ATTACHMENT I

TRAFFIC MONITORING AND MITIGATION AGREEMENT

This Traffic Monitoring and Mitigation Agreement ("Agreement") is entered into as of November 22, 1999 by and between the Town of Belmont, acting by and through its Board of Selectmen ("Belmont"), and The McLean Hospital Corporation ("McLean"). This Agreement establishes the maximum level of permitted traffic to be generated by uses within the Research & Development, Senior Living and McLean Institutional zoning subdistricts and to set forth the recourse actions to be taken by Belmont in the event that the actual traffic levels exceed such permitted levels. This Agreement also details the traffic mitigation measures for which McLean agrees to provide the funding.

Belmont acknowledges that McLean intends that one or more unrelated parties will actually develop the proposed uses within the Research & Development and Senior Living Subdistricts and that McLean will likely sell the land within such subdistricts to such parties in connection with the development. Performance of the obligations set forth in Section I with respect to any subdistrict shall be the responsibility of (and at the expense of) the owner of the land within such subdistrict (the joint and several liability of the owners of the land, if more than one party owns the land within a given subdistrict). Performance of the obligations set forth in Section II shall be the responsibility (and at the expense of) McLean, except that no building within any subdistrict shall be occupied if McLean has failed to perform any obligation under Section II which was required by the provisions of Section II to have been performed by such time.

MONITORING PROGRAM

A traffic monitoring program is to be conducted following completion and substantial occupancy of any building within any of the Research & Development; Senior Living; and McLean Institutional Subdistricts. The traffic monitoring program and recourse actions described herein will ensure that these components of the project generate:

- peak hour traffic flows at a rate that is less than or equal to a rate of 692 peak
 hour trips during morning peak hours (206 for the Research & Development
 Subdistrict; 36 for the Senior Living Subdistrict and 450 for the McLean
 Institutional Subdistrict) and a rate of 742 peak hour trips during evening peak
 hours (180 for the Research & Development Subdistrict; 92 for the Senior Living
 Subdistrict and 470 for the McLean Institutional Subdistrict); and
- 2. daily traffic flows at a rate that is less than or equal to a rate of 7,692 daily trips (1,784 for the Research & Development Subdistrict; 1,148 for the Senior Living Subdistrict and 4,760 for the McLean Institutional Subdistrict).

Notwithstanding any provision hereof to the contrary, this Agreement shall not be applicable to the McLean Institutional Subdistrict so long as the buildings and improvements within such subdistrict continue to be used exclusively for psychiatric hospital purposes and uses functionally dependent upon and necessary to psychiatric hospital use, except for either (i) up to 75,000 square feet of gross floor area of other uses permitted by zoning (other than medical offices) or (ii) up to 25,000 square feet of gross floor area of medical offices as permitted by zoning. McLean represents that as of the date hereof the entirety of the McLean Institutional Subdistrict is used for psychiatric hospital purposes and uses functionally dependent upon and necessary to psychiatric hospital use, Belmont acknowledging that such representation includes the Arlington School and the existing day care facility (but no expansion thereof) within such definition. McLean agrees to provide Belmont with an annual certification, on a building-by-building basis, of the number of square feet used for other purposes. McLean shall notify Belmont promptly upon any change in use causing the foregoing thresholds to be exceeded. Belmont shall have the right to obtain such further reasonable evidence from McLean as it shall require to confirm the accuracy of such certifications.

A. STUDY DATA

Data collected for the traffic monitoring program will include traffic volumes entering and exiting the Research & Development Subdistrict; Senior Living Subdistrict and McLean Institutional Subdistrict. Monitoring will involve continuous Automatic Traffic Recorder (ATR) counts on a daily basis. (Data will be collected in 15 minute increments.) Data collected shall be retained for at least one year from the date of collection.

A "weekly sampling report" shall mean a data collection report providing monitoring results over five consecutive, non-holiday weekdays, summarized by one hour intervals and by daily totals. The morning and evening peak hour volumes for each weekday will be determined and average morning and evening peak hour volumes will be determined for the week. In addition, the daily trip totals for each weekday will be determined and average daily trip totals will be determined for the week.

B. PROGRAM

- 1. Within six months after the issuance of a building permit for a structure within a subdistrict governed hereby, the owner of the land within the subdistrict (the "owner") shall file with the Town Engineer a detailed Traffic Demand Management (TDM) plan, describing the measures to be taken by the owner to avoid traffic generation in excess of the levels permitted hereby and describing the further measures to be taken by the owner in the event traffic generation exceeds permitted levels. The owner shall consult with the Town Engineer prior to filing the TDM plan and shall take into account any comments of the Town Engineer with respect thereto. The owner shall file an updated TDM plan annually thereafter.
- 2. Within thirty days of such structure reaching a 90% occupancy level, or one year after a certificate of occupancy has been issued, whichever is earlier, the owner shall notify the Town Engineer. The Town Engineer shall thereafter have the right (in the McLean Institutional Subdistrict, whenever this Agreement becomes applicable thereto) to require submission of a weekly sampling report for such subdistrict for any week designated by the Town Engineer. A weekly sampling report shall thereupon be submitted to the Town Engineer within seven days of such request (or seven days after the end of the week to be reported upon, if later). Notwithstanding the foregoing, Belmont agrees to observe the guideline that weekly sampling reports should generally not be required more than bi-monthly during development of a subdistrict and more than annually after one year following substantial completion of the build-out and occupancy within the subdistrict, reserving Belmont's right to require more frequent weekly sampling reports upon changes in use, changes in ownership, the occurrence of violations or other reasonable basis for more frequent reporting.

C. RECOURSE ACTIONS

- 1. There shall be deemed to be a violation of this Agreement whenever a weekly sampling report reveals that:
 - (a) either the morning or evening average peak hour trip generation rate exceeds the permitted rate; or
 - (b) the average daily trip total exceeds the permitted rate.
- 2. If a weekly sampling report contains a violation, then the owner shall: (a) prepare and submit to the Town Engineer an updated TDM plan (if one has not been filed within the previous three months); (b) use diligent efforts to implement such plan as soon as possible and (c) provide follow-up weekly sampling reports to the Town Engineer until no further violations exist. If a weekly sampling report (including a follow-up report) contains a violation, then the owner shall pay the Town of Belmont a traffic mitigation payment of \$10,000 (\$2,500 for a follow-up report) for each such weekly sampling report, which shall be applied by the Town against its costs in monitoring and enforcing this Agreement and/or in taking further action to mitigate the effect of traffic generated by the Property upon Town streets.

- 3. If follow-up weekly sampling reports continue to show violations for two weeks, then Belmont may restrict the number of parking spaces which can be used during the morning and evening peak hours to the extent that the Town Engineer determines is needed to correct the violations. If follow-up weekly sampling reports still continue to show violations thereafter, the Town Engineer may further increase such parking restrictions.
- 4. If for two consecutive months, follow-up weekly sampling reports evidence that average trip generation is below the permitted rates, the Town will return full control of parking to the owner. The owner shall continue to be obligated to file follow-up weekly sampling reports for one month after full control of parking has been returned.

II. MITIGATION PROGRAM

McLean agrees to provide the funding for the mitigation measures listed below. The measures proposed will mitigate project related traffic impacts at intersections where:

- 1. the project may have a material impact on traffic operations.
- state funding is not readily available to fund the improvements, or where pursuing or securing state funding will jeopardize, or compete with, the prospects of other eligible projects where funding is being sought. (State funding has been approved for the reconstruction of Pleasant Street, consequently, it is assumed suggested improvements for Pleasant Street can be incorporated into the ongoing Pleasant Street project. McLean shall fund increased design and construction costs associated with changes to the Pleasant Street design, if they are not able to be incorporated in the normal design process).
- there is sufficient public support to ensure timely implementation of proposed improvements.

Based on these criteria, McLean agrees to fund the design and construction of improvements at two intersections in the project vicinity at an estimated cost of \$690,000. McLean agrees it will collaborate with Belmont in developing the actual mitigation to be implemented. In addition, McLean will provide an additional \$310,000 to Belmont to be spent at Belmont's discretion for the design and construction of improvements at other intersections in the site vicinity not listed below.

Location	Proposed Action	Schedule	Estimated Construction Cost
A. Pleasant Street At McLean Driveway	Construct Drive Add Left Turn Lane Signalize	Prior to Occupancy of R&D Building or Senior Housing	\$375,000
B. Pleasant Street at Trapelo Road	Extend Right Turn Lane Extend R.O.W. Install Signal Interconnect Signal	Prior to Occupancy of R&D Building	\$315,000
C. At discretion of Town	Design and/or construct offsite intersection improvements		\$310,000
		TOTAL	\$1,000,000

The above-identified funding will be provided based on the following conditions:

- A. Upon certification by Belmont that Belmont has need for funding with respect to an improvement (identifying the dollar amount required therefor), McLean shall place such funds in escrow.
- B. Belmont and McLean shall maintain such funds in an interest bearing account with the full amount of interest earned payable to McLean.
- C. Belmont shall cooperate with McLean in seeking PWED and/or CDAG grants available by application filed prior to June 30, 2000 in order to fund any or all of the above improvements at locations A and B.
- D. If such grants are obtained to fund such improvements McLean's obligation to fund such improvements shall be reduced by the amount of such state grants and, upon actual receipt by Belmont of such state grants, McLean shall be entitled to withdraw such amount from the escrowed funds; provided, however, that if such grants exceed \$500,000 in the aggregate, then McLean's commitment to fund other improvements shall increase dollar-for-dollar up to a limit of \$190,000 (raising McLean's total commitment to other improvements to \$500,000).
- E. Belmont shall be authorized to draw funds from the escrow account in order to pay for the above-identified mitigation projects. Projects under Heading C must pertain to one or more of the following intersections: Mill Street at McLean Driveway; Mill Street at Trapelo Road; Trapelo Road at Waverley Oaks Road; Trapelo Road at Star Market Driveway; Concord Avenue at Winter Street; Concord Avenue at Mill Street; Pleasant Street at Clifton/Leonard Streets; Pleasant Street at Brighton Street; Concord Avenue at Blanchard Road; Concord Avenue at Pleasant Street; Concord Avenue at Common Street; Concord Avenue at Channing/Leonard Streets. Such projects can include transit, shuttle, pedestrian and/or bicycle enhancements associated with such intersections. Draws under Headings A and B shall not exceed \$690,000 (less any grant funds as described above) and McLean shall be responsible for any additional sums needed to complete such projects.
- F. Two years from the date of the certificate of occupancy representing 85% or more completion (on a square footage basis) of the permitted development within the Research & Development and Senior Living subdistricts, McLean shall be entitled to withdraw any remaining funds from the escrow account unless and for so long as Belmont is actively pursuing any of the above-identified mitigation projects and such remaining funds are necessary to pay for such project.

III. LEGAL EFFECT

The foregoing obligations shall run with the land now owned by McLean Hospital Corporation in Belmont, Massachusetts. McLean shall require any successor owner of land governed hereby to acknowledge in writing its obligations hereunder and to provide the same to Belmont prior to or upon transfer. A notice hereof shall, at the request of Belmont, be executed by McLean and recorded with the Registry of Deeds. This Agreement shall not take effect until ratified by a majority vote of Town Meeting of the Town of Belmont. Upon such ratifying vote, this Agreement shall not be amended in any material respect except by a further majority vote of Town Meeting.

Town of Belmont
By: Mary Marchan Sejectman
By: The State of the Selectman
By:
The McLean Hospital Corporation
ву:
Its Duly Authorized President

Transportation Impact Assessment

McLean Zone 3 Residential Development Belmont, Massachusetts

Prepared for:

Northland Residential Corporation Concord, Massachusetts

April 2021

Prepared by:



TRANSPORTATION IMPACT ASSESSMENT

MCLEAN HOSPITAL ZONE 3 RESIDENTIAL DEVELOPMENT BELMONT, MASSACHUSETTS

Prepared for:

Northland Residential Corporation Concord, Massachusetts

April 16, 2021

Prepared by:

VANASSE & ASSOCIATES, INC. 35 New England Business Center Drive Suite 140 Andover, MA 01810

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EXECUTIVE SUMMARY

Vanasse & Associates, Inc. (VAI) has prepared this Transportation Impact Assessment (TIA) to assess the potential traffic impacts associated with the proposed residential development located off Olmsted Drive in Belmont, Massachusetts (the "Project"). This study evaluates the following specific areas as they relate to the Project:

- Access requirements;
- Potential off-site improvements;
- Safety considerations;
- Identifies and analyzes existing and future traffic conditions, both with and without the Project;
- Traffic Signal Warrant Analysis (TSWA) for the intersection of Pleasant Street at Olmsted Drive; and,
- Monitoring and Compliance.

PROJECT DESCRIPTION

The project is located within Zone 3 of the McLean District and is projected to consist of a total of 150 residential units. The proposal entails construction of 40 for sale, age-restricted townhouse condominiums and 110 multi-family rental units comprised of 53 age-restricted units and 57 nonage restricted units. On-site parking will be provided for approximately 257 vehicles (165 parking spaces for the apartment buildings (1.5 spaces per unit), 80 spaces for the townhouse residents (2.0 spaces per unit), and 12 visitor spaces (0.3 spaces per unit). The proposed parking spaces comply with the *McLean Zone 3 Zoning Bylaw - Section 6B.3.1g and 6B.3.2.g* for allowed parking spaces per unit per subdistrict, as amended through the September 2020 Special Town Meeting Article. Access to the Project site is provided by Olmsted Drive. Olmsted Drive is an existing private, deadend roadway which extends from an unsignalized intersection with Pleasant Street and serves Zone 3 and 4 parcels of the Mclean District.

EXISTING CONDITIONS

A comprehensive field inventory of traffic conditions on the study area roadways was conducted in December 2020. The field investigations consisted of an inventory of existing roadway geometrics, traffic volumes, operating characteristics, posted speed limits and land use information within the study area. The study area for the Project was selected to focus on the major roadways providing access to the Project site.

Existing Traffic Volumes

In order to determine existing traffic-volume demands and flow patterns within the study area, manual turning movement counts (TMCs) and automatic traffic recorder counts (ATRs) were conducted on Tuesday, November 10, 2020. The TMC counts at Trapelo Road with Pleasant Street were conducted during the weekday morning (7:00 to 9:00 AM) and weekday evening (4:00 to 6:00 PM) peak periods. The TMC counts at the Olmsted Drive intersection with Pleasant Street were conducted from 7:00 AM to 7:00 PM. The ATR was placed on Pleasant Street, east of Olmsted Drive for a 48-hour count. These time periods were selected for analysis purposes as they are representative of the peak-traffic-volume hours for both the Project and the adjacent roadway network.

In order to account for COVID-19 travel restrictions, historic traffic count data conducted in April 2018¹ and November 2019² in the same study area were analyzed. It is important to note that the 2018 data were obtained from the earlier town wide traffic study conducted by the Town of Belmont. Using the historic 2018 and 2019 data of the study periods, the November 2020 weekday morning and evening peak-hour volumes were found to be approximately 40 percent lower. The traffic counts that form the basis of this assessment have been/are adjusted upward by 40 percent in order to provide an appropriate and conservative estimate of roadway operating conditions. It is important to note that in order to establish a 2021 Baseline condition, the November 2020 existing traffic volumes were grown by 1.0 percent per year.

Pleasant Street, east of Olmsted Drive was found to accommodate approximately 12,175 vehicles on an average weekday (24-hour, two-way volume), with approximately 949 vehicles per hour (vph) during the weekday morning peak hour and 1,148 vph during the weekday morning peak hour. The predominant flow on Pleasant Street during the weekday morning and evening peak hours is in the westbound direction.

A review of the peak-period traffic counts indicates that the weekday morning peak hour generally occurs between 7:30 and 8:30 AM with the weekday evening peak hour generally occurring between 4:30 and 5:30 PM.

FUTURE CONDITIONS

Traffic volumes within the study area were projected to 2028, a seven-year planning horizon consistent with State traffic study guidelines, by applying a 1 percent per year compounded annual background traffic growth rate to the 2021 Baseline condition peak-hour traffic volumes (discussion follows). The 2028-year traffic-volume projections incorporated identified specific development by others expected to be complete by 2028, as well as general background traffic

¹Town of Belmont – Town Wide Traffic Study by BSC group, April 2019.

²Transportation Impact Assessment, Proposed Marijuana Dispensary, Belmont MA, VAI, February 2020.

growth as a result of development external to the study area and presently unforeseen projects. Anticipated project-generated traffic added to these future conditions reflect 2028 Build conditions with the Project.

Site-Generated Traffic Volumes

The proposal entails construction of 40 for sale, age-restricted townhouse condominiums and 110 multi-family residences comprised of 53 age-restricted units and 57 non-age restricted units. In order to estimate the trip-generation characteristics of the proposed development, the Institute of Transportation Engineers (ITE) *Trip Generation* manual³ for ITE Land Use Code (LUC) 221, *Multifamily Housing (Mid-Rise)* and LUC 252 *Senior Adult Housing* were used to project traffic volume of the Project. Adjustments were applied to account for transit usage.

The proposed 150 housing units are expected to generate approximately 610 vehicle trips on an average weekday (two-way, 24-hour volume), with 36 vehicle trips (11 vehicles entering and 25 exiting) expected during the weekday morning peak hour and 46 vehicle trips (26 vehicles entering and 20 exiting) expected during the weekday evening peak hour.

Trip Distribution and Assignment

The directional distribution of site-generated trips to and from the proposed development was determined based on a review of existing travel patterns at the study area intersections. In summary, 40 percent will arrive and depart the site to/from Pleasant Street to the east, 15 percent will arrive and depart the site to/from Trapelo Road to the north, and 45 percent will arrive and depart the site to/from Trapelo Road to the south.

TRAFFIC OPERATIONS ANALYSIS

In order to assess the impact of the proposed residential on the Project area roadway network, traffic operations analyses were performed at the study intersections under:

- 2021 Baseline condition Existing,
- 2028 No-Build, and
- 2028 Build conditions.

The addition of site-related traffic will result in a measurable, but not a significant, impact on overall operations at the signalized study intersection.

OLMSTED DRIVE/PLEASANT STREET TRAFFIC SIGNAL WARRANT ANALYSIS

Under all Existing and Future conditions analyzed, the intersection of Pleasant Street at Olmsted Drive does not meet any criteria requiring the installation of a traffic signal at this intersection; therefore, the installation of a traffic signal is not recommended. The Traffic Signal Warrant Analysis section starts on page 29 of this report.

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³Trip Generation, 10th Edition; Institute of Transportation Engineers; Washington, DC; 2017.

RECOMMENDATIONS

A transportation improvement program has been developed that is designed to provide safe and efficient access to the Project and provide measures to reduce the Project vehicle trip generation and in turn congestion in the study area. The following recommendations are noted with regard to Project access and Transportation Demand Management (TDM) measures.

Project Access

Access to the Project site will be provided by way of private access driveways from the development site to Olmsted Drive. The following recommendations are offered with respect to the design and operation of the Development site driveway:

- Improved Olmstead Drive through striping of travel lanes and centerlines, with signage provided where appropriate. Vehicles exiting the Project site onto Olmsted Drive should be placed under STOP-sign control (Manual on Uniform Traffic Devices (MUTCD)⁴ R-1), with a painted STOP bar included.
- Any landscaping or building features near the new intersections and driveways should be limited to 24 inches in height or should be located out of the lines of sight for motorists.
- Snow windrows within sight triangle areas will be promptly removed where such accumulations would impede sight lines.

Off-Site Improvements

Olmsted Drive at Pleasant Street (Route 60)

In order to improve definition for vehicle movements, it is recommended that Olmstead Drive be improved through striping of travel lanes and centerlines. A painted STOP bar is also recommended to accompany the existing STOP-sign currently present at the intersection.

Transportation Demand Management (TDM) Plan

As is the case with many developments, a major focus of the traffic mitigation plan focuses on the reduction of single-occupant vehicles arriving and departing to and from the site. This is accomplished by developing a comprehensive TDM strategy. The proponent is supportive of the development of a balanced multimodal transportation plan to serve the residents when demand is warranted, and the provision of such service is economically feasible. The major features of this TDM plan that support this commitment are as follows:

• The property management team will assign a transportation coordinator to focus on coordinating transportation aspects of the Project with the Town and the promotion of alternative modes of transportation to and from the site.

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⁴Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, DC; 2009.

- While currently there is no Transportation Management Association (TMA) responsible for the Project area, the Applicant is willing to consider providing funds to initiate a TMA for the area.
- A "welcome packet" will be provided to residents detailing available public transportation services, bicycle and walking alternatives, and commuter options available.
- The Project Management Team is committed to coordinate with area shuttle services to provide site connections to the downtown areas, recreational centers, and public transportation connections.
- In order to encourage the use of public transportation, the property management team will make available public transportation schedules which will be posted in a centralized location for the residents. Transit screens/displays will be provided in the building lobby to display real-time transportation information (similar to https://transitscreen.com).
- To encourage car/vanpooling, the property management team will identify car/vanpool resources that may be available to residents of the proposed Project. This information will be posted in a centralized location for the residents, employees and visitors.
- The property management team will provide information on available pedestrian and bicycle facilities in the vicinity of the Project site. This information will be posted in a centralized location.
- Bicycle racks will be provided on-site both inside and outside the buildings.

The Project proponent will investigate the implementation of these traffic reduction strategies and will work with the Town to implement such programs.

TMMA - TRAFFIC MONITORING PROGRAM

To ensure compliance with the Traffic Monitoring and Mitigation Agreement (TMMA), the proposed Zone 3 development in conjunction with the proposed Zone 4 development shall be subject to a post occupancy traffic monitoring reporting to the Town of Belmont, including the following features:

- Data collected for the traffic monitoring program will include traffic volume entering and existing the proposed Research and Development subdistrict and the Senior Living subdistrict developments. The monitoring will involve continuous Automatic Traffic Recorder (ATR) counts on a daily basis (Data will be collected in 15-minute increments). Data shall be retained for at least one year from the data of collection.
- A "Weekly Sampling Report" shall mean a data collection report providing monitoring results over five consecutive, non-holiday weekdays, summarized by on hour intervals and by daily totals. The morning and evening peak hour volumes or each weekday will be determined, and average morning and evening peak hour volumes will be determined for the week. In addition, the daily trip totals for each weekday will be determined and average daily trips totals will be determined for the week.

- Within six months after the issuance of a building permit for a structure for the Proposed Project, a TDM plan shall be submitted to the Town.
- Within thirty days of a project located within the Research and Development subdistrict or the Senior Living subdistrict reaching a 90% occupancy level, or one year after certificate of occupancy has been issued, whichever is earlier, the proponent shall coordinate with the Town Engineer to provide a Weekly Sampling Report. The Town engineer will designate which week the data should be collected. After the determination of the week the sampling report shall be submitted to the town within seven days.

TMMA - RECOURSE ACTIONS

The proponent will take additional actions to manage site traffic conditions should the weekly sampling repot indicate that the performance goals are not being met. Triggers requiring further action include:

- Either the morning or evening average peak hour trip generation rate exceeds the permitted rate.
- The average daily trip total exceeds the permitted rate.

Additional actions that may be implemented if the performance criteria are not met may include but are not limited to:

- Prepare and submit to the Town Engineers an updated TDM plan.
- Use diligent efforts to implement such plan as soon as possible.

The proponent will provide follow-up weekly sampling reports to the Town engineer until no further violation exists. If a weekly sampling report contains a violation, then the proponent shall pay the town of Belmont a traffic mitigation payment. If the weekly sampling report continues to show violations for two weeks, then Belmont may restrict the number of parking spaces which can be used during the morning and evening peak hour to the extent that Town Engineer determines is needed to correct the violations. If two consecutive months follow-up weekly sampling reports indicate that average trip generation is below the permitted rate, the Town will return full control of parking to the proponent. The proponent will be obligated to file follow-up weekly sampling reports for one month after full control of parking has been returned.

