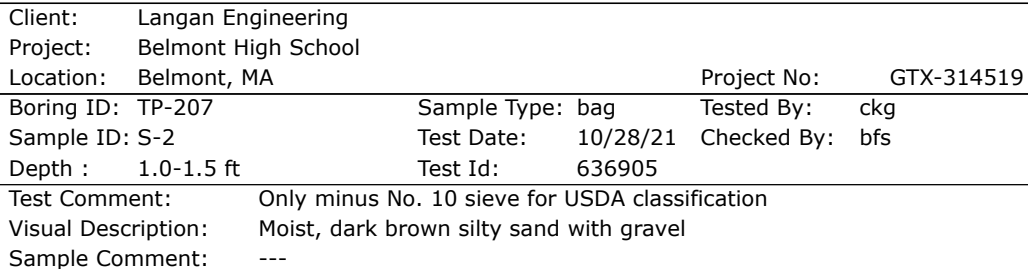
Sand/Gravel Hardness: ---

Dispersion Device: Apparatus A - Mech Mixer

Dispersion Period: 1 minute

Est. Specific Gravity: 2.65

Separation of Sample: #270 Sieve



Grain size distribution curve for a sample of sand. The graph plots Percent Finer (Y-axis, 0 to 100) against Grain Size (mm) on a logarithmic scale (X-axis, 1000 to 0.001). The curve shows that approximately 100% of the sand is finer than 4.75 mm, and about 0% is finer than 0.075 mm. Key sieve sizes are marked at the top: 0.75 in, 0.5 in, 0.375 in, #4, #10, #20, #40, #50, #60, #100, #140, #200, and #270.

Grain Size (mm)	Percent Finer (%)
4.75	100
2.0	90
0.85	73
0.425	62
0.25	56
0.15	42
0.075	35
0.0425	30
0.025	26
0.015	20
0.0075	16
0.00425	11
0.0025	8
0.0015	6
0.00075	4
0.000425	3
0.00025	2
0.00015	1
0.000075	0

% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	70.1	29.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
# 10	2.00	100		
# 20	0.85	89		
# 40	0.42	73		
# 50	0.30	62		
# 60	0.25	55		
# 100	0.15	42		
# 140	0.11	35		
# 200	0.075	30		
# 270	0.053	26		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0343	20		
---	0.0210	16		
---	0.0132	11		
---	0.0095	8		
---	0.0066	6		
---	0.0047	4		
---	0.0034	3		
---	0.0014	0		

D ₈₅ =0.7134 mm	D ₃₀ =0.0755 mm
D ₆₀ =0.2864 mm	D ₁₅ =0.0189 mm
D ₅₀ =0.2046 mm	D ₁₀ =0.0119 mm
C _u = 24.067	C _c = 1.673

ASTM N/A

AASHTO Silty Gravel and Sand (A-2-4 (0))

Sand/Gravel Particle Shape : ---

Sand/Gravel Hardness : ---

Dispersion Device : Apparatus A - Mech Mixer

Dispersion Period : 1 minute

Est. Specific Gravity : 2.65

Separation of Sample: #270 Sieve

APPENDIX J

Infiltration Testing Results – East Campus

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INFILTRATION TESTS						
IT-2 performed in TP-2						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE June 8th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 75-85 degrees			
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM				
1	3	Surface Elevation	Approx.	261.5	Town of Belmont Datum	
2	7	Top of Hole Elevation	Approx.	258.5	Town of Belmont Datum	
		Bottom of Hole Elevation	Approx.	258.0	Town of Belmont Datum	
METHOD OF INFILTRATION TEST						
TP-2 was excavated to a depth of about 36 inches below existing grades. An 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 3 cm and H2 = 7 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) was measured using the inner reservoir cylinder only. Test 2 (B) was measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	43.5	-	-	-	Gray coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace boulders (moist)
	1	51.6	8.1	3.2	191.5	
	2	55.3	3.7	1.5	87.5	
	3	59.6	4.3	1.7	101.7	
	4	62.5	2.9	1.1	68.6	
	5	65.3	2.8	1.1	66.2	
	6	69.6	4.3	1.7	101.7	
	7	72.1	2.5	1.0	59.1	
	8	74.8	2.7	1.1	63.8	
	Steady State Rate:				64.4	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: N/A
	0	5.5	-	-	-	
	1	5.9	0.4	0.2	9.5	
	2	6.5	0.6	0.2	14.2	
	3	7.1	0.6	0.2	14.2	
	4	7.8	0.7	0.3	16.5	
	5	8.4	0.6	0.2	14.2	
	6	9.0	0.6	0.2	14.2	
	7	9.5	0.5	0.2	11.8	
	8	10.1	0.6	0.2	14.2	
	9	10.7	0.6	0.2	14.2	
	10	11.3	0.6	0.2	14.2	
	Steady State Rate:				14.2	inches/hour

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INFILTRATION TESTS

IT-3 performed in TP-3

PROJECT Belmont Hill School		PROJECT NO. 151014301			
LOCATION Belmont, MA		DATE June 8th, 2021			
INSPECTOR Alex Macon		WEATHER Sunny, 75-85 degrees			
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM			
1	5	Surface Elevation	Approx.	261.0	Town of Belmont Datum
2	10	Top of Hole Elevation	Approx.	257.0	Town of Belmont Datum
		Bottom of Hole Elevation	Approx.	256.4	Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-3 was excavated to a depth of about 48 inches below existing grades. An 7-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	3.5	-	-	-	Gray coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace boulders (moist)
	1	5.1	1.6	0.6	37.8	
	2	6.4	1.3	0.5	30.7	
	3	7.6	1.2	0.5	28.4	
	4	8.8	1.2	0.5	28.4	
	5	9.9	1.1	0.4	26.0	
	6	11.0	1.1	0.4	26.0	
	7	12.1	1.1	0.4	26.0	
	8	13.2	1.1	0.4	26.0	
Steady State Rate:					26.0	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.80 in/hr
	0	4.0	-	-	-	
	1	10.6	6.6	2.6	156.0	
	2	12.4	1.8	0.7	42.6	
	3	14.0	1.6	0.6	37.8	
	4	15.5	1.5	0.6	35.5	
	5	17.0	1.5	0.6	35.5	
	6	18.6	1.6	0.6	37.8	
	7	20.2	1.6	0.6	37.8	
	8	21.8	1.6	0.6	37.8	
	9	22.4	0.6	0.2	14.2	
Steady State Rate:					37.8	inches/hour