CONCLUSIONS

Overall, the proposed Project will result in a measurable but not significant impact on overall operations. With the implementation of the above recommendations, safe and efficient access will be provided to the planned development and the development can be constructed with minimal impact to the area.

INTRODUCTION

Vanasse & Associates, Inc. (VAI) has prepared this Transportation Impact Assessment (TIA) to evaluate potential traffic impacts associated with a proposed residential development to be located off Olmsted Drive in Belmont, Massachusetts (the "Project"). This study evaluates the following specific areas as they relate to the Project:

- access requirements;
- ii) potential off-site improvements;
- iii) safety considerations; and
- identifies and analyzes existing and future traffic conditions, both with and without the Project.

PROJECT DESCRIPTION

The project is located within Zone 3 of the McLean District and is projected to consist of a total of 150 residential units. The proposal entails construction of 40 for sale, age-restricted townhouse condominiums and 110 multi-family rental units comprised of 53 age-restricted units and 57 nonage restricted units. On-site parking will be provided for approximately 257 vehicles (165 parking spaces for the apartment buildings (1.5 spaces per unit), 80 spaces for the townhouse residents (2.0 spaces per unit), and 12 visitor spaces (0.3 spaces per unit). The proposed parking spaces comply with the *McLean Zone 3 Zoning Bylaw - Section 6B.3.1g and 6B.3.2.g* for allowed parking spaces per unit per subdistrict, as amended through the September 2020 Special Town Meeting Article. Access to the Project site is provided by Olmsted Drive. Olmsted Drive is an existing private, deadend roadway which extends from an unsignalized intersection with Pleasant Street and serves Zone 3 and 4 parcels of the Mclean District.

STUDY METHODOLOGY

This study was prepared in consultation with the Town of Belmont officials and in accordance with the MassDOT Guidelines for *Transportation Impact Assessment (TIA) Guideline;* and the standards of the Traffic Engineering and Transportation Planning professions for the preparation of such reports; and was conducted in three distinct stages.

The first stage involved an assessment of existing conditions in the study area, and included:

- An inventory of roadway geometrics
- Pedestrian facilities
- Observations of traffic flow
- Review of safety characteristics along area roadways
- The collection of daily and peak-period traffic counts.

In the second stage of the study, future traffic conditions were projected out and analyzed. Specific travel demand forecasts for the Project were assessed along with future traffic demands due to expected traffic growth independent of the Project. A seven-year time horizon was selected for analyses consistent with State guidelines for the preparation of TIAs. The traffic analysis conducted in stage two identifies existing or projected future roadway capacity, traffic safety, and site access issues.

The third stage of the study presents and evaluates measures to address traffic and safety issues, if any, identified in stage two of the study.

EXISTING CONDITIONS

A comprehensive field inventory of existing conditions within the study area was conducted in December 2020. The field investigation consisted of inventorying existing roadway geometrics, pedestrian facilities, traffic volumes, operating characteristics, posted speed limits, and land use information for the major roadways providing access to the Project. The study area for the Project is listed below and graphically depicted on Figure 1.

- 1. Trapelo Road (Route 60) at Pleasant Street (Route 60)
- 2. Olmsted Drive at Pleasant Street (Route 60)

The following describes the study area roadway and intersections:

GEOMETRY

Roadways

Pleasant Street (Route 60)

Pleasant Street (Route 60) is a two-lane local arterial roadway under the jurisdiction of the Town of Belmont which traverses the study area in a general north-south orientation, providing access between Trapelo Road to the south and the Town of Arlington to the north. Within the study area, Pleasant Street provides a single 12-foot wide travel lane in each direction with an 4-foot shoulder provided along both sides of the corridor. In the vicinity of the Project site, on-street parking is prohibited along both sides of the corridor. A sidewalk is provided along the eastern side of the corridor, adjacent to the Project site. The speed limit on Pleasant Street is not posted in the vicinity of the Project. Land use along the corridor consists primarily of a mix of commercial and residential properties.





Site location and Study Area Map

Intersections

Pleasant Street at Trapelo Road

Pleasant Street and private residential driveways intersect Trapelo Road from the north and south to form a four-way intersection that operates under traffic signal control. The Trapelo Road eastbound approach provides an approximate 12-foot wide exclusive left-turn lane and an approximate 12-foot wide through/right-turn lane with an approximate 5-foot wide marked shoulder provided. The Trapelo Road westbound approach provides two approximate 12-foot wide general-purpose travel lanes with an approximate 4-foot wide marked shoulder provided. The Pleasant Street southbound approach provides an approximate 14-foot wide left-turn/through lane and an approximate 12-foot wide channelized right-turn lane that operates under YIELD-sign control. The northbound approach consists of two residential driveways that provide approximately 25 total feet in width. The traffic signal at this location operates under a three-phase signal sequence, with a protected left-turn phase provided for eastbound traffic on Trapelo Road. Sidewalks are provided along both sides of Trapelo Road and the eastern side of Pleasant Street at this location, with painted crosswalks provide across the eastbound and southbound approaches to this intersection. Land use in the vicinity of this intersection consists primarily of a mix of commercial and residential uses.

Olmsted Drive at Pleasant Street

Olmsted Drive intersects Pleasant Street from the north to form a three-way intersection that operates under STOP-sign control. The Pleasant Street eastbound approach provides an approximate 10-foot wide exclusive left-turn lane and an approximate 11-foot wide through/right-turn lane with an approximate 4-foot wide marked shoulder provided. The Pleasant Street westbound approach provides one approximate 11-foot wide general-purpose travel lane with an approximate 4-foot wide marked shoulder provided. The Olmsted Drive southbound approach provides an approximate 20-foot wide general-purpose travel lane. A sidewalk is provided along the eastern side of Pleasant Street at this location. Crosswalks are not provided for this intersection as there is no sidewalk along the north side of Pleasant Street or on Olmstead Drive. Land use in the vicinity of this intersection consists primarily of commercial uses and open space.

Figure 2 summarizes existing lane use and travel lane widths at the study area intersections as observed in December 2020.

EXISTING TRAFFIC VOLUMES

In order to determine existing traffic-volume demands and flow patterns within the study area, manual TMCs and ATRs were conducted on Tuesday, November 10, 2020. The TMC counts at Trapelo Road with Pleasant Street were conducted during the weekday morning (7:00 to 9:00 AM) and weekday evening (4:00 to 6:00 PM) peak periods. The TMC counts at the Olmsted Drive intersection with Pleasant Street were conducted from 7:00 AM to 7:00 PM. The ATR was placed at Pleasant Street east of Olmsted Drive for a 48-hour count. These time periods were selected for analysis purposes as they are representative of the peak-traffic-volume hours for both the Project and the adjacent roadway network.

(j) Unsignalized Intersection

– – Sidewalk

Crosswalk

Lane Use and Travel Lane Width

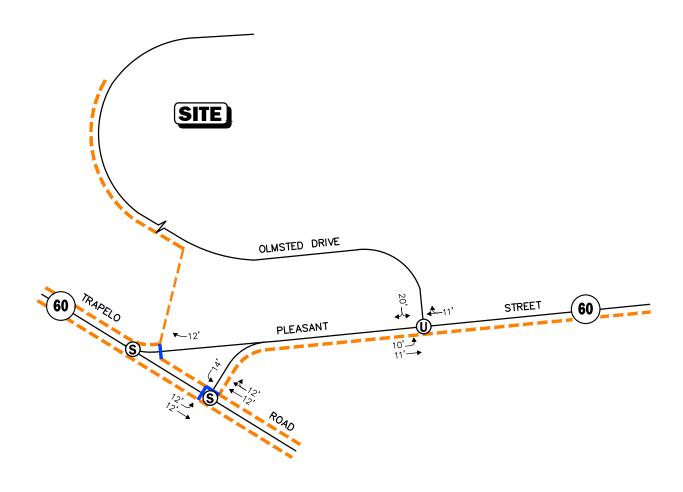




Figure 2
Existing Intersection Lane Use,

Travel Lane Width and Pedestrian Facilities

Seasonal Adjustment

In order to determine whether traffic volumes collected in November are representative of average annual conditions, historical traffic data collected by MassDOT was examined. Based on a review of this data, it was found that November traffic volumes are approximately 1 percent above averagementh conditions. In order to provide a conservative analysis, November traffic count were not adjusted downward.

Traffic Adjustment

In order to account for the reduction in traffic volumes caused by COVID-19 travel restrictions, historic traffic count data conducted in April 2018⁵ and November 2019⁶ in the same study area was reviewed. It is important to note that the 2018 data were obtained from the earlier town wide traffic study conducted by the Town of Belmont. Based upon this comparison, the November 2020 weekday morning and evening peak-hour volumes were found to be approximately 40 percent lower. The traffic counts that form the basis of this assessment were adjusted upward by 40 percent in order to provide an appropriate and conservative estimate of roadway operating conditions. It is important to note that in order to provide a 2021 Baseline condition, the November 2020 existing traffic volumes were grown by 1.0 percent per year.

The 2021 Baseline condition traffic volumes are summarized in Table 1, with the weekday morning and evening peak-hour traffic volumes graphically depicted on Figure 3.

Table 1
EXISTING ROADWAY TRAFFIC-VOLUME SUMMARY

		Weekday Morning Peak Hour (7:30–8:30 AM)				day Evening (4:30 – 5:30	
Location	Daily Volume (vpd) ^a	Volume (vph) ^b	Percent of Daily Traffic ^c	Predominant Flow	Volume (vph)	Percent of Daily Traffic	Predominant Flow
Pleasant Street east of Olmsted Drive	12,175	949	7.8	52% WB	1,148	9.4	52% WB

^aAverage daily traffic in vehicles per day (vpd) based on ATR counts collected in November 2020 (increased by 40 percent year to represent COVID adjustment and increased by 1 percent year to represent 2021 exiting condition).

WB= westbound.

As reflected in Table 1, Pleasant Street east of Olmsted Drive was found to accommodate approximately 12,175 vehicles on an average weekday (24-hour, two-way volume), with approximately 949 vph during the weekday morning peak hour and 1,148 vph during the weekday morning peak hour. The predominant flow on Pleasant Street during the weekday morning and evening peak hour is in the westbound direction.

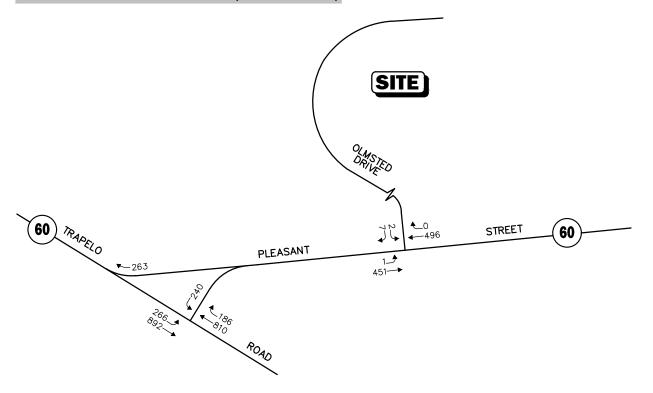
^b Manual TMCs conducted in November 2020.

^cThe percent of daily traffic that occurs during the peak hour.

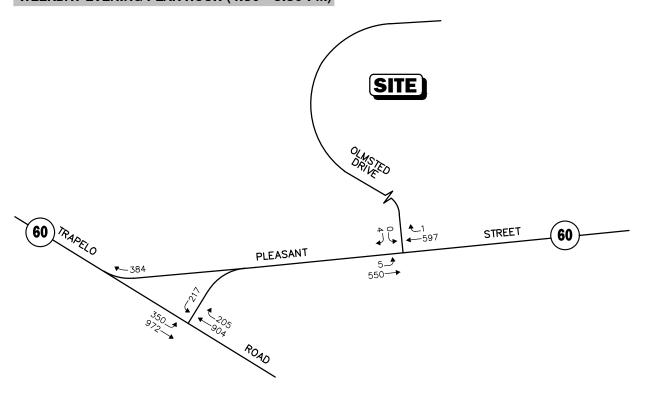
⁵Town of Belmont – Town Wide Traffic Study by BSC group, April 2019.

⁶Transportation Impact Assessment, Proposed Marijuana Dispensary, Belmont MA, VAI, February 2020.

WEEKDAY MORNING PEAK HOUR (7:30 - 8:30 AM)



WEEKDAY EVENING PEAK HOUR (4:30 - 5:30 PM)





2021 Existing Conditions Weekday Peak Hour Traffic Volumes

Figure 3

A review of the peak-period traffic counts indicates that the weekday morning peak hour generally occurs between 7:30 and 8:30 PM with the weekday evening peak hour generally occurring between 4:30 and 5:30 PM.

PEDESTRIAN AND BICYCLE FACILITIES

A comprehensive field inventory of pedestrian and bicycle facilities within the study area was undertaken in December 2020. The field inventory consisted of a review of the location of sidewalks and pedestrian crossing locations along the study area roadways and at the study area intersections.

In general, sidewalks are currently provided along the eastern side of Pleasant Street within the study area. Painted crosswalks and pedestrian signal equipment are provided at the signalized intersections with Trapelo Road and Concord Avenue. No formal bicycle facilities were noted in the study area, though the combined travel width along Pleasant Street (travel lane and shoulder) can accommodate vehicular and bicycle traffic in a shared manner.

PUBLIC TRANSPORTATION

Public transportation services, including bus and commuter rail service, are provided within the study area by the Massachusetts Bay Transportation Authority (MBTA). Specifically, commuter rail service is provided on the Fitchburg commuter rail line, with service provided within the study area via the Waverly station, located approximately 1/3 of a mile from the site and within walking distance. It is important to note that on weekdays, March 1-May 2, 2021, buses will replace all Fitchburg Line trains between Littleton/495 & Alewife for a connection with the Red Line while the signal system for Positive Train Control (PTC) be upgraded.

Local bus service is also provided within the study area by the MBTA. Bus service within the study area is provided along Trapelo Road, within walking distance of the Project site. Specifically, bus service is provided via the following routes:

- Route 73 Waverly Square Harvard Route 73 stops at the Trapelo Road intersection with Church Street approximately 0.4 miles south of the Project site. Route 73 provides a connection to Mount Auburn Hospital, Cushing Square, Harvard University, Red Line and Fitchburg Commuter Rail. Roundtrip fares for adults are \$1.70 with a Charlie Card, \$0.85 for students with valid ID, and \$0.85 for senior citizens (65 years of age or older). This line operates seven days a week Monday through Sunday (inbound: 5:02 AM to 1:41 AM; outbound: 4:47 AM to 1:25 AM), Saturday (inbound: 5:01 AM to 1:33 AM; outbound: 4:45 AM to 1:18 AM) and Sunday (inbound: 6:37 AM to 12:58 AM; outbound: 6:27 AM to 1:27 AM). All MBTA buses are handicapped and wheelchair accessible.
- Route 554 Waverly Square Downtown Boston Route 554 stops at the Trapelo Road intersection with Church Street approximately 0.4 miles south of the Project site. Route 554 provides a connection to Central Square, Waltham, Newtonville, Newton Courthouse, Brandeis University, Bentley College. This line does not operate on Saturday and Sundays. Roundtrip fares for adults are \$1.70 with a Charlie Card, \$0.85 for students with valid ID, and \$0.85 for senior citizens (65 years of age or older). This line operates Monday through Friday (inbound: 6:44 AM to 8:01 PM; outbound: 6:00 AM to 7:16 PM). All MBTA buses are handicapped and wheelchair accessible.

SPOT SPEED MEASUREMENTS

Vehicle travel speed measurements were performed on Pleasant Street (Route 60) in the vicinity of the Project site. Table 2 summarizes the vehicle travel speed measurements.

Table 2
VEHICLE TRAVEL SPEED MEASUREMENTS

	Pleasant Street Eastbound	Pleasant Street Westbound
Mean Travel Speed (mph)	31	31
85 th Percentile Speed (mph)	34	34
Speed Limit (mph)		

mph = miles per hour.

As reflected in Table 2, the mean (average) vehicle travel speed along Pleasant Street, in the vicinity of the Project site, was found to be approximately 31 miles per hour (mph) in both directions. The measured 85th percentile vehicle travel speed, or the speed at which 85 percent of the observed vehicles traveled at or below, was found to be approximately 34 mph in both directions. It is important to note that there are no speed limit signs along both directions of Pleasant Street in the vicinity of the Project site.

MOTOR VEHICLE CRASH DATA

Motor vehicle crash data was acquired from the MassDOT Safety Management/Traffic Operations Unit for the most recent five-year period available (2013 through 2017) to examine motor vehicle crash trends occurring within the study area. The data is summarized by intersection, type, and severity, and is presented in Table 3.

Table 3 MOTOR VEHICLE CRASH DATA SUMMARY^a

Scenario	Trapelo Road at Pleasant Street (Signalized)	Pleasant Street at Olmsted Drive (Unsignalized)
Year:		
2013	7	0
2014	13	0
2015	14	0
2016	8	0
<u>2017</u>	<u>7</u>	<u>1</u>
Total	49	1
Average ^b	9.8	0.20
Crash Rate ^c	0.72	0.04
Significant	No	No
Manner of Collision:		
Angle	15	1
Rear-End	19	0
Head-On	3	0
Sideswipe	10	0
Fixed Object	2	0
<u>Unknown/Other</u>	_0	<u>0</u>
Total	49	1
Time of Day:		
Weekday (Monday through Friday)	44	1
Saturday	1	0
Sunday	<u>4</u>	<u>0</u>
Total	49	1
Lighting Conditions:		
Daylight	40	0
Dawn/Dusk	3	1
Dark (lit)	5	0
Dark (unlit)	0	0
<u>Unknown</u>	<u>1</u>	<u>0</u>
Total	49	1
Pavement Conditions		
Dry	38	1
Wet	6	0
Snow	2	0
Ice	1	0
<u>Unknown (Other)</u>	_2	0
Total	49	1
Severity:		
Property Only	34	0
Injury Accident	9	1
Fatal Accident	0	0
Other	6	0
Total	49	1

^aSource: MassDOT, 2013 through 2017. ^bAverage crashes over five-year period.

^{*}Crash rate per million entering vehicles.

Unsignalized intersections are significant if rate >0.57 crashes per million vehicles (District 4) or if rate >0.57 crashes per million vehicles (Statewide).

Signalized intersections are significant if rate >0.73 crashes per million vehicles (District 4) or if rate

>0.78 crashes per million vehicles (Statewide).

As summarized in Table 3, the intersection of Pleasant Street with Trapelo Road experienced the highest frequency of accidents over the five-year review period with a total of 49 accidents reported at the intersection, averaging 9.8 accidents per year. The majority of the accidents involved property damage only (37 out of 49), occurred on dry pavement (38 out of 49), were during daylight (40 out 49), and involved angle-type and rear-end collisions (34 out of 49). The motor vehicle crash rate for this location exceeds MassDOT's average crash rate for signalized intersections in this MassDOT District (District 4). No fatalities were reported at any of the study area intersections over the five-year period reviewed. The intersection of Pleasant Street at Trapelo Road appears on the high crash location database and is included on MassDOT's HSIP listing as a high crash location. Designated as an HSIP location allows for MassDOT to prioritize funding for safety-related improvements in a specific region of the state. According to the MassDOT RSA database, no RSAs have been conducted at this location.

FUTURE CONDITIONS

Traffic volumes in the study area were projected to the year 2028, which reflects a seven-year planning horizon consistent with State Traffic Study Guidelines. Independent of the Project, traffic volumes on the roadway network in the year 2028 under No-Build conditions include all existing traffic and new traffic resulting from background traffic growth. Anticipated Project-generated traffic volumes superimposed upon this 2028 No-Build traffic network reflect the 2028 Build conditions with the Project.

FUTURE TRAFFIC GROWTH

Future traffic growth is a function of the expected land development in the immediate area and the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed estimates an annual percentage increase in traffic growth and applies that percentage to all traffic volumes under study. The drawback to such a procedure is that some turning volumes may grow at either a higher or a lower rate at particular intersections. An alternative procedure identifies the location and type of planned development, estimates the traffic to be generated, and assigns it to the area roadway network. This procedure produces a more realistic estimate of growth for local traffic. However, the drawback of this procedure is that the potential growth in population and development external to the study area would not be accounted for in the traffic projections. To provide a conservative analysis framework, both procedures were used, the salient components of which are described below.

GENERAL BACKGROUND TRAFFIC GROWTH

Traffic-volume data compiled by MassDOT from permanent count stations and historic traffic counts in the area were reviewed in order to determine general background traffic growth trends. Based on a review of this data, it was determined that traffic volumes within the study area have fluctuated over the past several years. In order to be consistent with previous traffic studies in the area, a 1.0 percent per year compounded annual growth rate was used to account for general background traffic growth for weekday morning and evening peak hour

SPECIFIC DEVELOPMENT BY OTHERS

The Town of Belmont was contacted to determine if there are any planned or approved specific development projects within the area that would have an impact on future traffic volumes at the study intersections. Based on these discussions, the following project was identified in the immediate area of the Project site, including a proposed marijuana facility to be located at 1010 Pleasant Street (Route 60).

• 1010 Pleasant Street - Proposed Marijuana Facility - This project entails the repurposing of approximately 4,150 square feet (sf) of commercial space within a multi-tenant commercial building located at 1010 Pleasant Street in Belmont, Massachusetts. The Project also includes the reconfiguration of the existing parking field in order to provide a total of 25 spaces for employees and customers of the facility. Traffic volumes associated with this project were obtained from the traffic study prepared for the facility and were added to the No-Build volumes (see distribution in Appendix).

No other developments were identified at this time that are expected to result in an increase in traffic within the study area beyond the general background traffic growth rate.

ROADWAY IMPROVEMENT PROJECTS

The Town of Belmont Planning Department was contacted to determine if there were any planned roadway improvement projects expected to be completed within the study area. Based on these discussions, no improvements are planned beyond general maintenance.

NO-BUILD TRAFFIC VOLUMES

The 2028 No-Build peak-hour traffic-volume networks were developed by applying the 1 percent per year compounded annual background traffic growth rate to the 2021 Baseline condition peak-hour traffic volumes plus the identified background development. The resulting 2028 No-Build weekday morning and evening peak-hour traffic-volume networks are shown on Figure 4.