LANGAN

INFILTRATION TESTS

IT-4 performed in TP-4

PROJECT Belmont Hill School		PROJECT NO. 151014301	
LOCATION Belmont, MA		DATE June 9th, 2021	
INSPECTOR Alex Macon		WEATHER Sunny, 75-85 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	5	Surface Elevation	Approx. 265.5 Town of Belmont Datum
2	15	Top of Hole Elevation	Approx. 264.0 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 263.3 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-4 was excavated to a depth of about 18 inches below existing grades. An 9-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 15 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	2.3	-	-	-	Brown fine sandy SILT, some medium sand, trace coarse to fine gravel, trace cobbles, trace boulders, trace roots (dry)
	1	2.9	0.6	0.2	14.2	
	2	3.6	0.7	0.3	16.5	
	3	4.5	0.9	0.4	21.3	
	4	5.2	0.7	0.3	16.5	
	5	6.0	0.8	0.3	18.9	
	6	6.7	0.7	0.3	16.5	
	7	7.4	0.7	0.3	16.5	
	8	8.1	0.7	0.3	16.5	
	9	8.8	0.7	0.3	16.5	
Steady State Rate:					16.5	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.61 in/hr
	0	9.8	-	-	-	
	1	12.0	2.2	0.9	52.0	
	2	13.0	1	0.4	23.6	
	3	14.7	1.7	0.7	40.2	
	4	16.5	1.8	0.7	42.6	
	5	18.3	1.8	0.7	42.6	
	6	19.9	1.6	0.6	37.8	
	7	21.4	1.5	0.6	35.5	
	8	22.9	1.5	0.6	35.5	
	9	24.4	1.5	0.6	35.5	
	10	25.9	1.5	0.6	35.5	
Steady State Rate:					35.5	inches/hour