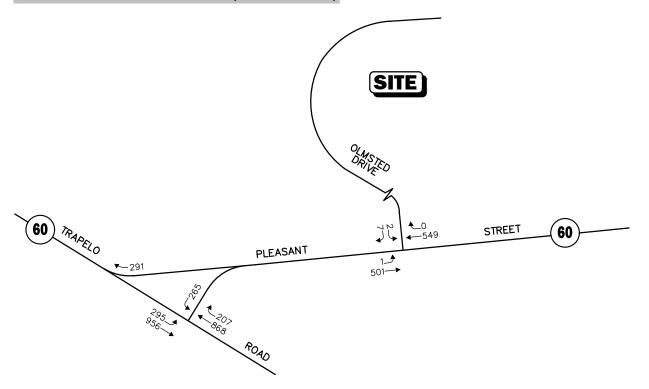
PROJECT-GENERATED TRAFFIC

The proposal entails construction of 40 age-restricted townhouse style condominiums and 110 multi-family residences including 53 age-restricted units and 57 non-age restricted units. In order to estimate the trip-generation characteristics of the proposed development, the ITE *Trip Generation* manual ITE LUC 221, *Multifamily Housing (Mid-Rise)* and LUC 252, *Senior Adult Housing* were used. Trip-generation calculations were performed for a typical weekday, as well as the weekday morning and weekday evening peak hours, the critical time periods for project-related traffic activity. Based on the Commuting Characteristics by Sex of the 2015-2019 American Community Survey 5-Year Estimation, approximately 22 percent of residents who live in Belmont (Census Tract 3577 Middlesex County) travel to work by either transit or walk trips. Due to the site's proximity to public transportation and for purposes of this study, a conservative 5 percent non-auto trip reduction was assumed for the age-restricted units and 10 percent non-auto trip reduction was assumed for the non-age restricted units. The expected vehicle-trip generation is

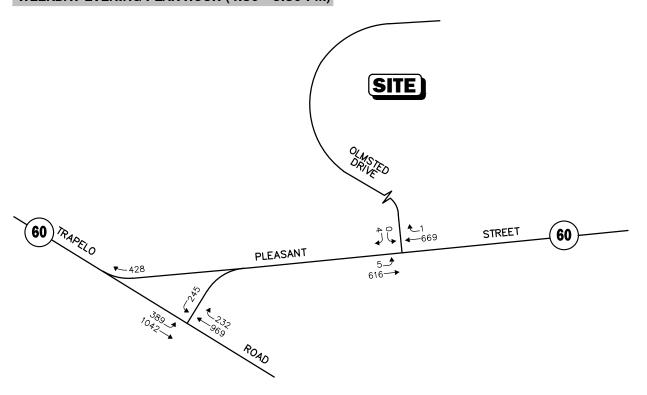
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⁷Ibid 3.

WEEKDAY MORNING PEAK HOUR (7:30 - 8:30 AM)



WEEKDAY EVENING PEAK HOUR (4:30 - 5:30 PM)





2028 No-Build Conditions Weekday Peak Hour Traffic Volumes

Figure 4

Table 4
TRIP-GENERATION SUMMARY

	ITE S	Senior Housing	3	ITE M	ultifamily Housi	ng		
Time Period/Direction	(93 Units) ^a	Transit Reduction Trips (5%)	Total	(57 Units) ^b	Transit Reduction Trips (10%)	Total	TOTAL	Total New trips
Average Weekday	348.49	17.42	331.07	308.90	30.89	278.01	609.08	610
Weekday Morning Peak Hour:								
Entering	6.45	0.32	6.13	5.34	0.53	4.81	10.94	11
<u>Exiting</u>	<u>11.97</u>	0.60	11.37	<u>15.18</u>	<u>1.52</u>	13.66	<u>25.03</u>	<u>25</u>
Total	18.42	0.92	17.50	20.52	2.05	18.47	35.97	36
Weekday Evening Peak Hour:								
Entering	13.27	0.66	12.61	15.30	1.53	13.77	26.38	26
Exiting	<u>11.31</u>	0.57	10.74	9.78	0.98	8.80	<u> 19.54</u>	<u>20</u>
Total	24.58	1.23	23.35	25.08	2.51	22.57	45.92	46

^aBased on ITE LUC 252, Senior Adult Housing.

As can be seen in Table 4, the proposed 150 housing units are expected to generate approximately 610 vehicle trips on an average weekday (two-way, 24-hour volume), with 36 vehicle trips (11 entering and 25 exiting) expected during the weekday morning peak hour and 46 vehicle trips (26 entering and 20 exiting) expected during the weekday evening peak hour.

TRAFFIC MONITORING AND MITIGATION AGREEMENT (TMMA)

In order to set up a goal for the McLean development a Traffic Mitigation and Monitoring Agreement (TMMA) was outlined in November of 1999. The TMMA includes trip-generation estimations for Zone 3 in which the proposed project is located. Trip-generation estimates for the proposed 150 housing units were compared to the TMMA limits. The comparison is summarized in Table 5.

^bBased on ITE LUC 221, Multifamily Housing (MidRise).

Table 5
PROJECT TRIP-TMMA LIMIT COMPARISON

Time Period/ Direction	150 Housing Units (Vehicle Trips)	TMMA (Vehicle Trips)
Average Weekday	610	1,148
Weekday Morning Peak Hour	36	36
Weekday Evening Peak Hour	46	92

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated trips to and from the Project was determined based on a review of existing travel patterns at the study area intersections and Journey-to-work data for Belmont obtained from the United States Census Bureau. The trip distribution for the Project is summarized in Table 6 and graphically depicted on Figure 5.

Table 6
TRIP-DISTRIBUTION SUMMARY

Roadway	Direction (To/From)	Percentage (To/From)
Trapelo Road Trapelo Road	North/West South/East	15% 45%
Pleasant Street	East	40%
TOTAL		100%

Based on these distribution patterns the weekday morning and evening peak-hour site-generated traffic volumes were assigned on the study area roadway network as shown on Figure 6.

FUTURE TRAFFIC VOLUMES - BUILD CONDITION

The 2028 Build condition networks consist of the 2028 No-Build traffic volumes, plus the proposed 150 housing units traffic added to them. The 2028 Build weekday morning and evening peak-hour traffic-volume networks are graphically depicted on Figure 7. A summary of peak-hour projected traffic-volume increases external to the study area that is the subject of this assessment is shown in Table 7. These volumes are based on the expected increases from the Project.

Table 7

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⁸ 2011-2015 5-Year American Community Survey; U.S. Census Bureau; 2019.

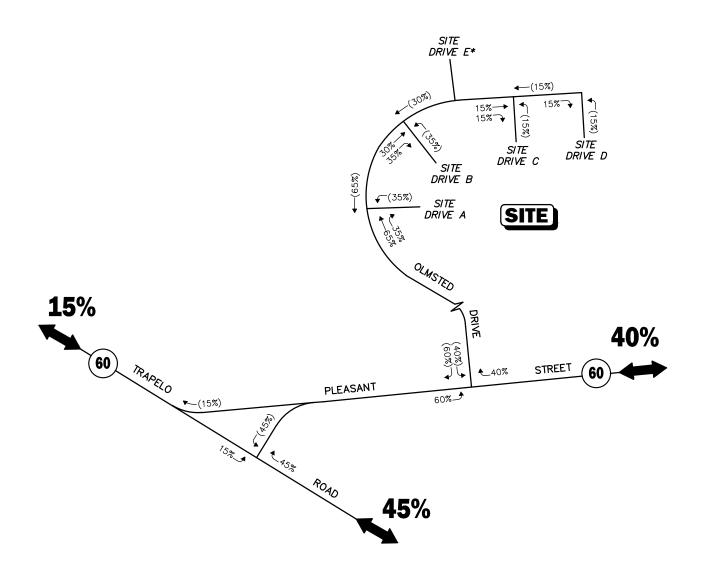
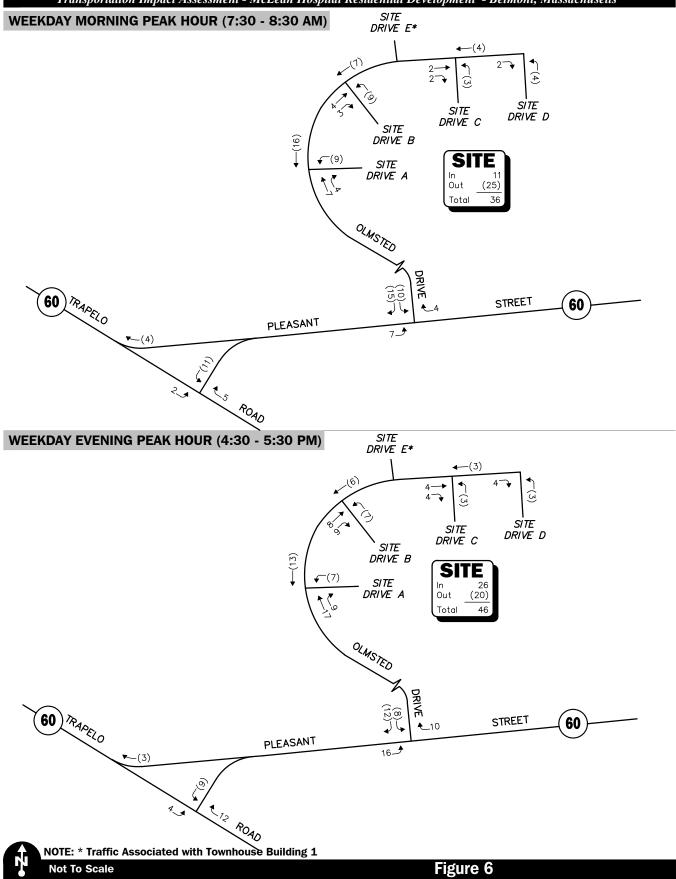




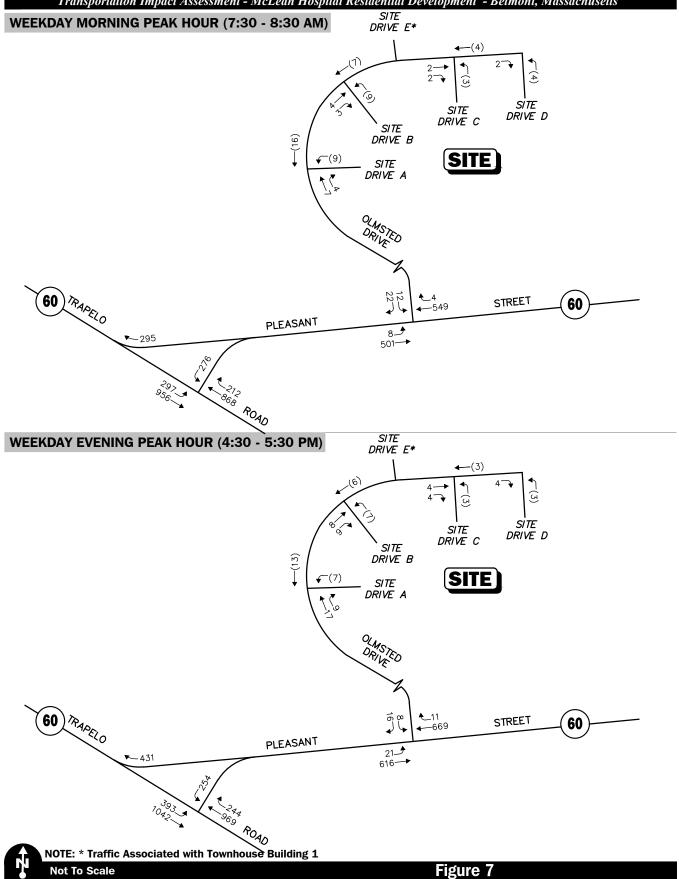
Figure 5

Trip Distribution Map





Project Generated Weekday Peak Hour Traffic Volumes





2028 Build Conditions Weekday **Peak Hour Traffic Volumes**

PEAK HOUR TRAFFIC-VOLUME INCREASES

Location/Peak Hour	2028 No-Build	2028 Build	Traffic Volume Increase Over No-Build	Percent Increase Over No-Build
Trapelo Road north of Pleasant Street:				
Weekday Morning	2,410	2,416	6	0.2
Weekday Evening	2,828	2,835	7	0.2
Trapelo Road south of Pleasant Street:				
Weekday Morning	2,296	2,312	16	0.7
Weekday Evening	2,488	2,509	21	0.8
Pleasant Street east of Olmsted Drive:				
Weekday Morning	1,052	1,066	14	1.3
Weekday Evening	1,286	1,304	18	1.4

As shown in Table 7, in comparison to future No-Build conditions, project-related traffic increases are projected to range between 0.2 to 1.4 percent on the periphery of the study area.

SIGHT DISTANCE EVALUATION

Sight distance measurements were performed at the Pleasant Street intersection with Olmsted Drive and at the proposed Project site driveway intersections with Olmsted Drive in accordance with MassDOT and American Association of State Highway and Transportation Officials (AASHTO)⁹ requirements. In brief, Stop Sight Distance (SSD) is the distance required by a vehicle traveling at the design speed of a roadway, on wet pavement, to stop prior to striking an object in its travel path. In accordance with AASHTO standards, if the measured distance is at least equal to the required SSD value for the appropriate design speed, the intersection can operate in a safe manner. Table 8 presents the measured SSD at the site driveways.

As can be seen in Table 8, the available lines of sight at the intersection of Pleasant Street at Olmsted Drive and all Project site driveway intersections with Olmsted Drive were found to exceed the recommended minimum sight distance requirements to function in a safe and efficient manner based on the observed 85th percentile approach speeds or posted speed limits.

⁹A Policy on Geometric Design of Highway and Streets, 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); Washington D.C.; 2011.

Table 8 SIGHT DISTANCE MEASUREMENTS^a

		Distance	s (Feet)	
			85 th	
	Posted	Posted	Percentile	
	Speed Limit	Speed Limit	Speed	
Sight Distance Measurement	(25 mph)	(30 mph)	(35 mph) ^b	Measured
Diamond Chart of Okused Division				
Pleasant Street at Olmsted Drive:				
Stopping Sight Distance: Pleasant Street approaching from the east		200	250	316
Pleasant Street approaching from the east		200	250	423
Fleasant Street approaching from the west		200	230	423
Site Drive A at Olmsted Drive:				
Stopping Sight Distance:				
Olmsted Drive approaching from the north	155			303
Olmsted Drive approaching from the south	155			229
Site Drive B at Olmsted Drive:				
Stopping Sight Distance:				
Olmsted Drive approaching from the north	155			234
Olmsted Drive approaching from the south	155			261
Site Drive C and D at Olmsted Drive:				
Stopping Sight Distance:				
Olmsted Drive approaching from the east	155			287
Olmsted Drive approaching from the west	155			227

^aRecommended minimum values obtained from *A Policy on Geometric Design of Highways and Streets*, 6th Edition; American Association of State Highway and Transportation Officials (AASHTO); 2011.

 $[^]b\textsc{Based}$ on 85^{th} percentile speed of 34 mph.

TRAFFIC OPERATIONS ANALYSIS

Measuring existing and future traffic volumes quantifies traffic flow within the study area. To assess quality of flow, roadway capacity and vehicle queue analyses were conducted under Existing, No-Build and Build traffic-volume conditions. Capacity analyses provide an indication of how well the roadway facilities serve the traffic demands placed upon them, with vehicle queue analyses providing a secondary measure of the operational characteristics of an intersection or section of roadway under study.

METHODOLOGY

Levels of Service

A primary result of capacity analyses is the assignment of level-of-service to traffic facilities under various traffic-flow conditions. ¹⁰ The concept of level-of-service is defined as a qualitative measure describing operational conditions within a traffic stream and their perception by motorists and/or passengers. A level-of-service definition provides an index to quality of traffic flow in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort, convenience, and safety.

Six levels of service are defined for each type of facility. They are given letter designations from A to F, with level-of-service (LOS) A representing the best operating conditions and LOS F representing congested or constrained operating conditions.

Since the level-of-service of a traffic facility is a function of the traffic flows placed upon it, such a facility may operate at a wide range of levels of service, depending on the time of day, day of week, or period of year.

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¹⁰The capacity analysis methodology is based on the concepts and procedures presented in the *Highway Capacity Manual;* Transportation Research Board; Washington, DC; 2010.

Signalized Intersections

The six levels of service for signalized intersections may be described as follows:

- LOS A describes operations with very low control delay; most vehicles do not stop at all.
- LOS B describes operations with relatively low control delay. However, more vehicles stop than LOS A.
- LOS C describes operations with higher control delays. Individual cycle failures may begin to appear. The number of vehicles stopping is significant at this level, although many still pass through the intersection without stopping.
- LOS D describes operations with control delay in the range where the influence of congestion becomes more noticeable. Many vehicles stop, and individual cycle failures are noticeable.
- LOS E describes operations with high control delay values. Individual cycle failures are frequent occurrences.
- LOS F describes operations with high control delay values that often occur with oversaturation. Poor progression and long cycle lengths may also be major contributing causes to such delay levels.

Levels of service for signalized intersections were calculated using the Percentile Delay Method implemented as a part of the SynchroTM 10 software as required by MassDOT. The Percentile Delay Method assesses the effects of signal type, timing, phasing, and progression; vehicle mix; and geometrics on "percentile" delay. Level-of-service designations are based on the criterion of percentile delay per vehicle and is a measure of: i) driver discomfort; ii) motorist frustration; and iii) fuel consumption; and includes a uniform delay based on percentile volumes using a Poisson arrival pattern, an initial queue move-up time, and a queue interaction delay that accounts for delays resulting from queues extending from adjacent intersections. Table 9 summarizes the relationship between level-of-service and percentile delay and uses the same numerical delay thresholds as the HCM method. The tabulated percentile delay criterion may be applied in assigning level-of-service designations to individual lane groups, to individual intersection approaches, or to entire intersections.

Table 9
LEVEL-OF-SERVICE CRITERIA
FOR SIGNALIZED INTERSECTIONS

Level of Service	Percentile Delay Per Vehicle (Seconds)
Α	<u>≤</u> 10.0
В	10.1 to 20.0
C	20.1 to 35.0
D	35.1 to 55.0
E	55.1 to 80.0
F	>80.0

Unsignalized Intersections

The six levels of service for unsignalized intersections may be described as follows:

- LOS A represents a condition with little or no control delay to minor street traffic.
- LOS B represents a condition with short control delays to minor street traffic.
- LOS C represents a condition with average control delays to minor street traffic.
- LOS D represents a condition with long control delays to minor street traffic.
- LOS E represents operating conditions at or near capacity level, with very long control delays to minor street traffic.
- LOS F represents a condition where minor street demand volume exceeds capacity of an approach lane, with extreme control delays resulting.

The levels of service of unsignalized intersections are determined by application of a procedure described in the 2010 *Highway Capacity Manual*. Level of service is measured in terms of average control delay. Mathematically, control delay is a function of the capacity and degree of saturation of the lane group and/or approach under study and is a quantification of motorist delay associated with traffic control devices such as traffic signals and STOP signs. Control delay includes the effects of initial deceleration delay approaching a STOP sign, stopped delay, queue move-up time, and final acceleration delay from a stopped condition. Definitions for level of service at unsignalized intersections are also given in the 2010 *Highway Capacity Manual*. Table 10 summarizes the relationship between level of service and average control delay for two-way STOP-sign-controlled and all-way STOP-sign-controlled intersections.

Table 10 LEVEL-OF-SERVICE CRITERIA FOR UNSIGNALIZED INTERSECTIONS^a

v/c < 1.0	olume-to-Capacity Ratio v/c > 1.0	_ Average Control Dela (Seconds Per Vehicle)
A	F	≤10.0
В	F	10.1 to 15.0
C	F	15.1 to 25.0
D	F	25.1 to 35.0
E	F	35.1 to 50.0
F	F	>50.0

^aSource: *Highway Capacity Manual*; Transportation Research Board; Washington, DC; 2010; page 19-2.

¹¹Highway Capacity Manual; Transportation Research Board; Washington, DC; 2010.

ANALYSIS RESULTS

Level-of-service and vehicle queue analyses were conducted for 2021 Baseline, 2028 No-Build, and 2028 Build conditions for the intersections within the study area. The results of the intersection capacity and vehicle queue analyses are summarized for signalized intersections in Table 11 and for unsignalized intersections in Table 12 with the detailed analysis results presented in the Appendix. The following is a summary of the level-of-service and delay analyses for the intersections within the study area:

Signalized Intersections

Trapelo Road at Pleasant Street

Under 2021 Existing conditions, this signalized intersection was shown to operate at an overall LOS C during the weekday morning and evening peak hours. Under future conditions, this signalized intersection is projected to continue operate at an overall LOS C during the weekday morning peak hour and at an overall LOS D during the weekday evening peak hour, with Project-related traffic increase resulting in increases to overall delays of approximately 2 seconds or less as compared to No-Build conditions.

Unsignalized Intersections

Pleasant Street at Olmsted Drive

Under existing conditions, the critical movements at this unsignalized intersection are expected to operate at LOS B during the weekday morning and evening peak hours. Under No-Build conditions, the critical movements at this unsignalized intersection are expected to operate at LOS C during the weekday morning peak hour and at LOS B during the weekday evening peak hour. Under Build conditions, the critical movements at this unsignalized intersection are expected to operate at LOS C during the weekday morning peak hour and at LOS D during the weekday evening peak hour. Under future conditions vehicle queues at this intersection were shown to range from 0 to 2 vehicle during the peak periods.

Olmsted Drive at Site Driveways

Under future conditions, the critical movements at the site driveways are expected to operate at LOS A during the weekday morning and evening peak hours. Vehicle queues are not expected at site driveways during the peak periods.

SIGNALIZED INTERSECTION LEVEL-OF-SERVICE SUMMARY Table 11

		2021 Baseline Condition	ine Conditi	on		2028	2028 No-Build			2028	2028 Build	
Signalized Intersection/ Peak Hour/Movement	V/C^a	Delay ^b	$_{\circ}\mathrm{SOT}$	Queue ^d Avg/95 th	V/C	Delay	SOT	Queue Avg/95 th	V/C	Delay	SOT	Queue Avg/95 th
Trapelo Road at Pleasant Street:												
Weekday Morning:	0.75	28.9	C	86/206	0.83	40.4	D	120/268	0.84	41.7	D	122/270
Trapelo Road EB LT	0.82	21	C	361/768	0.00	28.8	C	453/855	0.92	30.9	C	472/855
Trapelo Road EB TH	0.75	27.3	ပ	264/466	0.87	35.2	Q	326/524	0.89	37.4	Q	338/527
Trapelo Road WB TH/RT	0.7	41.2	D	135/195	0.72	40.7	Q	148/216	0.72	40.1	Q	152/226
Pleasant Street WB LT	0.35	9.9	A	39/68	0.37	7.1	A	47/85	0.37	7	A	46/87
Pleasant Street WB RT	1	24.6	ပ	1	1	31.3	ပ	1	ı	32.9	C	1
Overall												
Weekday Evening:	0.82	36.8	Q	144/322	0.94	55.5	Щ	185/371	0.94	59.5	Щ	189/378
Trapelo Road EB LT	0.87	23.3	ပ	409/848	0.93	29	ပ	514/873	0.93	28.3	ပ	514/873
Trapelo Road EB TH	0.92	40.8	D	364/549	86.0	47.2	Q	384/543	0.99	48.7	Q	397/552
Trapelo Road WB TH/RT	69.0	42.1	Q	123/182	0.77	49.4	Q	139/249	0.81	53.5	Q	145/263
Pleasant Street WB LT	0.47	6.6	A	80/136	0.58	18.2	В	159/253	0.71	12.4	В	31/132
Pleasant Street WB RT	1	31.0	၁	1	1	38.9	Q	!	1	39.0	Q	1
Overall												

^aVolume-to-capacity ratio.
^bControl (signal) delay per vehicle in seconds.
^cLevel-of-Service.

^dQueue length in feet.

NB = northbound; SB = southbound; RT = right-turning movements; TH = through movements; RT = right-turning movements.

UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE AND VEHICLE QUEUE SUMMARY Table 12

	2	2021 Baseline Condition	e Conditio	r		2028 N	2028 No-Build			2028	2028 Build	
Unsignalized Intersection/ Peak Hour/Movement	Demand ^a	Delay ^b	SOT	Queue 95 th Percentile	Demand	Delay	SOT	Queue 95 th Percentile	Demand	Delay	SOT	Queue 95 th Percentile
Olmsted Drive at Pleasant Street: Weekday Morning: Olmsted Drive SB LT RT	6	14.2	В	0.2	6	15.3	C	0.2	40	21.7	Ü	1.4
n eekaay Evening: Olmsted Drive SB LT RT	4	13.5	В	0.1	4	14.5	В	0.1	25	27.9	D	1.4
Olmsted Drive at Site Drive A: Weekday Morning: Olmsted Drive WB LT RT	ŀ	I	ı	!	ŀ	ŀ	ı	I	6	8.7	A	0.0
N eekaay Evening: Olmsted Drive WB LT RT	1	ł	ŀ	;	ŀ	ŀ	ŀ	ŀ	7	8.7	Ą	0.0
Olmsted Drive at Site Drive B: Weekday Morning: Olmsted Drive NB LT RT	1	ŀ	1	!	1	ı	1	ŀ	6	9.8	4	0.0
Weekday Evening: Olmsted Drive NB LT RT	ı	1	ı	:	I	ı	ı	ı	7	8.6	Ą	0.0
Olmsted Drive at Site Drive C: Weekday Morning: Olmsted Drive NB LT RT	ŀ	I	ı	1	ŀ	ŀ	ı	I	w	8.6	A	0.0
Weekday Evening: Olmsted Drive NB LT RT	1	ŀ	ŀ	;	ŀ	ŀ	ŀ	:	9	8.6	Ą	0.0
Olmsted Drive at Site Drive D: Weekday Morning: Olmsted Drive NB LT RT	1	ł	1	!	ŀ	I	1	ł	4	8.5	Ą	0.0
Weekday Evening: Olmsted Drive NB LT RT	ŀ	ŀ	ŀ	;	ŀ	ŀ	ŀ	ŀ	3	8.5	Ą	0.0

^aVolume-to-capacity ratio.

^bControl (signal) delay per vehicle in seconds.

^cLevel-of-Service.

^dQueue length in vehicle.

NB = northbound; SB = southbound; RB = westbound; LT = left-turning movements; TH = through movements; RT = right-turning movements.

OLMSTED DRIVE AT PLEASANT STREET TRAFFIC SIGNAL WARRANT ANALYSIS

A Traffic Signal Warrant Analysis (TSWA) has been conducted for the intersection of Pleasant Street at Olmsted Drive as required in the TMMA. The TMMA states that a traffic signal should be evaluated at this intersection along with traffic projections of the future McLean Zone 4. In order to project the impacts of the future development within the McLean Zone 4 District, the maximum level of permitted traffic for this District were reviewed, as stipulated in the TTMA. Based on the TTMA, the Zone 4 entails construction of a research and development building and is expected to generate approximately 1,784 vehicle trips on an average weekday, with 206 vehicle trips expected during the weekday morning peak hour and 180 vehicle trips expected during the weekday evening peak hour.

The MUTCD₁₂ establishes nine warrants or criteria to evaluate a location for the installation or retention of a traffic signal. At least one of the nine warrants should be satisfied in order to justify the installation of a traffic signal; however, satisfaction of a warrant in and of itself does not justify traffic signal control. An engineering evaluation of the location in question should indicate that the establishment of traffic signal control will improve the overall safety and/or operation of the intersection. Table 13 identifies the nine traffic signal warrants.

12 Ibid	1
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Table 13
TRAFFIC SIGNAL WARRANTS^a

Warrant No.	Description
1	Eight-Hour Vehicular Volume
	Condition A – Min. Vehicular Volume ^b
	Condition B – Interruption of Continuous Traffic
2	Four-Hour Vehicular Volume
3	Peak Hour
4	Pedestrian Volume
5	School Crossing
6	Coordinated Signal System
7	Crash Experience
8	Roadway Network
9	Intersection near a Grade Crossing

^aMUTCD; Federal Highway Administration; Washington, DC; 2009.

In order to performance an analysis a 12-hour TMC was conducted at Pleasant Street at Olmsted Drive in November 2020. The hourly intersection volumes were then grown by 40 percent per year to represent COVID-19 adjustments and increased by 1 percent year to represent 2021 existing conditions. The 2028 No-Build traffic-volume networks were developed by applying the 1 percent per year compounded annual background traffic growth rate to the 2021 baseline condition peak-hour traffic volumes plus the identified background developments. The proposed background development trips were then added to develop volumes for the 2028 No-Build conditions, to which the trips from the Project were added in order to obtains the 2028 Build traffic signal warrant analysis. An additional condition analyzed included trips associated with both the Project and Zone 4. Table 14 identifies the results of the TSWA for the study intersection under existing and Future conditions.

^bA large number of intersecting traffic is the principal reason to consider installing a traffic control signal.

^cTraffic volume on a major street is so heavy that traffic on a minor intersecting street suffer excessive delay in entering or crossing the major street.

Table 14
TRAFFIC SIGNAL WARRANTS ANALYSIS RESULTS

Warrant No.	Description	Satisfied for 2021 Existing Conditions	Satisfied for 2028 No-Build Conditions	Satisfied for 2028 Build Conditions w/Zone 3	Satisfied for 2028 Build Conditions w/Zones 3 and 4
1	Eight-Hour Vehicular Volume				
	Condition A – Min. Vehicular Volume	No	No	No	No
	Condition B – Interruption of Continuous Traffic	No	No	No	No
2	Four-Hour Vehicular Volume	No	No	No	No
3	Peak Hour	No	No	No	No
4	Pedestrian Volume	No	No	No	No
5	School Crossing	No	No	No	No
6	Coordinated Signal System	No	No	No	No
7	Crash Experience	No	No	No	No
8	Roadway Network	No	No	No	No
9	Grade Crossing	No	No	No	No

As can be seen in Table 14, under all conditions analyzed the intersection of Pleasant Street at Olmsted Drive does not meet any of the warrant criteria. Accordingly, the installation of a traffic signal at this intersection is not justified. The detailed TSWA worksheets are provided in the Appendix.

RECOMMENDATIONS AND CONCLUSION

VAI has prepared this TIA to evaluate potential traffic impacts associated with the proposed residential development to be located off Olmsted Drive in Belmont, Massachusetts. This study was prepared in accordance with the MassDOT Guidelines for *TIA Guidelines* and was conducted pursuant to the standards of the Traffic Engineering and Transportation Planning Professions for the preparation of such reports. Based on the results of this study, the following can be concluded:

- The proposed 150 housing units are expected to generate approximately 610 vehicle trips on an average weekday (two-way, 24-hour volume), with 36 vehicle trips (11 entering and 25 exiting) expected during the weekday morning peak hour and 46 vehicle trips (26 entering and 20 exiting) expected during the weekday evening peak hour.
- Project-related traffic increases in the area are expected to be between 0.2 and 1.4 percent during the peak hours.
- Lines of sight were found to exceed the recommended minimum sight distance requirements to function in a safe and efficient manner at the intersection of Pleasant Street at Olmsted Drive and all Project site driveway intersections with Olmsted Drive.
- The analysis has indicated that the Project will result in minimal impact on motorist delays at the study intersections, as compared to future No-Build conditions.
- The installation of a traffic signal at Pleasant Street at Olmsted Drive is not justified.

RECOMMENDATIONS

A transportation improvement program has been developed that is designed to provide safe and efficient access to the Project and provide measures to reduce the Project vehicle trip generation and in turn congestion in the study area. The following recommendations are noted with regard to Project access and Transportation Demand Management (TDM) measures.

Project Access

Access to the project site will be located by way of private access driveways from the development site to Olmsted drive. The following recommendations are offered with respect to the design and operation of the development site driveway:

- Improved Olmstead Drive through striping of travel lanes and centerlines, with signage provided where appropriate. Vehicles exiting the Project site onto Olmsted Drive should be placed under STOP-sign control (Manual on Uniform Traffic Devices (MUTCD)¹³ R-1), with a painted STOP bar included.
- Any landscaping or building features near the new intersections and driveways should be limited to 24 inches in height or located out of the lines of sight for motorists.
- Snow windrows within sight triangle areas will be promptly removed where such accumulations would impede sight lines.

Off-Site Improvements

Olmsted Drive at Pleasant Street (Route 60)

In order to improve definition for vehicle movements, it is recommended that Olmstead Drive leg be improved through striping of travel lanes and centerlines. A painted STOP bar is also recommended to accompany the existing STOP-sign currently present at the intersection.

Figure 8 graphically depicts the proposed improvements at the proposed site driveway and within the study area as is detailed above.

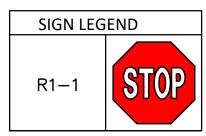
Transportation Demand Management (TDM) Plan

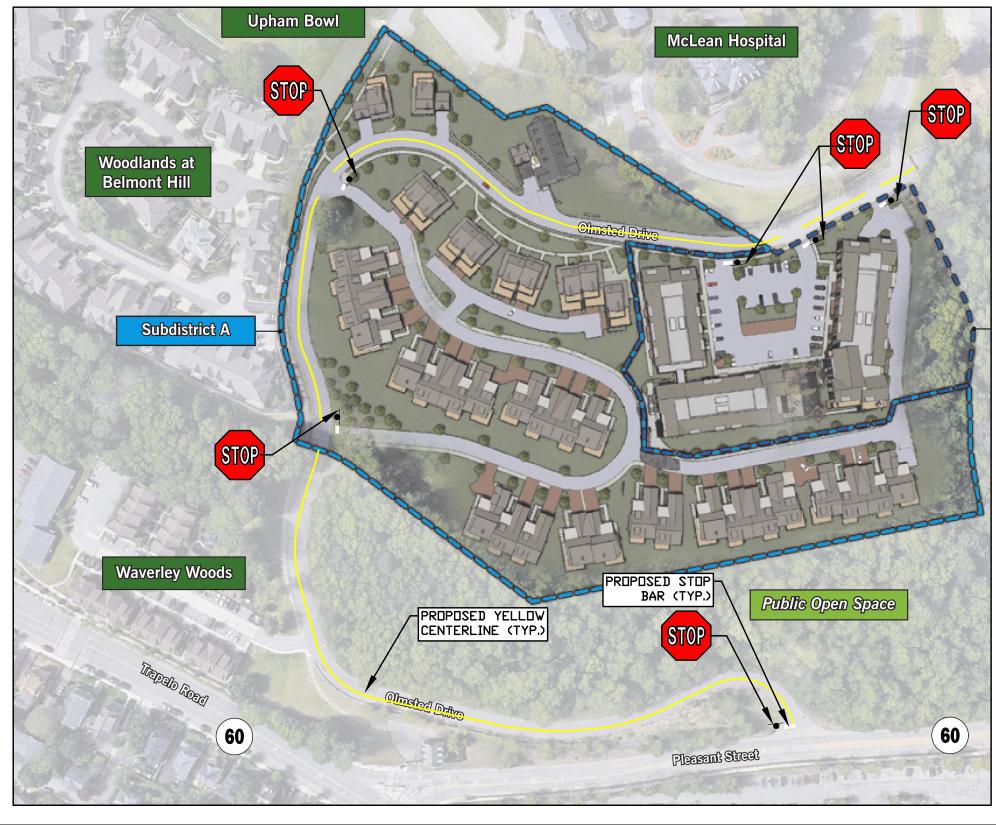
As is the case with many developments, a major focus of the traffic mitigation plan focuses on the reduction of single-occupant vehicles arriving and departing to and from the site. This is accomplished by developing a comprehensive TDM strategy. The proponent is supportive of the development of a balanced multimodal transportation plan to serve the residents when demand is warranted, and the provision of such service is economically feasible. The major features of this TDM plan that support this commitment are as follows:

- The property management team will assign a transportation coordinator to focus on coordinating transportation aspects of the Project with the Town and the promotion of alternative modes of transportation to and from the site.
- While currently there is no Transportation Management Association (TMA) responsible for the Project area, the Applicant is willing to consider providing funds to initiate a TMA for the area.
- A "welcome packet" will be provided to residents detailing available public transportation services, bicycle and walking alternatives, and commuter options available.
- The Project Management Team is committed to coordinate with area shuttle services to

-

¹³Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, DC; 2009.





NOTES: 1. THIS PLAN IS FOR REVIEW PURPOSES ONLY AND IS NOT INTENDED FOR CONSTRUCTION.



Figure 8

Conceptual Improvement Plan

provide site connections to the downtown areas, recreational centers, and public transportation connections.

- In order to encourage the use of public transportation, the property management team will make available public transportation schedules which will be posted in a centralized location for the residents. Transit screens/displays will be provided in the building lobby to display real-time transportation information (similar to https://transitscreen.com).
- To encourage car/vanpooling, the property management team will identify car/vanpool resources that may be available to residents of the proposed Project. This information will be posted in a centralized location for the residents, employees and visitors.
- The property management team will provide information on available pedestrian and bicycle facilities in the vicinity of the Project site. This information will be posted in a centralized location.
- Bicycle racks will be provided on-site both inside and outside the buildings.

The Project proponent will investigate the implementation of these traffic reduction strategies and will work with the Town to implement such programs.

TMMA - TRAFFIC MONITORING PROGRAM

To ensure compliance with the Traffic Monitoring and Mitigation Agreement (TMMA), the proposed Zone 3 development in conjunction with the proposed Zone 4 development shall be subject to a post occupancy traffic monitoring reporting to the Town of Belmont, including the following features:

- Data collected for the traffic monitoring program will include traffic volume entering and existing the proposed Research and Development subdistrict and the Senior Living subdistrict developments. The monitoring will involve continuous Automatic Traffic Recorder (ATR) counts on a daily basis (Data will be collected in 15-minute increments). Data shall be retained for at least one year from the data of collection.
- A "Weekly Sampling Report" shall mean a data collection report providing monitoring results over five consecutive, non-holiday weekdays, summarized by on hour intervals and by daily totals. The morning and evening peak hour volumes or each weekday will be determined, and average morning and evening peak hour volumes will be determined for the week. In addition, the daily trip totals for each weekday will be determined and average daily trips totals will be determined for the week.
- Within six months after the issuance of a building permit for a structure for the Proposed Project, a TDM plan shall be submitted to the Town.
- Within thirty days of a project located within the Research and Development subdistrict or the Senior Living subdistrict reaching a 90% occupancy level, or one year after certificate of occupancy has been issued, whichever is earlier, the proponent shall coordinate with the Town Engineer to provide a Weekly Sampling Report. The Town engineer will designate which week the data should be collected. After the determination of the week the sampling report shall be submitted to the town within seven days.

TMMA - RECOURSE ACTIONS

The proponent will take additional actions to manage site traffic conditions should the weekly sampling repot indicate that the performance goals are not being met. Triggers requiring further action include:

- Either the morning or evening average peak hour trip generation rate exceeds the permitted rate.
- The average daily trip total exceeds the permitted rate.

Additional actions that may be implemented if the performance criteria are not met may include but are not limited to:

- Prepare and submit to the Town Engineers an updated TDM plan.
- Use diligent efforts to implement such plan as soon as possible.

The proponent will provide follow-up weekly sampling reports to the Town engineer until no further violation exists. If a weekly sampling report contains a violation, then the proponent shall pay the town of Belmont a traffic mitigation payment. If the weekly sampling report continues to show violations for two weeks, then Belmont may restrict the number of parking spaces which can be used during the morning and evening peak hour to the extent that Town Engineer determines is needed to correct the violations. If two consecutive months follow-up weekly sampling reports indicate that average trip generation is below the permitted rate, the Town will return full control of parking to the proponent. The proponent will be obligated to file follow-up weekly sampling reports for one month after full control of parking has been returned.

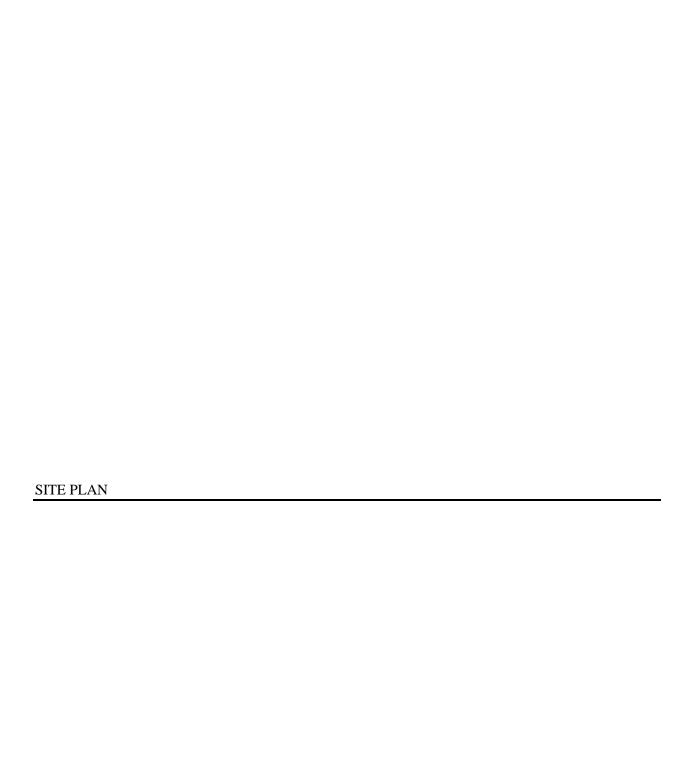
CONCLUSIONS

The proposed Project will result in a measurable but a significant impact on overall operations. With the implementation of the above recommendations, safe and efficient access will be provided to the planned development and the development can be constructed with minimal impact to the area.

APPENDIX

CAPACITY ANALYSIS

SITE PLAN
TURNING MOVEMENT COUNTS
AUTOMATIC TRAFFIC RECORDER
SEASONAL ADJUSTMENT
COVID ADJUSTMENTS
MOTOR VEHICLE CRASH DATA
PUBLIC TRANSPORTATION SCHEDULE
BACKGROUND DEVELOPMENT
GENERAL BACKGROUND TRAFFIC GROWTH
US CENSUS
TRIP GENERATION
TRIP DISTRIBUTION
TRAFFIC SIGNAL WARRANT ANALYSIS (TSWA)











N/S Street: Pleasant Street E/W Street: Trapelo Road City/State: Belmont, MA Weather: Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 1

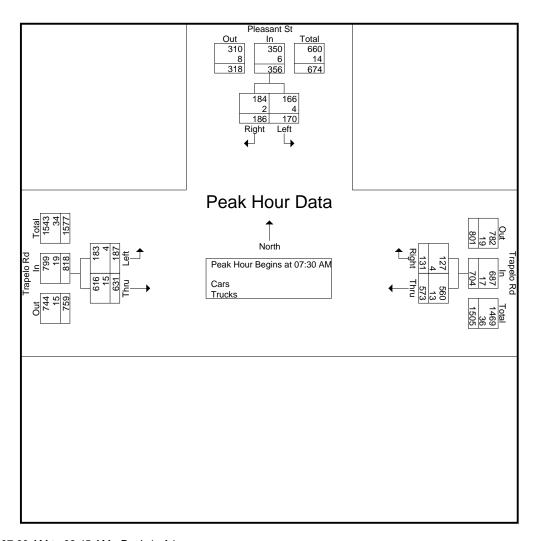
Groups Printed- Cars - Trucks

		Trapelo Rd From West		Trapelo Rd From East		Pleasant St From North	
Int. Total	Thru	Left	Right	Thru	Right	Left	Start Time
347	111	37	14	116	37	32	07:00 AM
383	146	46	17	101	36	37	07:15 AM
465	157	62	33	126	41	46	07:30 AM
463	160	40	34	145	41	43	07:45 AM
1658	574	185	98	488	155	158	Total
472	145	44	37	157	54	35	08:00 AM
478	169	41	27	145	50	46	08:15 AM
447	151	41	26	157	45	27	08:30 AM
428	155	38	19	128	54	34	08:45 AM
1825	620	164	109	587	203	142	Total
3483	1194	349	207	1075	358	300	Grand Total
	77.4	22.6	16.1	83.9	54.4	45.6	Apprch %
	34.3	10	5.9	30.9	10.3	8.6	Total %
3408	1165	342	201	1053	352	295	Cars
97.8	97.6	98	97.1	98	98.3	98.3	% Cars
75	29	7	6	22	6	5	Trucks
2.2	2.4	2	2.9	2	1.7	1.7	% Trucks

		Pleasant St From North			Trapelo Rd From East			Trapelo Rd From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From	n 07:00 AM to	08:45 AM - F	Peak 1 of 1		-					
Peak Hour for Entire Inte	rsection Begii	ns at 07:30 A	M							
07:30 AM	46	41	87	126	33	159	62	157	219	465
07:45 AM	43	41	84	145	34	179	40	160	200	463
08:00 AM	35	54	89	157	37	194	44	145	189	472
08:15 AM	46	50	96	145	27	172	41	169	210	478
Total Volume	170	186	356	573	131	704	187	631	818	1878
% App. Total	47.8	52.2		81.4	18.6		22.9	77.1		
PHF	.924	.861	.927	.912	.885	.907	.754	.933	.934	.982
Cars	166	184	350	560	127	687	183	616	799	1836
% Cars	97.6	98.9	98.3	97.7	96.9	97.6	97.9	97.6	97.7	97.8
Trucks	4	2	6	13	4	17	4	15	19	42
% Trucks	2.4	1.1	1.7	2.3	3.1	2.4	2.1	2.4	2.3	2.2

N/S Street: Pleasant Street E/W Street: Trapelo Road City/State: Belmont, MA Weather: Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

Lacil Appl	Oddin Dogins	aı.							
	07:30 AM			07:45 AM			07:30 AM		
+0 mins.	46	41	87	145	34	179	62	157	219
+15 mins.	43	41	84	157	37	194	40	160	200
+30 mins.	35	54	89	145	27	172	44	145	189
+45 mins.	46	50	96	157	26	183	41	169	210
Total Volume	170	186	356	604	124	728	187	631	818
% App. Total	47.8	52.2		83	17		22.9	77.1	
PHF	.924	.861	.927	.962	.838	.938	.754	.933	.934
Cars	166	184	350	591	118	709	183	616	799
% Cars	97.6	98.9	98.3	97.8	95.2	97.4	97.9	97.6	97.7
Trucks	4	2	6	13	6	19	4	15	19
% Trucks	2.4	1.1	1.7	2.2	4.8	2.6	2.1	2.4	2.3

N/S Street: Pleasant Street E/W Street : Trapelo Road City/State : Belmont, MA Weather : Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 10

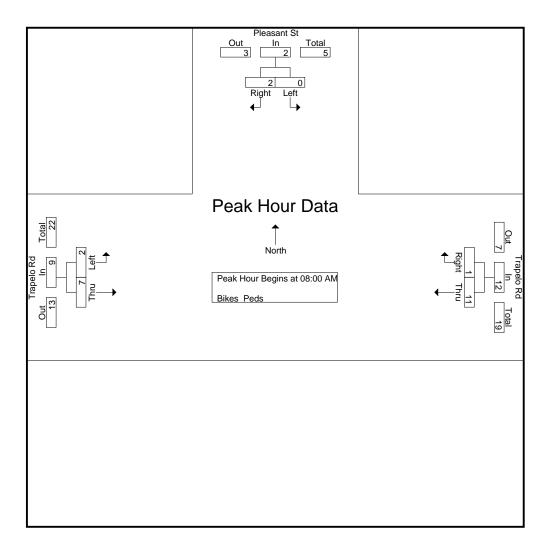
Groups Printed- Bikes Peds

		Groups Printed- Bikes Peas										
	PI	easant St		Tr	apelo Rd		Т	rapelo Rd				
	Fr	om North		Fi	rom East		F	rom West				
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	2	0	0	0	3	0	0	5	5
07:15 AM	0	0	2	0	0	1	0	3	0	3	3	6
07:30 AM	0	0	3	0	0	0	0	5	2	5	5	10
07:45 AM	1	0	2	0	0	0	1	0	0	2	2	4_
Total	1	0	7	2	0	1	1	11	2	10	15	25
08:00 AM	0	0	5	5	0	0	0	0	3	8	5	13
08:15 AM	0	0	1	0	0	1	0	2	1	3	2	5
08:30 AM	0	1	4	3	1	0	1	1	2	6	7	13
08:45 AM	0	1	2	3	0	0	1	4	0	2	9	11_
Total	0	2	12	11	1	1	2	7	6	19	23	42
Grand Total	1	2	19	13	1	2	3	18	8	29	38	67
Apprch %	33.3	66.7		92.9	7.1		14.3	85.7				
Total %	2.6	5.3		34.2	2.6		7.9	47.4		43.3	56.7	

		Pleasant St From North			Trapelo Rd From East		Trapelo Rd From West			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 07:00 AM to	08:45 AM - P	eak 1 of 1							
Peak Hour for Entire Inte	rsection Begin	ns at 08:00 AM	И .							
08:00 AM	0	0	0	5	0	5	0	0	0	5
08:15 AM	0	0	0	0	0	0	0	2	2	2
08:30 AM	0	1	1	3	1	4	1	1	2	7
08:45 AM	0	1	1	3	0	3	1	4	5	9_
Total Volume	0	2	2	11	1	12	2	7	9	23
% App. Total	0	100		91.7	8.3		22.2	77.8		
PHF	.000	.500	.500	.550	.250	.600	.500	.438	.450	.639

N/S Street: Pleasant Street E/W Street: Trapelo Road City/State: Belmont, MA Weather: Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 11



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

T Call Hour for Edon Appr		<u> </u>							
	07:45 AM			08:00 AM			07:00 AM		
+0 mins.	1	0	1	5	0	5	0	3	3
+15 mins.	0	0	0	0	0	0	0	3	3
+30 mins.	0	0	0	3	1	4	0	5	5
+45 mins.	0	1	1	3	0	3	1	0	1
Total Volume	1	1	2	11	1	12	1	11	12
% App. Total	50	50		91.7	8.3		8.3	91.7	
PHF	.250	.250	.500	.550	.250	.600	.250	.550	.600

N/S Street: Pleasant Street E/W Street : Trapelo Road City/State : Belmont, MA Weather : Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 1

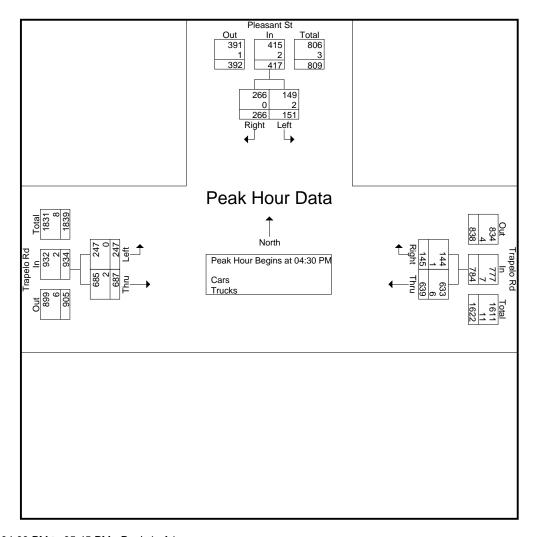
Groups Printed- Cars - Trucks

	Pleasant S From North		Trapelo From E		Trapel From \		
Start Time	Left	Right	Thru	Right	Left	Thru	Int. Total
04:00 PM	35	63	158	32	73	168	529
04:00 FM	36	48	157	35	48	168	492
1	42	-			_		
04:30 PM		54	166	45	67	186	560
04:45 PM	41	76	152	33	55	167	524
Total	154	241	633	145	243	689	2105
		1		1		1	
05:00 PM	24	70	161	26	71	161	513
05:15 PM	44	66	160	41	54	173	538
05:30 PM	32	53	131	26	51	138	431
05:45 PM	31	37	103	25	50	109	355
Total	131	226	555	118	226	581	1837
		1		1		1	
Grand Total	285	467	1188	263	469	1270	3942
Apprch %	37.9	62.1	81.9	18.1	27	73	
Total %	7.2	11.8	30.1	6.7	11.9	32.2	
Cars	283	467	1178	260	469	1265	3922
% Cars	99.3	100	99.2	98.9	100	99.6	99.5
Trucks	2	0	10	3	0	5	20
% Trucks	0.7	0	0.8	1.1	0	0.4	0.5

		Pleasant St		-	Trapelo Rd					
		From North			From East			From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 04:00 PM to	05:45 PM - P	eak 1 of 1							
Peak Hour for Entire Inte	rsection Begin	s at 04:30 PM	Μ .							
04:30 PM	42	54	96	166	45	211	67	186	253	560
04:45 PM	41	76	117	152	33	185	55	167	222	524
05:00 PM	24	70	94	161	26	187	71	161	232	513
05:15 PM	44	66	110	160	41	201	54	173	227	538
Total Volume	151	266	417	639	145	784	247	687	934	2135
% App. Total	36.2	63.8		81.5	18.5		26.4	73.6		
PHF	.858	.875	.891	.962	.806	.929	.870	.923	.923	.953
Cars	149	266	415	633	144	777	247	685	932	2124
% Cars	98.7	100	99.5	99.1	99.3	99.1	100	99.7	99.8	99.5
Trucks	2	0	2	6	1	7	0	2	2	11
% Trucks	1.3	0	0.5	0.9	0.7	0.9	0	0.3	0.2	0.5

N/S Street: Pleasant Street E/W Street: Trapelo Road City/State: Belmont, MA Weather: Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

I cak Hour for Lacil Appl	Dacii Degina	aı.							
	04:30 PM			04:30 PM			04:30 PM		
+0 mins.	42	54	96	166	45	211	67	186	253
+15 mins.	41	76	117	152	33	185	55	167	222
+30 mins.	24	70	94	161	26	187	71	161	232
+45 mins.	44	66	110	160	41	201	54	173	227
Total Volume	151	266	417	639	145	784	247	687	934
% App. Total	36.2	63.8		81.5	18.5		26.4	73.6	
PHF	.858	.875	.891	.962	.806	.929	.870	.923	.923
Cars	149	266	415	633	144	777	247	685	932
% Cars	98.7	100	99.5	99.1	99.3	99.1	100	99.7	99.8
Trucks	2	0	2	6	1	7	0	2	2
% Trucks	1.3	0	0.5	0.9	0.7	0.9	0	0.3	0.2

N/S Street : Pleasant Street E/W Street : Trapelo Road City/State : Belmont, MA Weather : Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 10

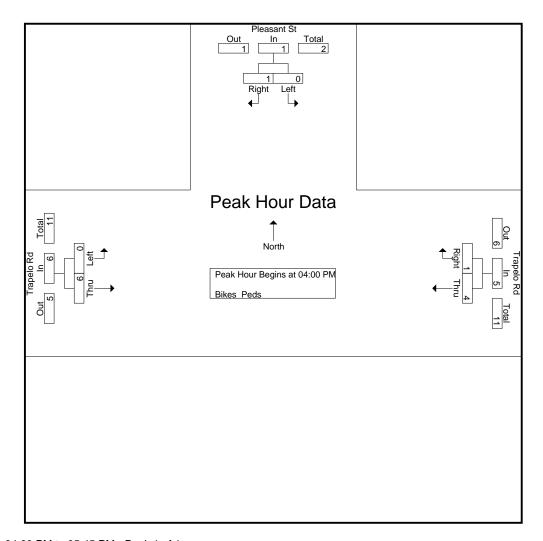
Groups Printed- Bikes Peds

		Oloups i liliteu- bikes i eus								,		
		leasant St			Γrapelo Rd			Trapelo Rd				
	Fr	rom North			From East			From West				
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	3	0	1	0	0	3	1	4	4	8
04:15 PM	0	1	5	3	0	0	0	1	1	6	5	11
04:30 PM	0	0	8	1	0	0	0	0	5	13	1	14
04:45 PM	0	0	7	0	0	0	0	2	1	8	2	10
Total	0	1	23	4	1	0	0	6	8	31	12	43
05:00 PM	0	0	6	0	0	0	0	2	5	11	2	13
05:15 PM	0	0	9	0	0	0	0	0	2	11	0	11
05:30 PM	0	0	2	0	0	0	0	0	1	3	0	3
05:45 PM	1	0	5	0	0	0	1	0	0	5	2	7_
Total	1	0	22	0	0	0	1	2	8	30	4	34
Grand Total	1	1	45	4	1	0	1	8	16	61	16	77
Apprch %	50	50		80	20		11.1	88.9				
Total %	6.2	6.2		25	6.2		6.2	50		79.2	20.8	

		Pleasant St From North			Trapelo Rd From East			Trapelo Rd From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From	n 04:00 PM to	05:45 PM - P	eak 1 of 1							
Peak Hour for Entire Inte	rsection Begin	s at 04:00 PN	Л .							
04:00 PM	0	0	0	0	1	1	0	3	3	4
04:15 PM	0	1	1	3	0	3	0	1	1	5
04:30 PM	0	0	0	1	0	1	0	0	0	1
04:45 PM	0	0	0	0	0	0	0	2	2	2
Total Volume	0	1	1	4	1	5	0	6	6	12
% App. Total	0	100		80	20		0	100		
PHF	.000	.250	.250	.333	.250	.417	.000	.500	.500	.600

N/S Street: Pleasant Street E/W Street: Trapelo Road City/State: Belmont, MA Weather: Clear

File Name: 80580002 Site Code : 80580002 Start Date : 11/10/2020 Page No : 11



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

	04:00 PM			04:00 PM			04:00 PM		
+0 mins.	0	0	0	0	1	1	0	3	3
+15 mins.	0	1	1	3	0	3	0	1	1
+30 mins.	0	0	0	1	0	1	0	0	0
+45 mins.	0	0	0	0	0	0	0	2	2
Total Volume	0	1	1	4	1	5	0	6	6
% App. Total	0	100		80	20		0	100	
PHF	.000	.250	.250	.333	.250	.417	.000	.500	.500

N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 1

Groups Printed- Cars - Trucks

		Group	s Printed- Cars - Trucl	KS			
	Olmsted Dr		Pleasant St		Pleasant St		
	From North		From East		From West		
Start Time	Left	Right	Thru	Right	Left	Thru	Int. Total
07:00 AM	0	0	77	0	0	51	128
07:15 AM	1	2	69	1	2	61	136
07:30 AM	1	5	79	0	0	93	178
07:45 AM	1	2	82	0	0	78	163
Total	3	9	307	1	2	283	605
		1		1		1	
08:00 AM	0	0	89	0	0	81	170
08:15 AM	0	0	98	0	1	67	166
08:30 AM	2	1	73	1	1	69	147
08:45 AM	0	1	84	1	0	55	141
Total	2	2	344	2	2	272	624
1		1				1	
09:00 AM	1	2	91	1	0	60	155
09:15 AM	1	2	61	0	1	54	119
09:30 AM	1	2	67	0	1	54	125
09:45 AM	0	1	74	1	1	61	138
Total	3	7	293	2	3	229	537
		1				1	
10:00 AM	3	2	68	2	4	54	133
10:15 AM	0	2	48	1	1	66	118
10:30 AM	0	5	65	3	0	62	135
10:45 AM	0	1	71	0	1	59	132
Total	3	10	252	6	6	241	518
				i			
11:00 AM	0	0	60	1	0	64	125
11:15 AM	2	0	56	0	2	62	122
11:30 AM	0	2	62	3	1	56	124
11:45 AM	1	1	66	1	2	70	141
Total	3	3	244	5	5	252	512
12:00 PM	1	1	67	0	2	69	140
12:15 PM	1	4	83	0	0	65	153
12:30 PM	0	2	74	1	1	50	128
12:45 PM	1	1	67	1	0	72	142
Total	3	8	291	2	3	256	563
01:00 PM	1	1	71	1	3	66	143
01:15 PM	1	0	56	3	2	87	149
01:30 PM	2	2	69	0	2	83	158
01:45 PM	2	1	64	3	1	73	144
Total	6	4	260	7	8	309	594
02:00 PM	1	0	76	0	1	69	147
02:15 PM	1	0	75	0	1	67	144
02:30 PM	0	2	98	0	3	78	181
02:45 PM	0	3	106	0	0	80	189
Total	2	5	355	0	5	294	661
				i			
03:00 PM	2	2	90	1	3	87	185
03:15 PM	3	2	73	2	2	84	166
03:30 PM	0	3	99	3	4	91	200
03:45 PM	1	3	95	2	2	92	195
Total	6	10	357	8	11	354	746
04:00 PM	2	3	93	0	0	109	207
04:15 PM	0	2	81	0	3	77	163
04:30 PM	0	0	102	0	2	115	219
04:45 PM	0	0	115	1	1	86	203
Total	2	5	391	1	6	387	792
. 514	_	- 1		- 1	-	1	
05:00 PM	0	1	102	0	0	96	199
05:15 PM	0	3	103	ő	2	92	200
33.131 WI	•	0 1	100	V 1	_	J=	200

N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 2

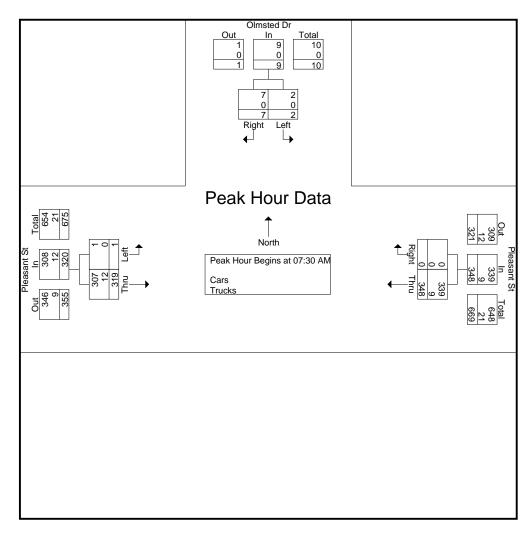
Groups Printed- Cars - Trucks

	Olmsted		Pleas		Pleasa	ant St	
	From No	orth	From	East	From '	West	
Start Time	Left	Right	Thru	Right	Left	Thru	Int. Total
05:30 PM	0	0	84	2	1	75	162
05:45 PM	11	0	69	1	0	76	147
Total	1	4	358	3	3	339	708
		1		1		1	
06:00 PM	3	2	61	1	6	73	146
06:15 PM	3	2	74	3	2	54	138
06:30 PM	1	2	49	1	1	55	109
06:45 PM	0	1	59	0	0	50	110
Total	7	7	243	5	9	232	503
0 17.11		1		40		0.4.0	7000
Grand Total		74	3695	42	63	3448	7363
Apprch %	35.7	64.3	98.9	1.1	1.8	98.2	
Total %	0.6	1	50.2	0.6	0.9	46.8	
Cars	41	73	3596	42	62	3348	7162
% Cars	100	98.6	97.3	100	98.4	97.1	97.3
Trucks	0	1	99	0	1	100	201
% Trucks	0	1.4	2.7	0	1.6	2.9	2.7

		Olmsted Dr From North		Pleasant St From East						
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	From West Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 07:00 AM to	09:45 AM - F	Peak 1 of 1							
Peak Hour for Entire Inte	rsection Begir	ns at 07:30 Al	M							
07:30 AM	1	5	6	79	0	79	0	93	93	178
07:45 AM	1	2	3	82	0	82	0	78	78	163
08:00 AM	0	0	0	89	0	89	0	81	81	170
08:15 AM	0	0	0	98	0	98	1	67	68	166
Total Volume	2	7	9	348	0	348	1	319	320	677
% App. Total	22.2	77.8		100	0		0.3	99.7		
PHF	.500	.350	.375	.888	.000	.888	.250	.858	.860	.951
Cars	2	7	9	339	0	339	1	307	308	656
% Cars	100	100	100	97.4	0	97.4	100	96.2	96.3	96.9
Trucks	0	0	0	9	0	9	0	12	12	21
% Trucks	0	0	0	2.6	0	2.6	0	3.8	3.8	3.1

N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 3

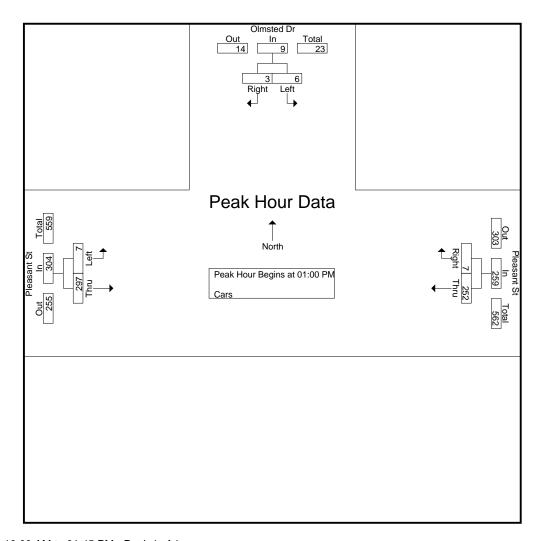


Peak Hour Analysis From 07:00 AM to 09:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak noul for Each Appl	<u>ioacii begiiis a</u>	มเ.							
	07:00 AM			08:15 AM			07:30 AM		J
+0 mins.	0	0	0	98	0	98	0	93	93
+15 mins.	1	2	3	73	1	74	0	78	78
+30 mins.	1	5	6	84	1	85	0	81	81
+45 mins.	1	2	3	91	1	92	1	67	68
Total Volume	3	9	12	346	3	349	1	319	320
% App. Total	25	75		99.1	0.9		0.3	99.7	
PHF	.750	.450	.500	.883	.750	.890	.250	.858	.860
Cars	3	9	12	332	3	335	1	307	308
% Cars	100	100	100	96	100	96	100	96.2	96.2
Trucks	0	0	0	14	0	14	0	12	12
% Trucks	0	0	0	4	0	4	0	3.8	3.8

N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 12



Peak Hour Analysis From 10:00 AM to 01:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

Cak Hour for Each Approach Bogino at:											
	10:00 AM			12:15 PM			01:00 PM				
+0 mins.	3	2	5	80	0	80	2	66	68		
+15 mins.	0	2	2	67	1	68	2	81	83		
+30 mins.	0	5	5	64	1	65	2	80	82		
+45 mins.	0	1	1	68	1	69	1	70	71		
Total Volume	3	10	13	279	3	282	7	297	304		
% App. Total	23.1	76.9		98.9	1.1		2.3	97.7			
PHF	.250	.500	.650	.872	.750	.881	.875	.917	.916		

N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 23

Groups	Printed-	Bikes	Peds	

	Ol	msted Dr	Dr Pleasant St			ntea- bikes		Pleasant St				
		om North		From East				rom West				
Stort Time			Dodo			Dodo	Left		Dodo	Exclu. Total	Inclu. Total	Int Total
Start Time	Left	Right	Peds	Thru	Right	Peds		Thru	Peds			Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	1	0	0	0	0	0	0	1	1
07:45 AM	0	0	0	1	0	0	0	1	0	0	2	2
Total	0	0	0	2	0	0	0	1	0	0	3	3
rotar ₁	O	O	O I	_	O	0	U	'	O	O	0	3
00.00 AM	0	0	0	0	0	0	0	0	ا م	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	1	0	0	0	2	0	0	3	3
08:45 AM	0	0	0	1	0	1	0	1	0	1	2	3_
Total	0	0	0	2	0	1	0	3	0	1	5	6
,			- 1			'			- 1			
09:00 AM	1	0	0	0	0	0	0	0	0	0	1	1
09:15 AM	0	1	ő	0	0	0	0	Ö	0	Ö	1	1
		•	1			1				_	•	1
09:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
09:45 AM	0	0	0	1	0	0	0	0	0	0	1_	1_
Total	1	1	0	1	0	0	0	0	0	0	3	3
10:00 AM	0	0	0	0	2	0	0	1	0	0	3	3
10:15 AM	0	1	0	1	0	0	0	0	0	0	2	2
10:30 AM	Ö	Ö	ő	Ö	Ö	ő	Ő	Ö	ő	Ö	0	0
10:45 AM	-	-	1	_		1	0	0		0	0	
	0	0	0	0	0	0			0			0
Total	0	1	0	1	2	0	0	1	0	0	5	5
44.00.444		•							ا م			
11:00 AM	0	0	0	0	0	0	0	1	0	0	1	1
11:15 AM	0	0	0	0	0	0	0	1	0	0	1	1
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0
11:45 AM	0	0	0	1	0	0	0	0	0	0	1	1
Total	0	0	0		0	0	0	2	0	0	3	3
Total	U	U	U	'	U	0	U	2	U	U	3	3
40:00 PM	0	0	0	4	0	0	0	0	ا م	0	4	
12:00 PM	0	0	0	1	0	0	0	0	0	0	1	1
12:15 PM	0	0	0	2	0	0	0	1	0	0	3	3
12:30 PM	0	0	0	1	0	0	0	0	0	0	1	1
12:45 PM	0	0	0	0	0	0	0	1	0	0	1	1
Total	0	0	0	4	0	0	0	2	0	0	6	6
rotar	O	O	O	7	O	0	U	_	0	O O	O	O .
01:00 PM	0	0	0	0	0	0	0	1	0	0	1	1
				_		- 1				-	1	<u> </u>
01:15 PM	0	0	0	1	0	0	0	0	0	0	1	1
01:30 PM	0	0	0	0	0	0	0	0	0	0	0	0
01:45 PM	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	1	0	0	0	1	0	0	2	2
•						·						
02:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
02:15 PM	Ö	Ö	0	2	0	0	0	2	0	0	4	4
	-	-	-		•	-	-		- 1	•	•	•
02:30 PM	0	0	0	0	0	0	0	3	0	0	3	3
02:45 PM	0	0	0	11	1	0	0	1	0	0	3	3_
Total	0	0	0	3	1	0	0	6	0	0	10	10
03:00 PM	0	0	0	1	0	0	0	1	0	0	2	2
03:15 PM	0	0	0	1	0	0	0	1	0	0	2	2
03:30 PM	1	Ö	ő	1	0	0	0	2	0	0	4	4
03:45 PM	0	0		-	0		0	2		0	2	
			0	0		0			0			2
Total	1	0	0	3	0	0	0	6	0	0	10	10
1			1						1			
04:00 PM	0	0	0	0	0	0	0	1	0	0	1	1
04:15 PM	0	0	0	1	0	0	0	1	0	0	2	2
04:30 PM	1	0	0	1	0	0	0	1	0	0	3	3
04:45 PM	0	Ö	ő	1	Ö	0	Ö	0	ő	0	1	1
Total	1	0	0	3	0	0	0	3	0	0	7	7
i otal	ı	U	O	3	U	O	U	3	J	U	,	,
05:00 PM	0	0	0	0	Λ	0	0	0	^	^	0	0
05:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
05:15 PM	0	0	0	0	0	0	0	0	0	0	0	0
05:30 PM	0	0	0	0	0	0	0	1	0	0	1	1
05:45 PM	0	0	0	0	0	0	0	1	0	0	1_	1_
Total	0	0	0	0	0	0	0	2	0	0	2	2
· Star	•	•	•	•	•	• 1	•	_	5	J	_	_

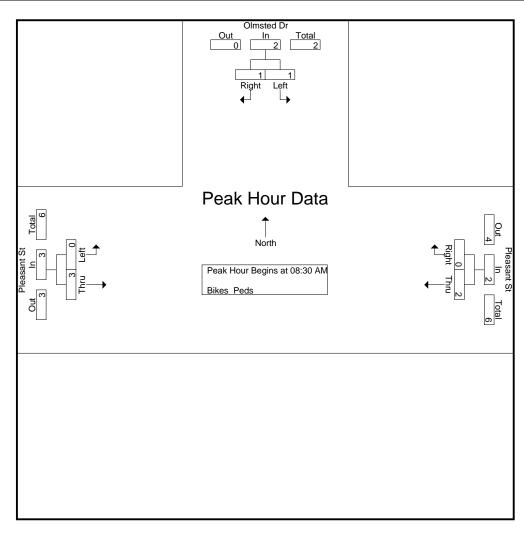
N/S Street : Olmsted Drive E/W Street : Pleasant Street City/State : Belmont, MA Weather : Clear