LANGAN

INFILTRATION TESTS IT-5 performed in TP-5																																																																																																																																																																																																																																												
PROJECT Belmont Hill School			PROJECT NO. 151014301																																																																																																																																																																																																																																									
LOCATION Belmont, MA			DATE June 9th, 2021																																																																																																																																																																																																																																									
INSPECTOR Alex Macon			WEATHER Sunny, 75-85 degrees																																																																																																																																																																																																																																									
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METHOD OF INFILTRATION TEST TP-5 was excavated to a depth of about 66 inches below existing grades. An 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.																																																																																																																																																																																																																																												
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th style="width: 10%;"></th> <th style="width: 10%;">TIME (MIN)</th> <th style="width: 10%;">HEIGHT OF WATER (CM)</th> <th style="width: 10%;">DROP (CM)</th> <th style="width: 10%;">RATE (IN/MIN)</th> <th style="width: 10%;">RATE (IN/HOUR)</th> <th style="width: 40%;">SOIL CONDITIONS</th> </tr> </thead> <tbody> <tr> <td rowspan="26" style="text-align: center; vertical-align: middle;">TEST 1 (A)</td> <td>0</td><td>2.5</td><td>-</td><td>-</td><td>-</td><td rowspan="26" style="text-align: center; vertical-align: middle;">Gray coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace boulders (moist)</td> </tr> <tr><td>1</td><td>3.1</td><td>0.6</td><td>0.2</td><td>14.2</td></tr> <tr><td>2</td><td>3.7</td><td>0.6</td><td>0.2</td><td>14.2</td></tr> <tr><td>3</td><td>4.2</td><td>0.5</td><td>0.2</td><td>11.8</td></tr> <tr><td>4</td><td>4.6</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>5</td><td>5.0</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>6</td><td>5.5</td><td>0.5</td><td>0.2</td><td>11.8</td></tr> <tr><td>7</td><td>5.9</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>8</td><td>6.2</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>9</td><td>6.6</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>10</td><td>7.0</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>11</td><td>7.3</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>12</td><td>7.6</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>13</td><td>8.1</td><td>0.5</td><td>0.2</td><td>11.8</td></tr> <tr><td>14</td><td>8.4</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>15</td><td>8.7</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>16</td><td>9.1</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>17</td><td>9.5</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>18</td><td>9.8</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>19</td><td>10.2</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>20</td><td>10.4</td><td>0.2</td><td>0.1</td><td>4.7</td></tr> <tr><td>21</td><td>10.8</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>22</td><td>11.1</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>23</td><td>11.5</td><td>0.4</td><td>0.2</td><td>9.5</td></tr> <tr><td>24</td><td>11.8</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>25</td><td>12.1</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr><td>26</td><td>12.4</td><td>0.3</td><td>0.1</td><td>7.1</td></tr> <tr> <td colspan="5" style="text-align: right;">Steady State Rate:</td> <td style="text-align: center;">7.1</td> <td style="text-align: center;">inches/hour</td> </tr> <tr> <td rowspan="15" style="text-align: center; vertical-align: middle;">TEST 2 (B)</td> <td>0</td><td>5.8</td><td>-</td><td>-</td><td>-</td><td rowspan="15" style="text-align: center; vertical-align: middle;">Field Saturated Hydraulic Conductivity, Ksat: 0.63 in/hr</td> </tr> <tr><td>1</td><td>6.4</td><td>0.6</td><td>0.2</td><td>14.2</td></tr> <tr><td>2</td><td>6.6</td><td>0.2</td><td>0.1</td><td>4.7</td></tr> <tr><td>3</td><td>7.1</td><td>0.5</td><td>0.2</td><td>11.8</td></tr> <tr><td>4</td><td>8.0</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr><td>5</td><td>9.1</td><td>1.1</td><td>0.4</td><td>26.0</td></tr> <tr><td>6</td><td>9.9</td><td>0.8</td><td>0.3</td><td>18.9</td></tr> <tr><td>7</td><td>10.6</td><td>0.7</td><td>0.3</td><td>16.5</td></tr> <tr><td>8</td><td>11.5</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr><td>9</td><td>12.3</td><td>0.8</td><td>0.3</td><td>18.9</td></tr> <tr><td>10</td><td>13.2</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr><td>11</td><td>14.1</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr><td>12</td><td>15.0</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr><td>13</td><td>15.9</td><td>0.9</td><td>0.4</td><td>21.3</td></tr> <tr> <td colspan="5" style="text-align: right;">Steady State Rate:</td> <td style="text-align: center;">21.3</td> <td style="text-align: center;">inches/hour</td> </tr> </tbody> </table>								TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS	TEST 1 (A)	0	2.5	-	-	-	Gray coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace boulders (moist)	1	3.1	0.6	0.2	14.2	2	3.7	0.6	0.2	14.2	3	4.2	0.5	0.2	11.8	4	4.6	0.4	0.2	9.5	5	5.0	0.4	0.2	9.5	6	5.5	0.5	0.2	11.8	7	5.9	0.4	0.2	9.5	8	6.2	0.3	0.1	7.1	9	6.6	0.4	0.2	9.5	10	7.0	0.4	0.2	9.5	11	7.3	0.3	0.1	7.1	12	7.6	0.3	0.1	7.1	13	8.1	0.5	0.2	11.8	14	8.4	0.3	0.1	7.1	15	8.7	0.3	0.1	7.1	16	9.1	0.4	0.2	9.5	17	9.5	0.4	0.2	9.5	18	9.8	0.3	0.1	7.1	19	10.2	0.4	0.2	9.5	20	10.4	0.2	0.1	4.7	21	10.8	0.4	0.2	9.5	22	11.1	0.3	0.1	7.1	23	11.5	0.4	0.2	9.5	24	11.8	0.3	0.1	7.1	25	12.1	0.3	0.1	7.1	26	12.4	0.3	0.1	7.1	Steady State Rate:					7.1	inches/hour	TEST 2 (B)	0	5.8	-	-	-	Field Saturated Hydraulic Conductivity, Ksat: 0.63 in/hr	1	6.4	0.6	0.2	14.2	2	6.6	0.2	0.1	4.7	3	7.1	0.5	0.2	11.8	4	8.0	0.9	0.4	21.3	5	9.1	1.1	0.4	26.0	6	9.9	0.8	0.3	18.9	7	10.6	0.7	0.3	16.5	8	11.5	0.9	0.4	21.3	9	12.3	0.8	0.3	18.9	10	13.2	0.9	0.4	21.3	11	14.1	0.9	0.4	21.3	12	15.0	0.9	0.4	21.3	13	15.9	0.9	0.4	21.3	Steady State Rate:					21.3	inches/hour
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Steady State Rate:					7.1	inches/hour																																																																																																																																																																																																																																						
TEST 2 (B)	0	5.8	-	-	-	Field Saturated Hydraulic Conductivity, Ksat: 0.63 in/hr																																																																																																																																																																																																																																						
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	Steady State Rate:						21.3	inches/hour																																																																																																																																																																																																																																				

LANGAN

INFILTRATION TESTS						
IT-6 performed in TP-6						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE June 9th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 75-85 degrees			
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM				
1	5	Surface Elevation	Approx.	272.0	Town of Belmont Datum	
2	10	Top of Hole Elevation	Approx.	269.0	Town of Belmont Datum	
		Bottom of Hole Elevation	Approx.	268.5	Town of Belmont Datum	
METHOD OF INFILTRATION TEST TP-6 was excavated to a depth of about 36 inches below existing grades. An 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	4.9	-	-	-	Gray coarse to fine SAND, some silt, trace coarse to fine gravel, trace cobbles, trace boulders (moist)
	1	7.2	2.3	0.9	54.4	
	2	7.6	0.4	0.2	9.5	
	3	8.9	1.3	0.5	30.7	
	4	9.9	1	0.4	23.6	
	5	10.9	1	0.4	23.6	
	6	11.9	1	0.4	23.6	
	7	12.8	0.9	0.4	21.3	
	8	13.7	0.9	0.4	21.3	
	9	14.5	0.8	0.3	18.9	
	10	15.4	0.9	0.4	21.3	
	11	16.3	0.9	0.4	21.3	
	12	17.2	0.9	0.4	21.3	
	13	18.0	0.8	0.3	18.9	
	14	18.9	0.9	0.4	21.3	
	15	19.7	0.8	0.3	18.9	
	16	20.5	0.8	0.3	18.9	
	17	21.3	0.8	0.3	18.9	
	18	22.1	0.8	0.3	18.9	
Steady State Rate:					18.9	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 1.37 in/hr
	0	9.4	-	-	-	
	1	11.1	1.7	0.7	40.2	
	2	13.1	2	0.8	47.3	
	3	14.9	1.8	0.7	42.6	
	4	16.6	1.7	0.7	40.2	
	5	18.5	1.9	0.7	44.9	
	6	20.2	1.7	0.7	40.2	
	7	22.0	1.8	0.7	42.6	
	8	23.8	1.8	0.7	42.6	
	9	25.6	1.8	0.7	42.6	
	10	27.2	1.6	0.6	37.8	
	11	29.0	1.8	0.7	42.6	
	12	30.8	1.8	0.7	42.6	
	13	32.4	1.6	0.6	37.8	
	14	34.1	1.7	0.7	40.2	
	15	35.8	1.7	0.7	40.2	
	16	37.5	1.7	0.7	40.2	
	17	39.2	1.7	0.7	40.2	
Steady State Rate:					40.2	inches/hour