File Name: 80580003 Site Code : 80580003 Start Date : 11/10/2020 Page No : 24

Groups Printed-Bikes Peds

_	Croaper times Bikee 1 eas												
		0	Imsted Dr		Pleasant St			Pleasant St					
L		Fi	rom North		From East			From West					
	Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
	06:00 PM	0	0	0	0	0	0	0	0	0	0	0	0
	06:15 PM	0	0	0	0	0	0	0	0	0	0	0	0
	06:30 PM	0	0	0	0	0	0	0	0	0	0	0	0
_	06:45 PM	0	0	0	11	0	0	0	1	0	0	2	2
	Total	0	0	0	1	0	0	0	1	0	0	2	2
	Grand Total Apprch %	3 60	2 40	0	22 88	3 12	1	0	28 100	0	1	58	59
	Total %	5.2	3.4		37.9	5.2		0	48.3		1.7	98.3	

		Olmsted Dr From North			Pleasant St From East					
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 09:45 AM - Peak 1 of 1										
Peak Hour for Entire Inte	rsection Begin	ns at 08:30 Al	M .							
08:30 AM	0	0	0	1	0	1	0	2	2	3
08:45 AM	0	0	0	1	0	1	0	1	1	2
09:00 AM	1	0	1	0	0	0	0	0	0	1
09:15 AM	0	1	1	0	0	0	0	0	0	1
Total Volume	1	1	2	2	0	2	0	3	3	7
% App. Total	50	50		100	0		0	100		
PHF	.250	.250	.500	.500	.000	.500	.000	.375	.375	.583





Location: Pleasant Street Location: East of Olmsted Drive City/State: Belmont, MA

City/State: Belmont, MA 8058VL01

Start	11/10/202	F	:B	Hour	Totals	V	/B	Hour	Totals	Combin	ed Totals
Time	Tue	Morning	Afternoon								
12:00		5	69			1	64				
12:15		4	66			4	88				
12:30		1	52			2	76				
12:45		3	72	13	259	1	64	8	292	21	551
01:00		2	66			1	74				
01:15		2	90			4	61				
01:30		1	83			1	66				
01:45		1	77	6	316	1	70	7	271	13	587
02:00		4	67			0	72				
02:15		1	72			0	75				
02:30		2	78			1	99				
02:45		2	80	9	297	2	103	3	349	12	646
03:00		1	88			2	95				
03:15		2	88			3	75				
03:30		0	91			0	99				
03:45		4	94	7	361	4	97	9	366	16	727
04:00		2	113			1	90				
04:15		7	77			2	81				
04:30		3	113			5	97				
04:45		3	88	15	391	15	116	23	384	38	775
05:00		6 5	95			8	99				
05:15		5	92			9	106				
05:30		13	73			19	88				
05:45		22	77	46	337	26	65	62	358	108	695
06:00		17	76			41	66				
06:15		35	54			41	73				
06:30		54	56			57	56				
06:45		58	47	164	233	70	58	209	253	373	486
07:00		53	75			76	45				
07:15		62	49			71	37				
07:30		93	43			85	36				
07:45		81	35	289	202	84	52	316	170	605	372
08:00		79	45			86	31				
08:15		68	33			100	29				
08:30		72	25			72	30				
08:45		57	30	276	133	86	23	344	113	620	246
09:00		59	25			91	23				
09:15		54	33			62	22				
09:30		58	24			63	17				
09:45		62	18	233	100	74	22	290	84	523	184
10:00		57	21			69	16				
10:15		65	12			49	20				
10:30		61	18			68	17				
10:45		57	8	240	59	72	16	258	69	498	128
11:00		67	13			61	9				
11:15		65	13			57	11				
11:30		56	15			64	8				
11:45		70	5	258	46	68	4	250	32	508	78
Total		1556	2734			1779	2741			3335	5475
Percent		36.3%	63.7%			39.4%	60.6%			37.9%	62.1%

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA 8058VL01

Start	11/11/202	F	В	Hour	Totals	W	/B	Hour	Totals	Combin	ed Totals
Time	Wed	Morning	Afternoon								
12:00		3	87			2	76				
12:15		3	67			5	88				
12:30		0	82			5	82				
12:45		0	84	6	320	3	79	15	325	21	645
01:00		2	66			0	68				
01:15		1	64			4	82				
01:30		2	86			5	80				
01:45		0	73	5	289	1	92	10	322	15	611
02:00			66			1	90				
02:15		2	71			4	95				
02:30		3	100			0	86				
02:45		1	85	9	322	4	79	9	350	18	672
03:00		2	84			2	93				
03:15		1	90			0	101				
03:30		0	103			2	92				
03:45		0	77	3	354	1	106	5	392	8	746
04:00		3	89			1	71				
04:15		3	102			3	81				
04:30		0	80			3	84				
04:45		2	98	8	369	11	100	18	336	26	705
05:00		3	85			6	78				
05:15		1	93			11	78				
05:30		11	75			11	69				
05:45		15	66	30	319	23	73	51	298	81	617
06:00		21	73			28	75				
06:15		28	43			36	65				
06:30		39	56			50	49				
06:45		44	42	132	214	48	54	162	243	294	457
07:00		53	46			59	36				
07:15		46	29			63	36				
07:30		64	33			59	29				
07:45		60	44	223	152	53	33	234	134	457	286
08:00		60	40			64	36				
08:15		51	28			67	22				
08:30		51	27			61	28				
08:45		52	29	214	124	90	23	282	109	496	233
09:00		48	25			74	15				
09:15		44	28			63	23				
09:30		56	22			63	24				
09:45		59	16	207	91	77	20	277	82	484	173
10:00		69	17			71	23				
10:15		69	14			70	14				
10:30		75	14			60	19				
10:45		70	13	283	58	88	13	289	69	572	127
11:00		52	10			71	7				
11:15		80	12			76	7				
11:30		66	15			71	12				
11:45		91	8	289	45	86	2	304	28	593	73
Total		1409	2657			1656	2688			3065	5345
Percent		34.7%	65.3%			38.1%	61.9%			36.4%	63.6%
Grand		2965	5391			3435	5429			6400	10820
Total											
Percent		35.5%	64.5%			38.8%	61.2%			37.2%	62.8%

ADT ADT 8,610 AADT 8,610

Location: Pleasant Street Location: East of Olmsted Drive City/State: Belmont, MA

ADT

ADT 8,610

AADT 8,610

8058VL01

Start	11/9/2	020	Tu	ue	We	ed	Thu	ı	Fr	i	Sat		Sur	1	Week A	verage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB
12:00 AM	*	*	13	8	6	15	*	*	*	*	*	*	*	*	10	12
01:00	*	*	6	7	5	10	*	*	*	*	*	*	*	*	6	8
02:00	*	*	9	3	9	9	*	*	*	*	*	*	*	*	9	6
03:00	*	*	7	9	3	5	*	*	*	*	*	*	*	*	5	7
04:00	*	*	15	23	8	18	*	*	*	*	*	*	*	*	12	20
05:00	*	*	46	62	30	51	*	*	*	*	*	*	*	*	38	56
06:00	*	*	164	209	132	162	*	*	*	*	*	*	*	*	148	186
07:00	*	*	289	316	223	234	*	*	*	*	*	*	*	*	256	275
08:00	*	*	276	344	214	282	*	*	*	*	*	*	*	*	245	313
09:00	*	*	233	290	207	277	*	*	*	*	*	*	*	*	220	284
10:00	*	*	240	258	283	289	*	*	*	*	*	*	*	*	262	274
11:00	*	*	258	250	289	304	*	*	*	*	*	*	*	*	274	277
12:00 PM	*	*	259	292	320	325	*	*	*	*	*	*	*	*	290	308
01:00	*	*	316	271	289	322	*	*	*	*	*	*	*	*	302	296
02:00	*	*	297	349	322	350	*	*	*	*	*	*	*	*	310	350
03:00	*	*	361	366	354	392	*	*	*	*	*	*	*	*	358	379
04:00	*	*	391	384	369	336	*	*	*	*	*	*	*	*	380	360
05:00	*	*	337	358	319	298	*	*	*	*	*	*	*	*	328	328
06:00	*	*	233	253	214	243	*	*	*	*	*	*	*	*	224	248
07:00	*	*	202	170	152	134	*	*	*	*	*	*	*	*	177	152
08:00	*	*	133	113	124	109	*	*	*	*	*	*	*	*	128	111
09:00	*	*	100	84	91	82	*	*	*	*	*	*	*	*	96	83
10:00	*	*	59	69	58	69	*	*	*	*	*	*	*	*	58	69
11:00	*	*	46	32	45	28	*	*	*	*	*	*	*	*	46	30
Lane	0	0	4290	4520	4066	4344	0	0	0	0	0	0	0	0	4182	4432
Day	0		881	10	841	0	0		0		0		0		8614	.
AM Peak	-	-	07:00	08:00	11:00	11:00	-	-	-	-	-	-	-	-	11:00	08:00
Vol.	-	-	289	344	289	304	-	-	-	-	-	-	-	-	274	313
PM Peak	-	-	16:00	16:00	16:00	15:00	-	-	-	-	-	-	-	-	16:00	15:00
Vol.	-	-	391	384	369	392	-	-	-	-	=	-	-	-	380	379
Comb.																
Total	0	1	8	3810	8	410	()		0	0		()	86	614

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA

EΒ

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	4	5	4	0	0	0	0	0	0	0	0	13
01:00	0	0	0	0	5	1	0	0	0	0	0	0	0	0	6
02:00	0	0	0	3	4	2	0	0	0	0	0	0	0	0	9
03:00	0	0	1	2	0	4	0	0	0	0	0	0	0	0	7
04:00	0	0	1	4	8	0	2	0	0	0	0	0	0	0	15
05:00	0	0	1	19	18	8	0	0	0	0	0	0	0	0	46
06:00	0	2	12	40	85	24	1	0	0	0	0	0	0	0	164
07:00	0	2	17	67	144	56	2	1	0	0	0	0	0	0	289
08:00	0	0	16	83	122	49	6	0	0	0	0	0	0	0	276
09:00	0	2	10	69	112	39	1	0	0	0	0	0	0	0	233
10:00	0	2	17	83	117	16	5	0	0	0	0	0	0	0	240
11:00	0	1	20	100	109	28	0	0	0	0	0	0	0	0	258
12 PM	0	1	18	103	111	25	1	0	0	0	0	0	0	0	259
13:00	0	0	16	115	148	36	1	0	0	0	0	0	0	0	316
14:00	0	0	21	84	148	40	4	0	0	0	0	0	0	0	297
15:00	1	0	24	111	183	40	2	0	0	0	0	0	0	0	361
16:00	1	3	33	153	164	34	3	0	0	0	0	0	0	0	391
17:00	0	0	38	143	130	26	0	0	0	0	0	0	0	0	337
18:00	0	0	16	113	84	19	1	0	0	0	0	0	0	0	233
19:00	0	1	13	92	84	11	1	0	0	0	0	0	0	0	202
20:00	0	3	11	50	57	8	4	0	0	0	0	0	0	0	133
21:00	0	0	1	40	43	13	3	0	0	0	0	0	0	0	100
22:00	0	0	3	17	24	13	1	0	1	0	0	0	0	0	59
23:00	0	0	2	16	17	8	3	0	0	0	0	0	0	0	46
Total	2	17	291	1511	1922	504	41	1	1	0	0	0	0	0	4290

Daily

15th Percentile: 26 MPH
50th Percentile: 30 MPH
85th Percentile: 34 MPH
95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3433

 Percent in Pace:
 80.0%

 Number of Vehicles > 30 MPH:
 2469

 Percent of Vehicles > 30 MPH:
 57.6%

8058SP01

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA

EΒ

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	_
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	0	0	3	2	1	0	0	0	0	0	0	0	0	6
01:00	0	0	0	2	2	1	0	0	0	0	0	0	0	0	5
02:00	0	0	2	3	3	1	0	0	0	0	0	0	0	0	9
03:00	0	0	1	0	1	1	0	0	0	0	0	0	0	0	3
04:00	0	0	0	2	5	1	0	0	0	0	0	0	0	0	8
05:00	0	0	5	9	11	3	2	0	0	0	0	0	0	0	30
06:00	0	3	7	35	60	24	3	0	0	0	0	0	0	0	132
07:00	0	1	13	62	108	35	3	1	0	0	0	0	0	0	223
08:00	0	0	12	62	102	31	5	2	0	0	0	0	0	0	214
09:00	0	0	9	59	111	24	4	0	0	0	0	0	0	0	207
10:00	1	2	15	76	143	44	2	0	0	0	0	0	0	0	283
11:00	0	1	12	119	115	36	6	0	0	0	0	0	0	0	289
12 PM	1	4	17	109	138	46	4	1	0	0	0	0	0	0	320
13:00	0	1	9	91	153	33	2	0	0	0	0	0	0	0	289
14:00	0	3	30	113	138	36	2	0	0	0	0	0	0	0	322
15:00	0	1	41	133	133	42	4	0	0	0	0	0	0	0	354
16:00	0	5	38	122	167	34	3	0	0	0	0	0	0	0	369
17:00	0	0	30	122	135	32	0	0	0	0	0	0	0	0	319
18:00	0	1	18	81	96	17	0	1	0	0	0	0	0	0	214
19:00	0	1	6	48	79	18	0	0	0	0	0	0	0	0	152
20:00	0	0	3	47	64	10	0	0	0	0	0	0	0	0	124
21:00	0	1	2	12	61	13	2	0	0	0	0	0	0	0	91
22:00	0	1	1	23	25	8	0	0	0	0	0	0	0	0	58
23:00	0	11	3	15	17	9	0	0	0	0	0	00	0	0	45
Total	2	26	274	1348	1869	500	42	5	0	0	0	0	0	0	4066

Daily 15th Percentile: 26 MPH

50th Percentile: 31 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

59.4%

58.5%

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3217

 Percent in Pace:
 79.1%

 Number of Vehicles > 30 MPH:
 2416

Grand Total 4 43 565 2859 3791 1004 83 6 1 0 0 0 0 0 0 8356_

Overall 15th Percentile : 26 MPH 50th Percentile : 30 MPH

Percent of Vehicles > 30 MPH:

Percent of Vehicles > 30 MPH:

85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6650

 Percent in Pace:
 79.6%

 Number of Vehicles > 30 MPH:
 4885

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA

WB

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	3	3	0	2	0	0	0	0	0	0	0	8
01:00	0	0	0	0	4	2	1	0	0	0	0	0	0	0	7
02:00	0	0	0	0	2	1	0	0	0	0	0	0	0	0	3
03:00	0	2	1	2	2	2	0	0	0	0	0	0	0	0	9
04:00	0	1	3	4	8	6	1	0	0	0	0	0	0	0	23
05:00	0	0	2	11	27	20	1	1	0	0	0	0	0	0	62
06:00	3	3	29	68	85	19	1	1	0	0	0	0	0	0	209
07:00	1	2	30	100	130	45	6	0	1	0	0	0	0	1	316
08:00	2	1	34	95	147	62	1	0	0	0	0	0	0	2	344
09:00	2	0	15	79	124	65	4	0	0	0	0	0	1	0	290
10:00	2	4	26	70	113	36	5	1	0	0	0	0	0	1	258
11:00	1	2	25	73	111	34	4	0	0	0	0	0	0	0	250
12 PM	3	1	28	85	122	50	2	0	0	0	0	0	0	1	292
13:00	1	7	21	84	120	31	7	0	0	0	0	0	0	0	271
14:00	1	2	26	106	159	49	4	1	1	0	0	0	0	0	349
15:00	1	2	40	110	154	54	5	0	0	0	0	0	0	0	366
16:00	1	4	62	138	139	31	7	2	0	0	0	0	0	0	384
17:00	0	4	40	172	115	26	1	0	0	0	0	0	0	0	358
18:00	1	3	30	113	87	16	2	1	0	0	0	0	0	0	253
19:00	0	1	22	71	58	17	1	0	0	0	0	0	0	0	170
20:00	1	3	21	40	34	12	2	0	0	0	0	0	0	0	113
21:00	1	0	9	30	33	10	1	0	0	0	0	0	0	0	84
22:00	0	0	4	30	27	7	0	0	1	0	0	0	0	0	69
23:00	0	0	0	12	12	4	4	0	0	0	0	0	0	0	32
Total	21	42	468	1496	1816	599	62	7	3	0	0	0	1	5	4520

Daily

 15th Percentile :
 25 MPH

 50th Percentile :
 30 MPH

 85th Percentile :
 34 MPH

 95th Percentile :
 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3312

 Percent in Pace:
 73.3%

 Number of Vehicles > 30 MPH:
 2493

 Percent of Vehicles > 30 MPH:
 55.2%

8058SP01

Accurate Counts

978-664-2565

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA 8058SP01

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	1	1	3	9	1	0	0	0	0	0	0	0	0	15
01:00	0	0	3	0	3	3	1	0	0	0	0	0	0	0	10
02:00	0	0	0	4	3	2	0	0	0	0	0	0	0	0	9
03:00	0	1	1	2	0	1	0	0	0	0	0	0	0	0	5
04:00	0	0	0	2	8	8	0	0	0	0	0	0	0	0	18
05:00	1	0	2	16	19	9	1	0	1	0	0	0	0	2	51
06:00	0	3	19	46	62	26	5	0	0	0	0	0	0	1	162
07:00	4	2	30	70	86	32	8	0	0	0	0	0	0	2	234
08:00	4	0	14	99	128	34	1	1	0	0	0	0	0	1	282
09:00	0	1	12	84	127	48	5	0	0	0	0	0	0	0	277
10:00	3	2	23	104	123	33	1	0	0	0	0	0	0	0	289
11:00	3	5	26	92	133	39	5	0	1	0	0	0	0	0	304
12 PM	2	1	27	97	140	54	4	0	0	0	0	0	0	0	325
13:00	3	3	27	107	142	36	4	0	0	0	0	0	0	0	322
14:00	3	9	43	134	129	28	3	1	0	0	0	0	0	0	350
15:00	1	3	43	152	145	45	2	1	0	0	0	0	0	0	392
16:00	3	2	46	155	106	23	1	0	0	0	0	0	0	0	336
17:00	0	3	42	139	89	24	1	0	0	0	0	0	0	0	298
18:00	0	1	31	88	107	13	3	0	0	0	0	0	0	0	243
19:00	1	4	23	42	52	10	2	0	0	0	0	0	0	0	134
20:00	2	4	6	47	38	12	0	0	0	0	0	0	0	0	109
21:00	0	1	8	30	31	10	2	0	0	0	0	0	0	0	82
22:00	0	2	4	36	22	4	1	0	0	0	0	0	0	0	69
23:00	0	0	3	10	12	3	0	0	0	0	0	0	0	0	28
Total	30	48	434	1559	1714	498	50	3	2	0	0	0	0	6	4344

Daily

15th Percentile : 25 MPH 50th Percentile : 30 MPH

85th Percentile: 34 MPH 95th Percentile: 38 MPH

Mean Speed(Average): 31 MPH 10 MPH Pace Speed: 26-35 MPH Number in Pace: 3273

Percent in Pace: 75.3%
Number of Vehicles > 30 MPH: 2273
Percent of Vehicles > 30 MPH: 52.3%

Grand Total 51 90 902 3055 3530 1097 112 10 5 0 0 0 1 11 8864_

Overall 15th Percentile: 25 MPH

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6585

 Percent in Pace:
 74.3%

Number of Vehicles > 30 MPH: 4766
Percent of Vehicles > 30 MPH: 53.8%

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA 8058SP01

EB, WB

,															
Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	7	8	4	2	0	0	0	0	0	0	0	21
01:00	0	0	0	0	9	3	1	0	0	0	0	0	0	0	13
02:00	0	0	0	3	6	3	0	0	0	0	0	0	0	0	12
03:00	0	2	2	4	2	6	0	0	0	0	0	0	0	0	16
04:00	0	1	4	8	16	6	3	0	0	0	0	0	0	0	38
05:00	0	0	3	30	45	28	1	1	0	0	0	0	0	0	108
06:00	3	5	41	108	170	43	2	1	0	0	0	0	0	0	373
07:00	1	4	47	167	274	101	8	1	1	0	0	0	0	1	605
08:00	2	1	50	178	269	111	7	0	0	0	0	0	0	2	620
09:00	2	2	25	148	236	104	5	0	0	0	0	0	1	0	523
10:00	2	6	43	153	230	52	10	1	0	0	0	0	0	1	498
11:00	1	3	45	173	220	62	4	0	0	0	0	0	0	0	508
12 PM	3	2	46	188	233	75	3	0	0	0	0	0	0	1	551
13:00	1	7	37	199	268	67	8	0	0	0	0	0	0	0	587
14:00	1	2	47	190	307	89	8	1	1	0	0	0	0	0	646
15:00	2	2	64	221	337	94	7	0	0	0	0	0	0	0	727
16:00	2	7	95	291	303	65	10	2	0	0	0	0	0	0	775
17:00	0	4	78	315	245	52	1	0	0	0	0	0	0	0	695
18:00	1	3	46	226	171	35	3	1	0	0	0	0	0	0	486
19:00	0	2	35	163	142	28	2	0	0	0	0	0	0	0	372
20:00	1	6	32	90	91	20	6	0	0	0	0	0	0	0	246
21:00	1	0	10	70	76	23	4	0	0	0	0	0	0	0	184
22:00	0	0	7	47	51	20	1	0	2	0	0	0	0	0	128
23:00	0	0	2	28	29	12	7	0	0	0	0	0	0	0	78
Total	23	59	759	3007	3738	1103	103	8	4	0	0	0	1	5	8810

Daily 15th Percentile: 25 MPH

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6745

 Percent in Pace:
 76.6%

 Number of Vehicles > 30 MPH:
 4962

 Percent of Vehicles > 30 MPH:
 56.3%

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA

EB,	WB
-----	----

,															
Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	1	1	6	11	2	0	0	0	0	0	0	0	0	21
01:00	0	0	3	2	5	4	1	0	0	0	0	0	0	0	15
02:00	0	0	2	7	6	3	0	0	0	0	0	0	0	0	18
03:00	0	1	2	2	1	2	0	0	0	0	0	0	0	0	8
04:00	0	0	0	4	13	9	0	0	0	0	0	0	0	0	26
05:00	1	0	7	25	30	12	3	0	1	0	0	0	0	2	81
06:00	0	6	26	81	122	50	8	0	0	0	0	0	0	1	294
07:00	4	3	43	132	194	67	11	1	0	0	0	0	0	2	457
08:00	4	0	26	161	230	65	6	3	0	0	0	0	0	1	496
09:00	0	1	21	143	238	72	9	0	0	0	0	0	0	0	484
10:00	4	4	38	180	266	77	3	0	0	0	0	0	0	0	572
11:00	3	6	38	211	248	75	11	0	1	0	0	0	0	0	593
12 PM	3	5	44	206	278	100	8	1	0	0	0	0	0	0	645
13:00	3	4	36	198	295	69	6	0	0	0	0	0	0	0	611
14:00	3	12	73	247	267	64	5	1	0	0	0	0	0	0	672
15:00	1	4	84	285	278	87	6	1	0	0	0	0	0	0	746
16:00	3	7	84	277	273	57	4	0	0	0	0	0	0	0	705
17:00	0	3	72	261	224	56	1	0	0	0	0	0	0	0	617
18:00	0	2	49	169	203	30	3	1	0	0	0	0	0	0	457
19:00	1	5	29	90	131	28	2	0	0	0	0	0	0	0	286
20:00	2	4	9	94	102	22	0	0	0	0	0	0	0	0	233
21:00	0	2	10	42	92	23	4	0	0	0	0	0	0	0	173
22:00	0	3	5	59	47	12	1	0	0	0	0	0	0	0	127
23:00	0	1	6	25	29	12	0	0	0	0	0	0	0	0	73
Total	32	74	708	2907	3583	998	92	8	2	0	0	0	0	6	8410