LANGAN

INFILTRATION TESTS

IT-7 performed in TP-7

PROJECT Belmont Hill School		PROJECT NO. 151014301	
LOCATION Belmont, MA		DATE June 9th, 2021	
INSPECTOR Alex Macon		WEATHER Sunny, 75-85 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	3	Surface Elevation	Approx. 272.5 Town of Belmont Datum
2	6	Top of Hole Elevation	Approx. 268.5 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 268.0 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-7 was excavated to a depth of about 48 inches below existing grades. An 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 3 cm and H2 = 6 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	1.0	-	-	-	Gray coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders (moist)
	1	1.2	0.2	0.1	4.7	
	2	1.5	0.3	0.1	7.1	
	3	1.9	0.4	0.2	9.5	
	4	2.1	0.2	0.1	4.7	
	5	2.5	0.4	0.2	9.5	
	6	2.7	0.2	0.1	4.7	
	7	2.9	0.2	0.1	4.7	
	8	3.1	0.2	0.1	4.7	
	9	3.4	0.3	0.1	7.1	
	10	3.6	0.2	0.1	4.7	
	11	3.9	0.3	0.1	7.1	
	12	4.2	0.3	0.1	7.1	
	13	4.4	0.2	0.1	4.7	
	14	4.6	0.2	0.1	4.7	
	15	4.9	0.3	0.1	7.1	
	Steady State Rate:				5.9	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.59 in/hr
	0	1.4	-	-	-	
	1	1.6	0.2	0.1	4.7	
	2	2.3	0.7	0.3	16.5	
	3	3.0	0.7	0.3	16.5	
	4	3.5	0.5	0.2	11.8	
	5	3.8	0.3	0.1	7.1	
	6	3.9	0.1	0.0	2.4	
	7	4.4	0.5	0.2	11.8	
	8	4.9	0.5	0.2	11.8	
	9	5.4	0.5	0.2	11.8	
	10	5.9	0.5	0.2	11.8	
	Steady State Rate:				11.8	inches/hour

LANGAN

INFILTRATION TESTS						
IT-8 performed in TP-8						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE June 9th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 75-85 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	4		Surface Elevation	Approx.	277.5	Town of Belmont Datum
2	8		Top of Hole Elevation	Approx.	273.5	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	273.0	Town of Belmont Datum
METHOD OF INFILTRATION TEST						
TP-8 was excavated to a depth of about 48 inches below existing grades. An 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 4 cm and H2 = 8 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	1.4	-	-	-	Gray coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders (moist)
	1	2.5	1.1	0.4	26.0	
	2	3.1	0.6	0.2	14.2	
	3	3.6	0.5	0.2	11.8	
	4	4.2	0.6	0.2	14.2	
	5	4.7	0.5	0.2	11.8	
	6	5.2	0.5	0.2	11.8	
	7	5.7	0.5	0.2	11.8	
	8	6.2	0.5	0.2	11.8	
	Steady State Rate:				11.8	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.39 in/hr
	0	2.5	-	-	-	
	1	2.6	0.1	0.0	2.4	
	2	2.8	0.2	0.1	4.7	
	3	3.5	0.7	0.3	16.5	
	4	4.4	0.9	0.4	21.3	
	5	5.1	0.7	0.3	16.5	
	6	5.8	0.7	0.3	16.5	
	7	6.4	0.6	0.2	14.2	
	8	7.1	0.7	0.3	16.5	
	9	7.8	0.7	0.3	16.5	
	10	8.5	0.7	0.3	16.5	
	11	9.2	0.7	0.3	16.5	
	Steady State Rate:				16.5	inches/hour

APPENDIX K

Infiltration Testing Results – East Campus Maintenance Area

LANGAN

INFILTRATION TESTS IT-301 performed in TP-301

PROJECT Belmont Hill School		PROJECT NO. 151021201	
LOCATION Belmont, MA		DATE January 4th, 2022	
INSPECTOR Tim Light		WEATHER Sunny, 25-30 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	5	Surface Elevation	Approx. 263.0 Town of Belmont Datum
2	10	Top of Hole Elevation	Approx. 261.5 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 261.0 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-302 was excavated to a depth of about 18 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 15 to 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Brown silty fine SAND, trace coarse-fine gravel, trace cobbles, trace roots (moist)
	1	4.9	-	-	-	
	2	5.9	1.0	0.4	23.6	
	3	6.9	1.0	0.4	23.6	
	4	7.9	1.0	0.4	23.6	
	5	8.9	1.0	0.4	23.6	
	6	9.6	0.7	0.3	16.5	
	7	10.3	0.7	0.3	16.5	
	8	11.0	0.7	0.3	16.5	
	9	12.0	1.0	0.4	23.6	
	10	12.8	0.8	0.3	18.9	
	11	13.6	0.8	0.3	18.9	
	12	14.4	0.8	0.3	18.9	
	13	15.2	0.8	0.3	18.9	
Steady State Rate:					18.9	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 3.53 in/hr
	0	-	-	-	-	
	0.25	6.0	-	-	-	
	0.5	9.1	3.1	4.9	293.1	
	0.75	12.1	3	4.7	283.7	
	1	15.1	3	4.7	283.7	
	1.25	18.1	3	4.7	283.7	
	1.5	21.1	3	4.7	283.7	
	1.75	24.1	3	4.7	283.7	
	2	27.1	3	4.7	283.7	
	2.25	30.1	3	4.7	283.7	
	3	36.1	6	3.2	189.1	
	4	43.2	7.1	2.8	167.8	
	5	50.5	7.3	2.9	172.6	
	6	57.0	6.5	2.6	153.7	
	7	63.5	6.5	2.6	153.7	
	8	70.0	6.5	2.6	153.7	
	9	76.5	6.5	2.6	153.7	
Steady State Rate:					153.7	inches/hour