Daily 15th Percentile: 25 MPH

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6490

 Percent in Pace:
 77.2%

 Number of Vehicles > 30 MPH:
 4689

 Percent of Vehicles > 30 MPH:
 55.8%

Grand Total 55 133 1467 5914 7321 2101 195 16 6 0 0 0 1 11 17220

Overall 15th Percentile: 25 MPH

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 13235

 Percent in Pace:
 76.9%

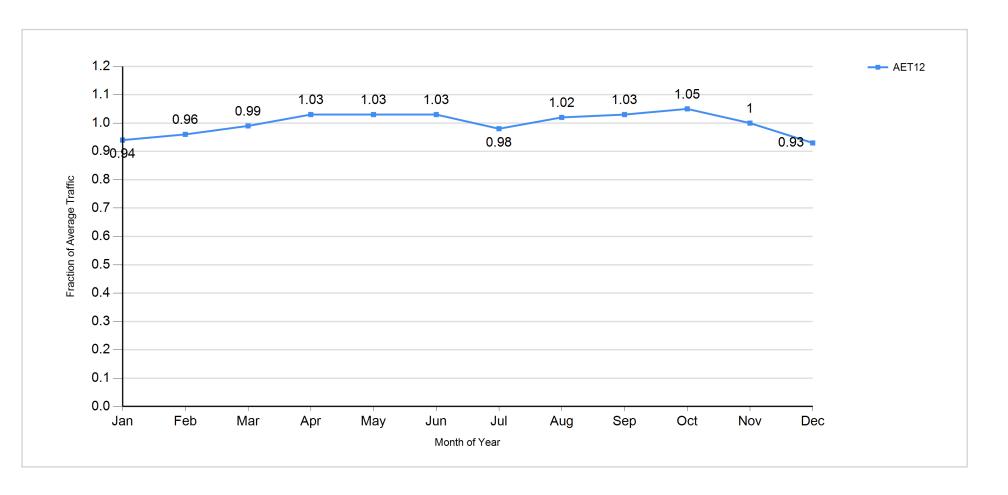
 Number of Vehicles > 30 MPH:
 9651

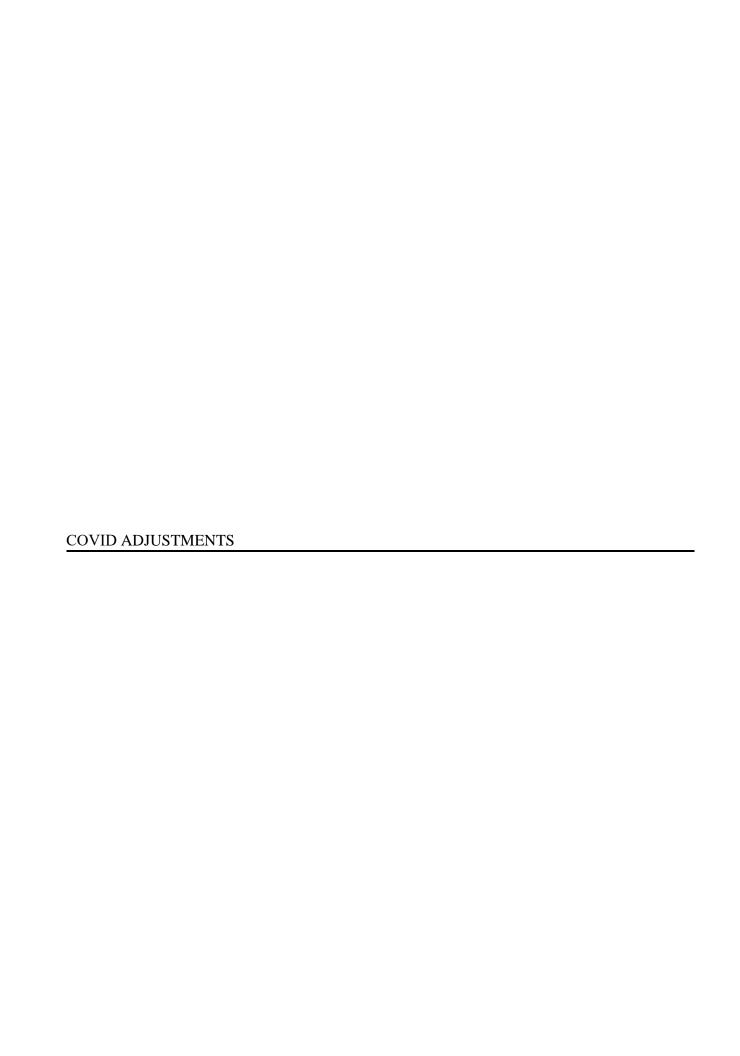
 Percent of Vehicles > 30 MPH:
 56.0%





Traffic Pattern by Month for 1/1/2019 - 12/31/2019





Comment 1: N/S Street : Trapelo Road
Comment 2: E/W Street : Mill Street
Comment 3: City/State : Belmont, MA
Comment 4: Weather : Clear

	Comment 4.	veaulei . C		Wednesday, A	pril 11, 2018 (6:00 AM										Tuesday,	November, 10	2020					
		Mill St		1	rapelo Rd		1	Trapelo Rd						Mill St			Trapelo Rd		T	rapelo Rd			
	F	rom North			From East		F	From West					F	rom North			From East		F	rom West			
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Total	Peak	Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Total	
7:00:00 AM	30	124	0	108	76	0	149	34	0	521		7:00:00 AM	56	19	0	75	72	0	22	89	0	333	
7:15:00 AM	31	125	0	136	131	0	150	35	0	608		7:15:00 AM	93	24	0	70	77	0	30	103	0	397	
7:30:00 AM	43	105	0	131	141	0	170	29	0	619		7:30:00 AM	89	27	0	81	81	0	27	131	0	436	
7:45:00 AM	25	136	0	175	146	0	163	25	0	670	2418	7:45:00 AM	91	23	0	89	97	0	27	104	0	431	1597
8:00:00 AM	50	150	0	162	147	0	150	34	0	693	2590	8:00:00 AM	89	22	0	105	103	0	18	114	0	451	1715
8:15:00 AM	53	149	0	129	169	0	132	36	0	668	2650	8:15:00 AM	94	36	0	87	110	0	21	109	0	457	1775
8:30:00 AM	69	142	0	152	150	0	138	19	0	670	2701	8:30:00 AM	91	26	0	91	113	0	15	102	0	438	1777
8:45:00 AM	69	145	0	142	154	0	134	29	0	673	2704	8:45:00 AM	74	24	0	92	91	0	30	100	0	411	1757
	370	1076	0	1135	1114	0	1186	241	0	5122	2704		677	201	0	690	744	0	190	852	0	3354	1777
								Seasona	al Adj 1 %	5173	2731								Co	omparisson		1.57	1.57
							202	20 Adjuste	d 1% year	5277	2786								Say	COVID A	dj į	1.5	7
4:00:00 PM	52	169	0	121	166	0	123	47	0	678		4:00:00 PM	88	29	0	113	90	0	44	152	0	516	
4:15:00 PM	41	136	0	121	177	0	134	23	0	632		4:15:00 PM	109	39	0	123	92	0	24	113	0	500	
4:30:00 PM	37	148	0	93	177	0	166	35	0	656		4:30:00 PM	114	33	0	123	89	0	27	132	0	518	
4:45:00 PM	40	151	0	91	183	0	150	32	0	647	2613		101	39	0	148	76	0	33	123	0	520	2054
5:00:00 PM	55	128	0	91	170	0	156	36	0	636	2571	5:00:00 PM	95	21	0	143	70	0	22	132	0	483	2034
5:15:00 PM	46	137	0	130	175	0	145	43	0	676	2615		102	34	0	168	77	0	22	107	0	510	2021
5:30:00 PM	33	140	0	100	173	0	163	35	0	661	2620		85	20	0	97	71	0	26	99	0	398	1911
5:45:00 PM	44	179	0	77	206	0	147	28	0	681	2654	5:45:00 PM	57	32	0	103	52	0	28	103	0	375	1766
3.43.00 i Wi	348	1188	0	824	1444	0	1184	279	0	5267	2654	3.43.00 T W	751	247	0		617	0	226	961	0	3820	2054
	340	1100	U	024	1444	U	1104		al Adj 1 %	5320	2681		731	241	U	1010	017	U I		mparisson	U	1.42	1.33
							201	20 Adjuste	•		2735							4		/ COVID A		1.42	
							202												Jaj		<u> </u>		
								TOTA	AL Peaks	10704	5521										L Peaks	7174	3831
																		Į.		omparisson		1.49	1.44
																			Say	/ COVID Ad	dj i	1.5	J

Comment 1: N/S Street : Pleasant Street
Comment 2: E/W Street : Trapelo Road
Comment 3: City/State : Belmont, MA
Comment 4: Weather : Clear

	Comment 4:	weather : 0	Jiear																				
_				Thrusday, November 14 ,2019								_					November, 10						
		Pleasant St			Trapelo Ro			Trapelo Rd					F	Pleasant St			Trapelo Rd		1	rapelo Rd			
		From North			From East		1	From West					F	rom North			From East		F	rom West			
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds			Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds		
7:00:00 AM											-	7:00:00 AM	32	37	0	116	14	0	37	111	0		
7:15:00 AM												7:15:00 AM	37	36	0	101	17	0	46	146	0		
7:30:00 AM												7:30:00 AM	46	41	0	126	33	0	62	157	0		
7:45:00 AM												7:45:00 AM	43	41	0	145	34	0	40	160	0		
8:00:00 AM												8:00:00 AM	35	54	0	157	37	0	44	145	0		
8:15:00 AM												8:15:00 AM	46	50	0	145	27	0	41	169	0		
8:30:00 AM												8:30:00 AM	27	45	0	157	26	0	41	151	0		
8:45:00 AM												8:45:00 AM	34	54	0	128	19	0	38	155	0		
4:00:00 PM	52		0			0	68	212	0	673		4:00:00 PM	35	63	0	158	32	0	73	168	0	529	
4:15:00 PM	50	70	0	218	56	0	69	236	0	699		4:15:00 PM	36	48	0	157	35	0	48	168	0	492	
4:30:00 PM	58	60	0	193	75	0	78	233	0	697		4:30:00 PM	42	54	0	166	45	0	67	186	0	560	
4:45:00 PM	46		0	222	55	0	78	234	0	698	2767	4:45:00 PM	41	76	0	152	33	0	55	167	0	524	2105
5:00:00 PM	52	57	0	180	53	0	79	242	0	663	2757	5:00:00 PM	24	70	0	161	26	0	71	161	0	513	2089
5:15:00 PM	59	67	0	166	78	0	79	227	0	676	2734	5:15:00 PM	44	66	0	160	41	0	54	173	0	538	2135
5:30:00 PM	73	53	0	172	65	0	71	194	0	628	2665	5:30:00 PM	32	53	0	131	26	0	51	138	0	431	2006
5:45:00 PM	57	53	0	182	52	0	65	221	0	630	2597	5:45:00 PM	31	37	0	103	25	0	50	109	0	355	1837
										5364	2767											3942	2135
								Season	al Adj 1 %	5418	2795							Ī	C	omparisson		1.40	1.34
							20:	20 Adjuste	d 1% year	5527	2851								Sa	y COVID Ad	dj	1.40	0

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

End Time: Class:	6:00 PM				Cars and	Heavy Ve	hicles (Co	mbined)					
		Mill St	reet		Т	rapelo Road	d (Route 60)		Tr	apelo Road	(Route 60)		
		from N				from				from W			
6:00 AM	Right 14	Left 50	U-Turn 0	Total 64	Right 51	Thru 37	U-Turn 0	Total 88	Thru 57	Left 8	U-Turn 0	Total 65	Total 217
6:15 AM	13	71	0	84	55	53	0	108	66	19	0	85	277
6:30 AM	21	88	0	109	103	55	0	158	129	18	0	147	414
6:45 AM Total	20 68	121 330	0	141 398	114 323	85 230	0	199 553	137 389	20 65	0	157 454	497 1405
7:00 AM	30	124	0	154	108	76	0	184		34	0	183	521
7:15 AM	31	125	0	156	136	131	0	267	150	35	0	185	608
7:30 AM	43	105	0	148	131	141	0	272	170	29	0	199	619
7:45 AM Total	25 129	136 490	0	161 619	175 550	146 494	0	321 1044	163 632	25 123	0	188 755	670 2418
8:00 AM	50	150	0	200	162	147	0	309	150	34	0	184	693
8:15 AM	53	149	0	202	129	169	0	298		36	0	168	668
8:30 AM	69	142	0	211	152	150	0	302	138	19	0	157	670
8:45 AM Total	69 241	145 586	0	214 827	142 585	154 620	0	296 1205	134 554	29 118	0	163 672	673 2704
9:00 AM	68	125	0	193	118	155	0	273	102	24	0	126	592
9:15 AM	50	119	0	169	93	152	0	245	101	17	0	118	532
9:30 AM	42	105	0	147	94	100	0	194	103	26	0	129	470
9:45 AM Total	57 217	107 456	0	164 673	123 428	103 510	0	226 938	108 414	26 93	0	134 507	524 2118
10:00 AM	44	90	0	134	66	115	0	181	109	23	0	132	447
10:15 AM	31	86	0	117	56	79	0	135	126	24	0	150	402
10:30 AM	29	64	0	93	94	100	0	194	97	24	0	121	408
10:45 AM Total	31 135	76 316	0	107 451	75 291	117 411	0	192 702	97 429	24 95	0	121 524	420 1677
11:00 AM	29	68	0	97	69	92	0	161	92	19	0	111	369
11:15 AM	36	80	0	116	74	94	0	168	107	21	0	128	412
11:30 AM	24	106	0	130	84	111	0	195		28	0	142	467
11:45 AM Total	32 121	88 342	0	120 463	86 313	102 399	0	188 712	122 435	39 107	0	161 542	469 1717
12:00 PM	35	85	0	120	81	104	0	185	100	24	0	124	429
12:15 PM	33	87	0	120	81	118	0	199	98	20	0	118	437
12:30 PM 12:45 PM	34 32	68 77	0	102 109	90 85	103 109	0 0	193 194	105 134	25 30	0 0	130 164	425 467
Total	134	317	0	451	337	434	0	771	437	99	0	536	1758
1:00 PM	30	86	0	116	74	107	0	181	120	32	0	152	449
1:15 PM	24	72	0	96	84	118	0	202	118	34	0	152	450
1:30 PM 1:45 PM	29 18	72 95	0	101 113	80 120	118 137	0 0	198 257	113 109	27 28	0 0	140 137	439 507
Total	101	325	0	426	358	480	0	838		121	0	581	1845
2:00 PM	40	88	0	128	97	112	0	209	136	28	0	164	501
2:15 PM	25	84	0	109	100	111	0	211	132	39	0	171	491
2:30 PM 2:45 PM	27 43	102 97	0	129 140	101 116	105 107	0	206 223	129 122	46 42	0	175 164	510 527
Total	135	371	0	506	414	435	0	849		155	0	674	2029
3:00 PM	38	133	0	171	117	129	0	246	129	32	0	161	578
3:15 PM	52	129	0	181	117	159	0	276	138	35	0	173	630
3:30 PM 3:45 PM	32 41	151 138	0	183 179	104 110	155 131	0 0	259 241	184 99	23 27	0 0	207 126	649 546
Total	163	551	0	714	448	574	0	1022		117	0	667	2403
4:00 PM	52	169	0	221	121	166	0	287	123	47	0	170	678
4:15 PM	41	136	0	177	121	177	0	298		23	0	157	632
4:30 PM 4:45 PM	37 40	148 151	0	185 191	93 91	177 183	0 0	270 274	166 150	35 32	0 0	201 182	656 647
Total	170	604	0	774	426	703	0	1129	573	137	0	710	2613
5:00 PM	55	128	0	183	91	170	0	261	156	36	0	192	636
5:15 PM	46	137	0	183	130	175	0	305	145	43	0	188	676
5:30 PM 5:45 PM	33 44	140 179	0	173 223	100 77	190 206	0 0	290 283	163 147	35 28	0 0	198 175	661 681
Total	178	584	0	762	398	741	0	1139	611	142	0	753	2654
Grand Total	1792	5272	0	7064	4871	6031	0	10902		1372	0	7375	25341
Approach % Total %	25.4	74.6	0.0	27.0	44.7	55.3	0.0	42.0	81.4	18.6	0.0	20.1	
Exiting Leg Total	7.1	20.8	0.0	27.9 6243	19.2	23.8	0.0	43.0 11275	23.7	5.4	0.0	29.1 7823	25341
Cars	1749	5135	0	6884	4727	5823	0	10550		1320	0	7131	24565
% Cars	97.6	97.4	0.0	97.5	97.0	96.6	0.0	96.8	96.8	96.2	0.0	96.7	96.9
Exiting Leg Total Heavy Vehicles	43	137	0	6047 180	144	208	0	10946 352	192	52	0	7572 244	24565 776
% Heavy Vehicles	2.4	2.6	0.0	2.5	3.0	3.4	0.0	3.2	3.2	3.8	0.0	3.3	3.1
Exiting Leg Total				196				329				251	776

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Class:

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM
End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

Cars and Heavy Vehicles (Combined)

	Mill	Street			Trapelo Roa	d (Route 60)	-	Trapelo Roa	d (Route 60)	
	from	North			from	East			from	West		
Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	Total

AM Peak Hour Analysis from 06:00 AM to 10:00 AM begins at:

8:00 AM		Mill S	treet		T	rapelo Road	d (Route 60)		Т	rapelo Roa	d (Route 60)		
		from I	North			from	East			from	West		
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	Total
8:00 AM	50	150	0	200	162	147	0	309	150	34	0	184	693
8:15 AM	53	149	0	202	129	169	0	298	132	36	0	168	668
8:30 AM	69	142	0	211	152	150	0	302	138	19	0	157	670
8:45 AM	69	145	0	214	142	154	0	296	134	29	0	163	673
Total Volume	241	586	0	827	585	620	0	1205	554	118	0	672	2704
% Approach Total	29.1	70.9	0.0		48.5	51.5	0.0		82.4	17.6	0.0		
PHF	0.873	0.977	0.000	0.966	0.903	0.917	0.000	0.975	0.923	0.819	0.000	0.913	0.975
Cars	235	573	0	808	567	610	0	1177	523	114	0	637	2622
Cars %	97.5	97.8	0.0	97.7	96.9	98.4	0.0	97.7	94.4	96.6	0.0	94.8	97.0
Heavy Vehicles	6	13	0	19	18	10	0	28	31	4	0	35	82
Heavy Vehicles %	2.5	2.2	0.0	2.3	3.1	1.6	0.0	2.3	5.6	3.4	0.0	5.2	3.0
Cars Enter Leg	235	573	0	808	567	610	0	1177	523	114	0	637	2622
Heavy Enter Leg	6	13	0	19	18	10	0	28	31	4	0	35	82
Total Entering Leg	241	586	0	827	585	620	0	1205	554	118	0	672	2704
Cars Exiting Leg				681				1096				845	2622
Heavy Exiting Leg				22				44				16	82
Total Exiting Leg		<u> </u>		703				1140		·		861	2704

MidDay Peak Hour Analysis from 10:00 AM to 2:00 PM begins at:

1:45 PM		Mill S	treet		Т	rapelo Road	d (Route 60)		Т	rapelo Road	d (Route 60)		
		from I	North			from	East			from	West		
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	Total
1:45 PM	18	95	0	113	120	137	0	257	109	28	0	137	507
2:00 PM	40	88	0	128	97	112	0	209	136	28	0	164	501
2:15 PM	25	84	0	109	100	111	0	211	132	39	0	171	491
2:30 PM	27	102	0	129	101	105	0	206	129	46	0	175	510
Total Volume	110	369	0	479	418	465	0	883	506	141	0	647	2009
% Approach Total	23.0	77.0	0.0		47.3	52.7	0.0		78.2	21.8	0.0		
PHF	0.688	0.904	0.000	0.928	0.871	0.849	0.000	0.859	0.930	0.766	0.000	0.924	0.985
Cars	105	353	0	458	394	441	0	835	491	134	0	625	1918
Cars %	95.5	95.7	0.0	95.6	94.3	94.8	0.0	94.6	97.0	95.0	0.0	96.6	95.5
Heavy Vehicles	5	16	0	21	24	24	0	48	15	7	0	22	91
Heavy Vehicles %	4.5	4.3	0.0	4.4	5.7	5.2	0.0	5.4	3.0	5.0	0.0	3.4	4.5
Cars Enter Leg	105	353	0	458	394	441	0	835	491	134	0	625	1918
Heavy Enter Leg	5	16	0	21	24	24	0	48	15	7	0	22	91
Total Entering Leg	110	369	0	479	418	465	0	883	506	141	0	647	2009
Cars Exiting Leg				528				844				546	1918
Heavy Exiting Leg				31				31				29	91
Total Exiting Leg				559				875				575	2009

PM Peak Hour Analysis from 2:00 PM to 06:00 PM begins at:

5:00 PM		Mill S	treet		Т	rapelo Road	l (Route 60)		Т	rapelo Road	d (Route 60)		
		from I	North			from	East			from	West		
	Right	Left	U-Turn	Total	Right	Thru	U-Turn	Total	Thru	Left	U-Turn	Total	Total
5:00 PM	55	128	0	183	91	170	0	261	156	36	0	192	636
5:15 PM	46	137	0	183	130	175	0	305	145	43	0	188	676
5:30 PM	33	140	0	173	100	190	0	290	163	35	0	198	661
5:45 PM	44	179	0	223	77	206	0	283	147	28	0	175	681
Total Volume	178	584	0	762	398	741	0	1139	611	142	0	753	2654
% Approach Total	23.4	76.6	0.0		34.9	65.1	0.0		81.1	18.9	0.0		
PHF	0.809	0.816	0.000	0.854	0.765	0.899	0.000	0.934	0.937	0.826	0.000	0.951	0.974
Cars	176	579	0	755	397	729	0	1126	604	140	0	744	2625
Cars %	98.9	99.1	0.0	99.1	99.7	98.4	0.0	98.9	98.9	98.6	0.0	98.8	98.9
Heavy Vehicles	2	5	0	7	1	12	0	13	7	2	0	9	29
Heavy Vehicles %	1.1	0.9	0.0	0.9	0.3	1.6	0.0	1.1	1.1	1.4	0.0	1.2	1.1
Cars Enter Leg	176	579	0	755	397	729	0	1126	604	140	0	744	2625
Heavy Enter Leg	2	5	0	7	1	12	0	13	7	2	0	9	29
Total Entering Leg	178	584	0	762	398	741	0	1139	611	142	0	753	2654
Cars Exiting Leg				537				1183				905	2625
Heavy Exiting Leg				3				12				14	29
Total Exiting Leg				540				1195				919	2654

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM
End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

Bicycles (on Roadway and Crosswalks)