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INFILTRATION TESTS IT-302 performed in TP-302

PROJECT Belmont Hill School		PROJECT NO. 151021201	
LOCATION Belmont, MA		DATE January 4th, 2022	
INSPECTOR Tim Light		WEATHER Sunny, 25-30 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	5	Surface Elevation	Approx. 262.0 Town of Belmont Datum
2	10	Top of Hole Elevation	Approx. 260.0 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 259.5 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-301 was excavated to a depth of about 24 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 30 to 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Brown silty coarse-fine SAND, some coarse-fine gravel, trace cobbles, trace clay, trace organics, trace roots (moist)
	1	1.2	-	-	-	
	2	2.0	0.8	0.3	18.9	
	3	2.8	0.8	0.3	18.9	
	4	3.5	0.7	0.3	16.5	
	5	4.1	0.6	0.2	14.2	
	6	4.8	0.7	0.3	16.5	
	7	5.5	0.7	0.3	16.5	
	8	6.2	0.7	0.3	16.5	
	9	6.9	0.7	0.3	16.5	
	10	7.6	0.7	0.3	16.5	
Steady State Rate:					16.5	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.80 in/hr
	0	-	-	-	-	
	0.5	5.1	-	-	-	
	1	5.5	0.4	0.3	18.9	
	1.5	5.8	0.3	0.2	14.2	
	2	6.0	0.2	0.2	9.5	
	3	6.6	0.6	0.2	14.2	
	4	8.3	1.7	0.7	40.2	
	5	9.6	1.3	0.5	30.7	
	6	10.9	1.3	0.5	30.7	
	7	12.1	1.2	0.5	28.4	
	8	13.2	1.1	0.4	26.0	
	9	14.3	1.1	0.4	26.0	
	10	15.4	1.1	0.4	26.0	
	11	16.5	1.1	0.4	26.0	
	12	17.6	1.1	0.4	26.0	
	13	18.7	1.1	0.4	26.0	
Steady State Rate:					26.0	inches/hour

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INFILTRATION TESTS IT-304 performed in TP-304

PROJECT Belmont Hill School		PROJECT NO. 151021201	
LOCATION Belmont, MA		DATE January 4th, 2022	
INSPECTOR Tim Light		WEATHER Sunny, 25-30 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	4.9	Surface Elevation	Approx. 257.0 Town of Belmont Datum
2	10	Top of Hole Elevation	Approx. 254.5 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 254.0 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-304 was excavated to a depth of about 30 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 4.9 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Brown to tan fine sandy SILT, trace coarse-fine gravel, trace cobbles (moist)
	1	2.8	-	-	-	
	2	3.3	0.5	0.2	11.8	
	3	3.8	0.5	0.2	11.8	
	4	4.3	0.5	0.2	11.8	
	5	4.8	0.5	0.2	11.8	
	6	5.3	0.5	0.2	11.8	
	7	5.8	0.5	0.2	11.8	
	8	6.3	0.5	0.2	11.8	
Steady State Rate:					11.8	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 1.28 in/hr
	0	-	-	-	-	
	1	3.7	-	-	-	
	2	4.8	1.1	0.4	26.0	
	3	6.0	1.2	0.5	28.4	
	4	7.0	1	0.4	23.6	
	5	8.0	1	0.4	23.6	
	6	9.0	1	0.4	23.6	
	7	10.0	1	0.4	23.6	
	8	11.0	1	0.4	23.6	
	9	12.0	1	0.4	23.6	
Steady State Rate:					23.6	inches/hour

LANGAN

INFILTRATION TESTS IT-305a performed in TP-305

PROJECT Belmont Hill School		PROJECT NO. 151021201	
LOCATION Belmont, MA		DATE January 5th, 2022	
INSPECTOR Alex Macon		WEATHER Sunny, 25-30 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	5	Surface Elevation	Approx. 263.5 Town of Belmont Datum
2	10	Top of Hole Elevation	Approx. 261.8 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 261.3 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-305 was excavated to a depth of about 21 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 30 to 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	0.0	-	-	-	Brown coarse-fine SAND, some silt, trace coarse-fine gravel, trace cobbles (moist)
	1	0.4	0.4	0.2	9.5	
	2	0.6	0.2	0.1	4.7	
	3	1.0	0.4	0.2	9.5	
	4	1.3	0.3	0.1	7.1	
	5	1.6	0.3	0.1	7.1	
	6	1.9	0.3	0.1	7.1	
	7	2.2	0.3	0.1	7.1	
	Steady State Rate:				7.1	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.74 in/hr
	0	3.5	-	-	-	
	0.5	7.7	4.2	3.3	198.6	
	1	9.0	1.3	1.0	61.5	
	1.5	9.8	0.8	0.6	37.8	
	2	10.5	0.7	0.6	33.1	
	2.5	11.1	0.6	0.5	28.4	
	3	11.7	0.6	0.5	28.4	
	3.5	12.3	0.6	0.5	28.4	
	4	12.9	0.6	0.5	28.4	
	4.5	13.5	0.6	0.5	28.4	
	Steady State Rate:				28.4	inches/hour

LANGAN

INFILTRATION TESTS IT-305b performed in TP-305

PROJECT Belmont Hill School		PROJECT NO. 151021201	
LOCATION Belmont, MA		DATE January 5th, 2022	
INSPECTOR Alex Macon		WEATHER Sunny, 25-30 degrees	
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM	
1	5	Surface Elevation	Approx. 263.5 Town of Belmont Datum
2	10	Top of Hole Elevation	Approx. 261.0 Town of Belmont Datum
		Bottom of Hole Elevation	Approx. 260.5 Town of Belmont Datum

METHOD OF INFILTRATION TEST

TP-305 was excavated to a depth of about 30 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 30 to 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.

	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	2.5	-	-	-	Gray coarse-fine SAND, some silt, some coarse-fine gravel, trace cobbles, trace boulders, trace weathered rock fragments (moist)
	0.5	2.7	0.2	0.2	9.5	
	1	3.2	0.5	0.4	23.6	
	1.5	3.5	0.3	0.2	14.2	
	2	3.9	0.4	0.3	18.9	
	2.5	4.2	0.3	0.2	14.2	
	3	4.6	0.4	0.3	18.9	
	3.5	5.0	0.4	0.3	18.9	
	4	5.3	0.3	0.2	14.2	
	4.5	5.7	0.4	0.3	18.9	
	5	6.1	0.4	0.3	18.9	
	5.5	6.3	0.2	0.2	9.5	
	6	6.5	0.2	0.2	9.5	
	6.5	6.9	0.4	0.3	18.9	
	7	7.2	0.3	0.2	14.2	
	7.5	7.5	0.3	0.2	14.2	
	8	7.8	0.3	0.2	14.2	
	8.5	8.1	0.3	0.2	14.2	
	Steady State Rate:				14.2	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 2.15 in/hr
	0	6.9	-	-	-	
	0.5	9.2	2.3	1.8	108.7	
	1.5	10.6	1.4	0.6	33.1	
	2	14.0	3.4	2.7	160.8	
	2.5	15.4	1.4	1.1	66.2	
	3	16.8	1.4	1.1	66.2	
	3.5	17.8	1	0.8	47.3	
	4	18.9	1.1	0.9	52.0	
	4.5	20.1	1.2	0.9	56.7	
	5	21.2	1.1	0.9	52.0	
	5.5	22.3	1.1	0.9	52.0	
	6	23.4	1.1	0.9	52.0	
	6.5	24.5	1.1	0.9	52.0	
	7	25.6	1.1	0.9	52.0	
	Steady State Rate:				52.0	inches/hour

APPENDIX L

Infiltration Testing Results – JAC Parking Lot

LANGAN

INFILTRATION TESTS						
IT-201 performed in TP-201						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon / Tim Light			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	5		Surface Elevation	Approx.	250.5	Town of Belmont Datum
2	10		Top of Hole Elevation	Approx.	245.8	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	245.3	Town of Belmont Datum
METHOD OF INFILTRATION TEST						
TP-201 was excavated to a depth of about 57 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 3 minutes, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner reservoir cylinder only. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Light brownish tan SILT, trace clay, trace fine sand, trace roots (moist)
	1	0.4	-	-	-	
	3	0.5	0.1	0.0	1.2	
	6	2.0	1.5	0.2	11.8	
	9	6.6	4.6	0.6	36.2	
	12	10.4	3.8	0.5	29.9	
	15	14.6	4.2	0.6	33.1	
	18	17.8	3.2	0.4	25.2	
	21	21.6	3.8	0.5	29.9	
	24	25.4	3.8	0.5	29.9	
	27	29.2	3.8	0.5	29.9	
Steady State Rate:					29.9	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.07 in/hr
	0	-	-	-	-	
	3	2.5	-	-	-	
	6	16.4	13.9	1.8	109.5	
	9	24.9	8.5	1.1	67.0	
	12	33.1	8.2	1.1	64.6	
	15	41.3	8.2	1.1	64.6	
	18	49.5	8.2	1.1	64.6	
Steady State Rate:					64.6	inches/hour

LANGAN

INFILTRATION TESTS						
IT-202 performed in TP-202						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon / Tim Light			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	8		Surface Elevation	Approx.	249.8	Town of Belmont Datum
2	14		Top of Hole Elevation	Approx.	245.8	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	245.3	Town of Belmont Datum
METHOD OF INFILTRATION TEST TP-202 was excavated to a depth of about 48 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 8 cm and H2 = 14 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using the inner reservoir cylinder only. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Light brown to brown coarse to fine SAND, some coarse gravel, some silt, trace cobbles, trace boulders, trace brick fragments, trace wire fragments, trace concrete fragments, trace asphalt fragments, trace tile fragments (moist) [FILL]
	0.5	27.0	-	-	-	
	1.5	28.5	1.5	0.6	35.5	
	2	29.5	1	0.8	47.3	
	3	30.8	1.3	0.5	30.7	
	4	31.9	1.1	0.4	26.0	
	5	33.2	1.3	0.5	30.7	
	6	34.5	1.3	0.5	30.7	
	7	36.2	1.7	0.7	40.2	
	8	38.2	2	0.8	47.3	
	9	39.8	1.6	0.6	37.8	
	10	41.4	1.6	0.6	37.8	
	11	43.0	1.6	0.6	37.8	
	12	44.6	1.6	0.6	37.8	
Steady State Rate:					37.8	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.17 in/hr
	0	-	-	-	-	
	1	4.2	-	-	-	
	2	6.6	2.4	0.9	56.7	
	3	9.8	3.2	1.3	75.6	
	4	12.8	3	1.2	70.9	
	5	15.8	3	1.2	70.9	
	6	18.8	3	1.2	70.9	
	7	21.8	3	1.2	70.9	
Steady State Rate:					70.9	inches/hour

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INFILTRATION TESTS						
IT-204 performed in TP-204						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon / Tim Light			WEATHER Sunny, 65-70 degrees			
TEST NUMBER		STATIC HEAD (CM)		ELEVATION AND DATUM		
1		5		Surface Elevation Approx. 253.5 Town of Belmont Datum		
2		10		Top of Hole Elevation Approx. 248.5 Town of Belmont Datum		
				Bottom of Hole Elevation Approx. 248.0 Town of Belmont Datum		
METHOD OF INFILTRATION TEST						
TP-204 was excavated to a depth of about 60 inches below existing grades. A 6-inch-deep and 11-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 15 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Grayish brown coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders (moist)
	0.5	14.8	-	-	-	
	1	20.0	5.2	4.1	245.9	
	1.5	24.9	4.9	3.9	231.7	
	2	30.6	5.7	4.5	269.5	
	2.5	36.0	5.4	4.3	255.3	
	3	41.3	5.3	4.2	250.6	
	3.5	46.9	5.6	4.4	264.8	
	4	52.9	6	4.7	283.7	
	4.25	55.5	2.6	4.1	245.9	
	4.5	58.1	2.6	4.1	245.9	
	4.75	60.8	2.7	4.3	255.3	
	5	63.8	3	4.7	283.7	
	5.25	66.8	3	4.7	283.7	
	5.5	69.8	3	4.7	283.7	
	5.75	72.8	3	4.7	283.7	
	6	75.8	3	4.7	283.7	
	Steady State Rate:					
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 14.46 in/hr
	0	-	-	-	-	
	0.25	13.5	-	-	-	
	0.5	17.7	4.2	6.6	397.2	
	0.75	21.6	3.9	6.1	368.8	
	1	24.6	3	4.7	283.7	
	1.25	28.1	3.5	5.5	331.0	
	1.5	31.5	3.4	5.4	321.5	
	1.75	35.0	3.5	5.5	331.0	
	2	38.5	3.5	5.5	331.0	
	2.25	42.0	3.5	5.5	331.0	
	2.5	45.5	3.5	5.5	331.0	
	2.75	49.0	3.5	5.5	331.0	
	3	52.5	3.5	5.5	331.0	
	3.25	56.0	3.5	5.5	331.0	
	3.5	59.5	3.5	5.5	331.0	
	3.75	63.0	3.5	5.5	331.0	
	4	66.5	3.5	5.5	331.0	
4.25	70.0	3.5	5.5	331.0		
4.5	73.5	3.5	5.5	331.0		
Steady State Rate:					331.0	inches/hour

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INFILTRATION TESTS						
IT-205 performed in TP-205						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM				
1	5	Surface Elevation	Approx.	251.4	Town of Belmont Datum	
2	10	Top of Hole Elevation	Approx.	249.4	Town of Belmont Datum	
		Bottom of Hole Elevation	Approx.	248.9	Town of Belmont Datum	
METHOD OF INFILTRATION TEST						
TP-205 was excavated to a depth of about 24 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 30 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	-	-	-	-	Gray coarse to fine SAND, some coarse to fine gravel, trace silt, trace cobbles, trace boulders (moist)
	0.5	3.4	-	-	-	
	1	3.9	0.5	0.4	23.6	
	1.5	4.3	0.4	0.3	18.9	
	2	4.7	0.4	0.3	18.9	
	2.5	5.1	0.4	0.3	18.9	
	3	5.5	0.4	0.3	18.9	
	3.5	5.9	0.4	0.3	18.9	
	Steady State Rate:				18.9	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 1.64 in/hr
	0	-	-	-	-	
	0.5	3.4	-	-	-	
	1	3.9	0.5	0.4	23.6	
	1.5	4.4	0.5	0.4	23.6	
	2	4.7	0.3	0.2	14.2	
	2.5	5.2	0.5	0.4	23.6	
	3	5.7	0.5	0.4	23.6	
	3.5	6.2	0.5	0.4	23.6	
	4	6.7	0.5	0.4	23.6	
	4.5	7.2	0.5	0.4	23.6	
	Steady State Rate:				23.6	inches/hour

LANGAN

INFILTRATION TESTS						
IT-206 performed in TP-206						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	5		Surface Elevation	Approx.	251.9	Town of Belmont Datum
2	10		Top of Hole Elevation	Approx.	250.4	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	249.9	Town of Belmont Datum
METHOD OF INFILTRATION TEST						
TP-206 was excavated to a depth of about 18 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 30 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	1.8	-	-	-	Light brown coarse to fine sandy SILT, trace coarse to fine gravel, trace roots (moist)
	0.5	1.9	-	-	-	
	1	2.4	0.5	0.4	23.6	
	1.5	2.7	0.3	0.2	14.2	
	2	2.9	0.2	0.2	9.5	
	2.5	3.2	0.3	0.2	14.2	
	3	3.5	0.3	0.2	14.2	
	3.5	3.7	0.2	0.2	9.5	
	4	3.9	0.2	0.2	9.5	
	4.5	4.2	0.3	0.2	14.2	
	5	4.5	0.3	0.2	14.2	
	5.5	4.8	0.3	0.2	14.2	
	6	5.1	0.3	0.2	14.2	
	Steady State Rate:					
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.41 in/hr
	0	9.7	-	-	-	
	0.5	9.9	-	-	-	
	1	10.3	0.4	0.3	18.9	
	1.5	10.9	0.6	0.5	28.4	
	2	11.4	0.5	0.4	23.6	
	2.5	11.8	0.4	0.3	18.9	
	3	12.2	0.4	0.3	18.9	
	3.5	12.7	0.5	0.4	23.6	
	4	13.2	0.5	0.4	23.6	
	4.5	13.5	0.3	0.2	14.2	
	5	13.9	0.4	0.3	18.9	
	5.5	14.3	0.4	0.3	18.9	
	6	14.7	0.4	0.3	18.9	
Steady State Rate:					18.9	inches/hour

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INFILTRATION TESTS						
IT-207 performed in TP-207						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)	ELEVATION AND DATUM				
1	5	Surface Elevation	Approx.	256.5	Town of Belmont Datum	
2	10	Top of Hole Elevation	Approx.	255.5	Town of Belmont Datum	
		Bottom of Hole Elevation	Approx.	255.0	Town of Belmont Datum	
METHOD OF INFILTRATION TEST						
TP-207 was excavated to a depth of about 12 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	1.9	-	-	-	Brown coarse to fine SAND, some silt, some coarse to fine gravel, trace cobbles, trace boulders, trace ceramic fragments, trace plastic fragments, trace roots (moist) [FILL]
	1	2.1	-	-	-	
	2	2.5	0.4	0.2	9.5	
	3	2.7	0.2	0.1	4.7	
	4	3.1	0.4	0.2	9.5	
	5	3.2	0.1	0.0	2.4	
	6	3.4	0.2	0.1	4.7	
	7	3.6	0.2	0.1	4.7	
	8	3.8	0.2	0.1	4.7	
	9	4.0	0.2	0.1	4.7	
	Steady State Rate:				4.7	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.18 in/hr
	0	3.6	-	-	-	
	1	3.8	-	-	-	
	2	4.3	0.5	0.2	11.8	
	3	4.6	0.3	0.1	7.1	
	4	5.0	0.4	0.2	9.5	
	5	5.3	0.3	0.1	7.1	
	6	5.6	0.3	0.1	7.1	
	7	5.9	0.3	0.1	7.1	
	8	6.2	0.3	0.1	7.1	
	Steady State Rate:				7.1	inches/hour

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INFILTRATION TESTS						
IT-208 performed in TP-208						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	3		Surface Elevation	Approx.	259.4	Town of Belmont Datum
2	6		Top of Hole Elevation	Approx.	256.4	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	255.9	Town of Belmont Datum
METHOD OF INFILTRATION TEST						
TP-208 was excavated to a depth of about 36 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 3 cm and H2 = 6 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	0.4	-	-	-	Light brown coarse to fine SAND, some silt, some fine gravel (moist)
	1	0.6	-	-	-	
	2	1.2	0.6	0.2	14.2	
	3	1.6	0.4	0.2	9.5	
	4	2.0	0.4	0.2	9.5	
	5	2.1	0.1	0.0	2.4	
	6	2.4	0.3	0.1	7.1	
	7	2.7	0.3	0.1	7.1	
	8	3.0	0.3	0.1	7.1	
	9	3.3	0.3	0.1	7.1	
	Steady State Rate:				7.1	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.94 in/hr
	0	1.3	-	-	-	
	1	2.4	-	-	-	
	2	3.1	0.7	0.3	16.5	
	3	4.0	0.9	0.4	21.3	
	4	4.2	0.2	0.1	4.7	
	5	4.5	0.3	0.1	7.1	
	6	4.9	0.4	0.2	9.5	
	7	5.4	0.5	0.2	11.8	
	8	6.0	0.6	0.2	14.2	
	9	6.5	0.5	0.2	11.8	
	10	7.0	0.5	0.2	11.8	
	11	7.5	0.5	0.2	11.8	
	Steady State Rate:				11.8	inches/hour

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INFILTRATION TESTS						
IT-209 performed in TP-209						
PROJECT Belmont Hill School			PROJECT NO. 151014301			
LOCATION Belmont, MA			DATE October 16th, 2021			
INSPECTOR Alex Macon			WEATHER Sunny, 65-70 degrees			
TEST NUMBER	STATIC HEAD (CM)		ELEVATION AND DATUM			
1	5		Surface Elevation	Approx.	262.0	Town of Belmont Datum
2	10		Top of Hole Elevation	Approx.	257.5	Town of Belmont Datum
			Bottom of Hole Elevation	Approx.	257.0	Town of Belmont Datum
METHOD OF INFILTRATION TEST						
TP-209 was excavated to a depth of about 54 inches below existing grades. A 6-inch-deep and 6-centimeter-diameter hole was then advanced using a hand auger. The hole was cleaned using a well prep brush and a sizing auger. The infiltration test was performed using a Guelph Permeameter with the two head method. H1 = 5 cm and H2 = 10 cm for these tests. After every 60 seconds, the height of the water was measured to calculate the total drop of water over the time period. Test 1 (A) and Test 2 (B) were measured using both the inner and outer reservoir cylinders. The table below summarizes the field data and the calculations for determining the rates in which the water infiltrated.						
	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	SOIL CONDITIONS
TEST 1 (A)	0	1.4	-	-	-	Light brown sandy SILT, trace coarse to fine gravel, trace roots (moist)
	1	1.8	-	-	-	
	2	2.1	0.3	0.1	7.1	
	3	2.6	0.5	0.2	11.8	
	4	2.8	0.2	0.1	4.7	
	5	3.2	0.4	0.2	9.5	
	6	3.5	0.3	0.1	7.1	
	7	3.7	0.2	0.1	4.7	
	8	4.0	0.3	0.1	7.1	
	9	4.2	0.2	0.1	4.7	
	10	4.4	0.2	0.1	4.7	
	11	4.7	0.3	0.1	7.1	
	12	5.0	0.3	0.1	7.1	
	13	5.3	0.3	0.1	7.1	
Steady State Rate:					7.1	inches/hour
TEST 2 (B)	TIME (MIN)	HEIGHT OF WATER (CM)	DROP (CM)	RATE (IN/MIN)	RATE (IN/HOUR)	Field Saturated Hydraulic Conductivity, Ksat: 0.38 in/hr
	0	2.9	-	-	-	
	1	3.1	-	-	-	
	2	3.8	0.7	0.3	16.5	
	3	4.4	0.6	0.2	14.2	
	4	5.1	0.7	0.3	16.5	
	5	5.6	0.5	0.2	11.8	
	6	6.1	0.5	0.2	11.8	
	7	6.6	0.5	0.2	11.8	
	8	7.1	0.5	0.2	11.8	
Steady State Rate:					11.8	inches/hour