Class:							Bicycles	(on F	Roadwa	ay and	Crossw	valks)	1						
			Mill St	reet				Trap	elo Road	l (Route	60)			Trape	lo Road	(Route	60)		
			from N	Iorth					from	East					from V	Vest			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	1	1
6:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
7:00 AM	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	2
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	1	1
7:30 AM	0	1	0	0	0	1	1	0	0	0	0	1		0	0	0	0	0	2
7:45 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
Total	0	1	0	0	0	1	1	3	0	0	0	4	1	0	0	0	0	1	6
8:00 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
8:15 AM	1	0	0	0	0	1	1	1	0	0	0	2		0	0	0	0	0	3
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
Total	1	0	0	0	0	1	1	3	0	0	0	4	0	0	0	0	0	0	5
9:00 AM	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	2
9:15 AM	0	1	0	0	0	1	0	0	0	0	0	0		0	0	0	0	0	1
9:30 AM	0	0	0	0	0	0	1	1	0	0	0	2		0	0	0	0	2	4
9:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
Total	0	1	0	0	0	1	1	3	0	0	0	4	2	0	0	0	0	2	7
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	1	0	0	0	0	1		0	0	0	0	1	2
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
Total	0	0	0	0	0	0	1	0	0	0	0	1	1	0	0	0	0	1	2
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	1	0	0	0	1	0	1	0	0	0	1		0	0	0	0	0	2
11:30 AM	0	1	0	0	0	1	0	0	0	0	0	0		0	0	0	0	0	1
11:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
Total	0	2	0	0	0	2	0	1	0	0	0	1	0	0	0	0	0	0	3
12:00 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	1	0	0	0	1	0	0	0	0	0	0		0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
12:45 PM Total	0	0	0	0	0	0	3	0	0	0	0	3		0	0	0	0	0	3
TOTAL	0	2	0	0	0	2	3	0	0	0	0	3	0	0	0	0	0	0	5
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
1:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
1:45 PM Total	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
		U	U	U	U		U	U			U			U	U	U	U		
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	1	1
2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
2:30 PM	0 0	0	0	0	0 0	0	0	0	0 0	0	0 0	0		0 0	0	0	0	0	0
2:45 PM Total	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	1	1
													-						J.
3:00 PM	0	0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	
3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
3:30 PM 3:45 PM	0 0	0	0 0	0	0 1	0	0	0	0 0	0	0 0	0		0	0	0	0	0 1	0
Total	0	0	0	0	1	1 1	0	0	0	0	0	0		0	0	0	0	1	2
													-						
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0	0	0	1	1
4:30 PM 4:45 PM	0	0	0 0	0	0	0	0	0	0 0	0	0 0	0		0	0	0	0 0	0 1	0 1
Total	0	0	0	0	0	0		0	0	0	0	0		0	0	0	0	2	2
													-						
5:00 PM	0	0	0	0	0	0		1	0	0	0	3		0	0	0	0	1	4
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		1	0	0	0	2	2
5:30 PM 5:45 PM	0 0	0 0	0 0	0	0 0	0	1	0	0 0	0	0 0	1		0 0	0 0	0	0 0	0 1	1 4
Total	0	0	0	0	0	0		1	0	0	0	7		1	0	0	0	4	11
. 0 (4)	·	Ü	J	Ü	Ü	اد		-	Ū	Ü	Ü	,		-	Ü	Ü	J	71	
Grand Total	1	6	0	0	1	8	13	11	0	0	0	24	12	1	0	0	0	13	45
Approach %	12.5	75.0	0.0	0.0	12.5		54.2	45.8	0.0	0.0	0.0		92.3	7.7	0.0	0.0	0.0		
Total %	2.2	13.3	0.0	0.0	2.2	17.8	28.9	24.4	0.0	0.0	0.0	53.3	26.7	2.2	0.0	0.0	0.0	28.9	
Exiting Leg Total						15						18						12	45
	-												-						

AM Peak Hour Analysis from 06:00 AM to 10:00 AM begins at:

			007	0 10.00	Бед.														_	
8:45 AM			Mill S	Street				Trap	oelo Roa	d (Route	60)			Trap	elo Roa	d (Route	60)			
			from	North					from	East					from	West				1
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total	ı

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

Bicycles (on Roadway and Crosswalks)

Class:							Bicycle	s (on I	Roadw	ay and	l Cross	walks)							
			Mill S	treet				Trap	elo Roa	d (Route	60)			Trap	oelo Roa	d (Route	60)		
			from	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
8:45 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
9:00 AM	0	0	0	0	0	0	0	2	0	0	0	2	0	0	0	0	0	0	2
9:15 AM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
9:30 AM	0	0	0	0	0	0	1	1	0	0	0	2	2	0	0	0	0	2	4
Total Volume	0	1	0	0	0	1	1	4	0	0	0	5	2	0	0	0	0	2	8
% Approach Total	0.0	100.0	0.0	0.0	0.0		20.0	80.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.250	0.000	0.000	0.000	0.250	0.250	0.500	0.000	0.000	0.000	0.625	0.250	0.000	0.000	0.000	0.000	0.250	0.500
Entering Leg	0	1	0	0	0	1	1	4	0	0	0	5	2	0	0	0	0	2	8
Exiting Leg						1						3						4	8
Total						2						8						6	16

MidDay Peak Hour Analysis from 10:00 AM to 2:00 PM begins at:

12:00 PM			Mill S	treet				Trap	elo Road	d (Route	60)			Trap	elo Roa	d (Route	60)		
			from I	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
12:00 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:15 PM	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
12:45 PM	0	0	0	0	0	0	3	0	0	0	0	3	0	0	0	0	0	0	3
Total Volume	0	2	0	0	0	2	3	0	0	0	0	3	0	0	0	0	0	0	5
% Approach Total	0.0	100.0	0.0	0.0	0.0		100.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.500	0.000	0.000	0.000	0.500	0.250	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.417
Entering Leg	0	2	0	0	0	2	3	0	0	0	0	3	0	0	0	0	0	0	5
Exiting Leg						3						2						0	5
Total		•	·	·		5		·			·	5						0	10

PM Peak Hour Analysis from 2:00 PM to 06:00 PM begins at:

5:00 PM			Mill S	treet				Trap	elo Road	d (Route	60)			Trap	oelo Roa	d (Route	60)		
			from	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
5:00 PM	0	0	0	0	0	0	2	1	0	0	0	3	1	0	0	0	0	1	4
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	2	2
5:30 PM	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	3	0	0	0	0	3	1	0	0	0	0	1	4
Total Volume	0	0	0	0	0	0	6	1	0	0	0	7	3	1	0	0	0	4	11
% Approach Total	0.0	0.0	0.0	0.0	0.0		85.7	14.3	0.0	0.0	0.0		75.0	25.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.500	0.250	0.000	0.000	0.000	0.583	0.750	0.250	0.000	0.000	0.000	0.500	0.688
Entering Leg	0	0	0	0	0	0	6	1	0	0	0	7	3	1	0	0	0	4	11
Exiting Leg						7						3						1	11
Total						7						10						5	22

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

Pedestrians

End Time: Class:	6:00 PM								Pedes	trians									
			Mill St					Trap		d (Route	60)			Trap		ad (Route	60)		
			from N						from		П	1		T 1		West			
6:00 AM	Right	Left 0	U-Turn	CW-EB	сw-wв	Total 0	Right	Thru 0	U-Turn	cw-sa	CW-NB	Total 1	Thru 0	Left 0	U-Turn	cw-nb	cw-sb	Total	Total 1
6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
6:30 AM	0	0	0	0	0	0	0	0	0	0	2			0	0		0	0	2
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	1	2	3	0	0	0	0	0	0	3
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	1	1
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
7:45 AM Total	0	0	0	0	0	0		0	0	0	0	0		0			0	1	
		U	U	U	U	U	. 0	U	U	U	U	0		U	U	1	U	1	
8:00 AM	0	0	0	0	1	1	0	0	0	0	0	0		0	0		0	0	
8:15 AM 8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
8:45 AM	0	0	0	0	1	1	0	0	0	0	0	0		0			0	0	1 0
Total	0	0	0	0	2	2		0	0	0	0	0		0			0	0	
9:00 AM	I .	0	0	0	0			0	0	0	0	0		0	0		1	1	•
9:00 AM 9:15 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		1	1	1
9:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
9:45 AM	0	0	0	0	0	0	0	0	0	0	1	1		0			0	0	
Total	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	1	1	2	3
10:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0		0			0	2	2
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	
11:15 AM	0	0	0	1	0	1	0	0	0	1	0	1		0	0		2	2	4
11:30 AM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	0
11:45 AM Total	0	0	0	0	0	1		0	0	0	0	1	_	0			2	2	
12:00 PM 12:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	2	2
12:30 PM	0	0	0	0	0	0	0	0	0	0	0	0		0			1	1	1
12:45 PM	0	0	0	0	0	0	0	0	0	0	0	0		0			0	1	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	1	4	4
1:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:15 PM	0	0	0	1	0	1	0	0	0	0	0	0		0	0		1	2	3
1:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
1:45 PM	0	0	0	0	0	0	0	0	0	0	0	0		0			0	0	0
Total	0	0	0	1	0	1	0	0	0	0	0	0	0	0	0	1	1	2	3
2:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
2:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	0	
2:30 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		0	1	1
2:45 PM Total	0	0	0	0	0	0	0	0	0	0	0	0		0			1	3	1
													-						
3:00 PM 3:15 PM	0	0	0	0	0	0	0	0	0	0	0	0		0	0		1	1	1 2
3:30 PM	0	0	0	0	1	1	0	0	0	0	0	0		0			0	1	0
3:45 PM	0	0	0	0	0	0	0	0	0	0	0			0			0	0	0
Total	0	0	0	0	1	1		0	0	0	0			0			2	2	
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0			0			0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0			0			0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0			0			1	1	1
Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	1	0	1	0	0	0	0	0			0	0		1	2	3
5:30 PM	0	0	0	0	0	0	0	0	0	0	0			0			0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0			0			0	0	0
Total	0	U	0	1	0	1	0	0	0	0	0	0	0	0	0	1	1	2	I 3
			_					_									_		
Grand Total Approach %	0	0	0	3 50	3 50	6	0	0	0	2 40	3 60	5	0	0			10 47.619	21	32
Total %	0	0	0	9.375	9.375	18.75	0	0	0	6.25	9.375	15.625		0		34.375		65.625	
Exiting Leg Total	ΙŤ			2.3,3	2.373	6				3.23	2.3.3	5					-1.23	21	32
-						_	•						•					_	•

AM Peak Hour Analysis from 06:00 AM to 10:00 AM begins at:

6:00 AM			Mill S	treet				Tra	oelo Roa	d (Route	60)			Trap	elo Roa	d (Route	: 60)		<u> </u>
	from North								from	East					from	West			
	Right	Right Left U-Turn CW-EB CW-WB Total					Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total

Location: E: Trapelo Road (Route 60) W: Trapelo Road (Route 60)

City, State: Belmont, MA

Client: BSC Group/ S. Offei-Addo

Site Code: TBA

Count Date: Wednesday, April 11, 2018

Start Time: 6:00 AM End Time: 6:00 PM



46 Morton Street, Framingham, MA 01702 Office: 508-875-0100 Fax: 508-875-0118 Email: datarequests@pdillc.com

Pedestrians

Class:									Pedes	trians									
			Mill S	treet				Trap	elo Roa	d (Route	60)			Trap	elo Roa	d (Route	60)		
			from	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
6:00 AM	0	0	0	0	0	0	0	0	0	1	0	1	0	0	0	0	0	0	1
6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 AM	0	0	0	0	0	0	0	0	0	0	2	2	0	0	0	0	0	0	2
6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Volume	0	0	0	0	0	0	0	0	0	1	2	3	0	0	0	0	0	0	3
% Approach Total	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	33.3	66.7		0.0	0.0	0.0	0.0	0.0		
PHF	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.250	0.375	0.000	0.000	0.000	0.000	0.000	0.000	0.375
Entering Leg	0	0	0	0	0	0	0	0	0	1	2	3	0	0	0	0	0	0	3
Exiting Leg						0						3						0	3
Total		•	•		•	0		•		·	•	6		•	·	•	·	0	6

MidDay Peak Hour Analysis from 10:00 AM to 2:00 PM begins at:

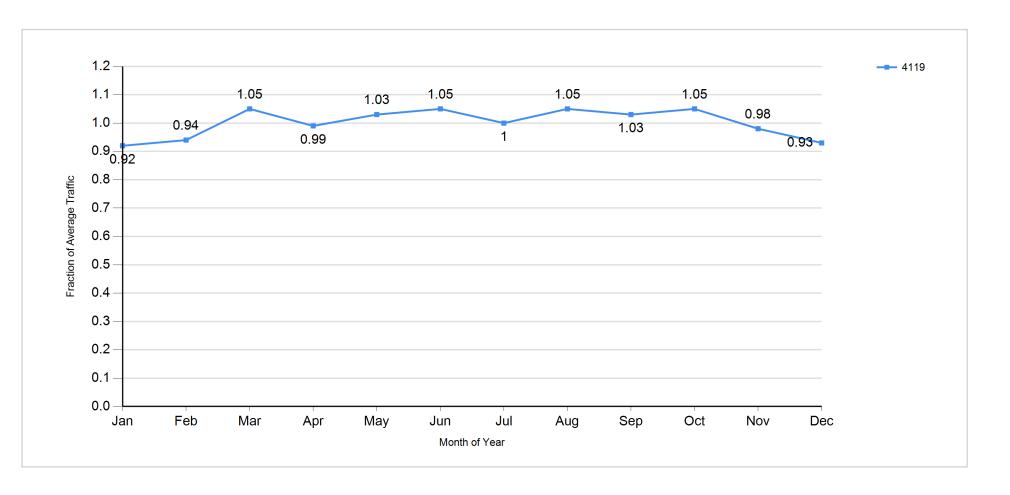
10:30 AM			Mill S	treet				Trap	elo Road	d (Route	60)			Trap	elo Roa	d (Route	60)		
			from I	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
10:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
10:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
11:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11:15 AM	0	0	0	1	0	1	0	0	0	1	0	1	0	0	0	0	2	2	4
Total Volume	0	0	0	1	0	1	0	0	0	1	0	1	0	0	0	2	2	4	6
% Approach Total	0.0	0.0	0.0	100.0	0.0		0.0	0.0	0.0	100.0	0.0		0.0	0.0	0.0	50.0	50.0		
PHF	0.000	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.250	0.250	0.500	0.375
Entering Leg	0	0	0	1	0	1	0	0	0	1	0	1	0	0	0	2	2	4	6
Exiting Leg						1						1						4	6
Total						2						2						8	12

PM Peak Hour Analysis from 2:00 PM to 06:00 PM begins at:

2:30 PM			Mill S	treet				Trap	elo Road	d (Route	60)			Trap	oelo Roa	d (Route	60)		
			from	North					from	East					from	West			
	Right	Left	U-Turn	CW-EB	CW-WB	Total	Right	Thru	U-Turn	CW-SB	CW-NB	Total	Thru	Left	U-Turn	CW-NB	CW-SB	Total	Total
2:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
2:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
3:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
3:15 PM	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	1	1	2
Total Volume	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	1	3	4	5
% Approach Total	0.0	0.0	0.0	0.0	100.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	25.0	75.0		
PHF	0.000	0.000	0.000	0.000	0.250	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.750	1.000	0.625
Entering Leg	0	0	0	0	1	1	0	0	0	0	0	0	0	0	0	1	3	4	5
Exiting Leg						1						0						4	5
Total						2						0						8	10



Traffic Pattern by Month for 1/1/2018 - 12/31/2018



N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 1

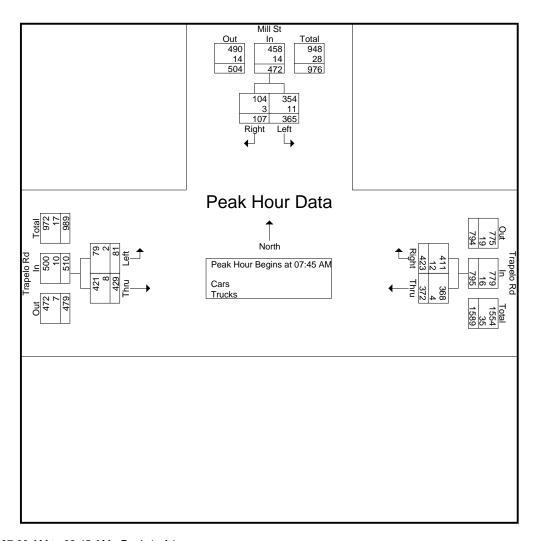
Groups Printed- Cars - Trucks

	Mill S From No		Trape From		Trape From		
Start Time	Left	Right	Thru	Right	Left	Thru	Int. Total
07:00 AM	56	19	75	72	22	89	333
07:15 AM	93	24	70	77	30	103	397
07:30 AM	89	27	81	81	27	131	436
07:45 AM	91	23	89	97	27	104	431
Total	329	93	315	327	106	427	1597
08:00 AM	89	22	105	103	18	114	451
08:15 AM	94	36	87	110	21	109	457
08:30 AM	91	26	91	113	15	102	438
08:45 AM	74	24	92	91	30	100	411
Total	348	108	375	417	84	425	1757
Grand Total	677	201	690	744	190	852	3354
Apprch %	77.1	22.9	48.1	51.9	18.2	81.8	
Total %	20.2	6	20.6	22.2	5.7	25.4	
Cars	657	198	681	725	186	832	3279
% Cars	97	98.5	98.7	97.4	97.9	97.7	97.8
Trucks	20	3	9	19	4	20	75
% Trucks	3	1.5	1.3	2.6	2.1	2.3	2.2

		Mill St From North			Trapelo Rd From East			Trapelo Rd From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From	n 07:00 AM to 0	08:45 AM - F	Peak 1 of 1		_					
Peak Hour for Entire Inte	rsection Begins	s at 07:45 A	M							
07:45 AM	91	23	114	89	97	186	27	104	131	431
08:00 AM	89	22	111	105	103	208	18	114	132	451
08:15 AM	94	36	130	87	110	197	21	109	130	457
08:30 AM	91	26	117	91	113	204	15	102	117	438
Total Volume	365	107	472	372	423	795	81	429	510	1777
% App. Total	77.3	22.7		46.8	53.2		15.9	84.1		
PHF	.971	.743	.908	.886	.936	.956	.750	.941	.966	.972
Cars	354	104	458	368	411	779	79	421	500	1737
% Cars	97.0	97.2	97.0	98.9	97.2	98.0	97.5	98.1	98.0	97.7
Trucks	11	3	14	4	12	16	2	8	10	40
% Trucks	3.0	2.8	3.0	1.1	2.8	2.0	2.5	1.9	2.0	2.3

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

Lacil Appl	Dacii Degino	ui.							
	07:45 AM			07:45 AM			07:15 AM		
+0 mins.	91	23	114	89	97	186	30	103	133
+15 mins.	89	22	111	105	103	208	27	131	158
+30 mins.	94	36	130	87	110	197	27	104	131
+45 mins.	91	26	117	91	113	204	18	114	132
Total Volume	365	107	472	372	423	795	102	452	554
% App. Total	77.3	22.7		46.8	53.2		18.4	81.6	
PHF	.971	.743	.908	.886	.936	.956	.850	.863	.877
Cars	354	104	458	368	411	779	101	443	544
% Cars	97	97.2	97	98.9	97.2	98	99	98	98.2
Trucks	11	3	14	4	12	16	1	9	10
% Trucks	3	2.8	3	1.1	2.8	2	1	2	1.8

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

Groups Printed- Bikes Peds

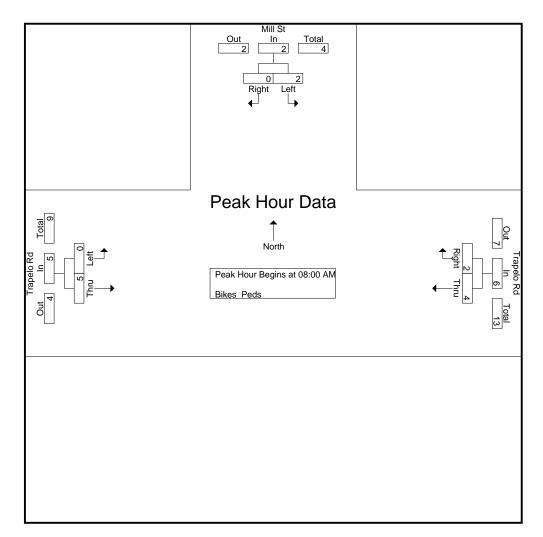
File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 10

		Mill St om North			apelo Rd rom East			rapelo Rd rom West				
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
07:00 AM	0	0	0	1	0	0	0	1	0	0	2	2
07:15 AM	2	0	0	0	0	1	0	2	0	1	4	5
07:30 AM	1	0	1	1	0	2	0	1	0	3	3	6
07:45 AM	1	0	2	0	0	2	0	0	0	4	1	5
Total	4	0	3	2	0	5	0	4	0	8	10	18
00.00.444		•	0		•	2		•				
08:00 AM	0	0	0	1	0	2	0	0	1	3	1	4
08:15 AM	1	0	0	0	0	3	0	1	1	4	2	6
08:30 AM	0	0	0	1	2	1	0	1	0	1	4	5
08:45 AM	1	0	0	2	0	0	0	3	2	2	6	8
Total	2	0	0	4	2	6	0	5	4	10	13	23
Grand Total Apprch %	6 100	0 0	3	6 75	2 25	11	0 0	9 100	4	18	23	41
Total %	26.1	0		26.1	8.7		0	39.1		43.9	56.1	

		Mill St			Trapelo Rd			Trapelo Rd		
		From North			From East			From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 07:00 AM to 0	08:45 AM - P	eak 1 of 1		_					
Peak Hour for Entire Inte										
08:00 AM	0	0	0	1	0	1	0	0	0	1
08:15 AM	1	0	1	0	0	0	0	1	1	2
08:30 AM	0	0	0	1	2	3	0	1	1	4
08:45 AM	1	0	1	2	0	2	0	3	3	6
Total Volume	2	0	2	4	2	6	0	5	5	13
Mapp. Total	100	0		66.7	33.3		0	100		
PHF	.500	.000	.500	.500	.250	.500	.000	.417	.417	.542

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 11



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

I can riour for Each Appl	loach Bogine	ut.							
	07:00 AM			08:00 AM			08:00 AM		
+0 mins.	0	0	0	1	0	1	0	0	0
+15 mins.	2	0	2	0	0	0	0	1	1
+30 mins.	1	0	1	1	2	3	0	1	1
+45 mins.	1	0	1	2	0	2	0	3	3
Total Volume	4	0	4	4	2	6	0	5	5
% App. Total	100	0		66.7	33.3		0	100	
PHF	.500	.000	.500	.500	.250	.500	.000	.417	.417

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 1

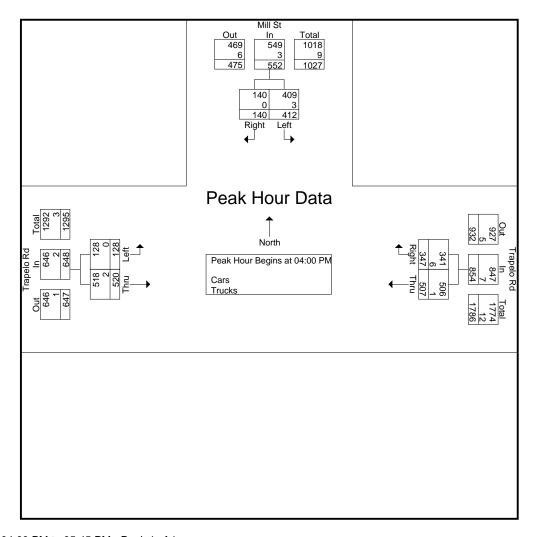
Groups Printed- Cars - Trucks

	Mill St From Nor		Trapel From		Trape From		
Start Time	Left	Right	Thru	Right	Left	Thru	Int. Total
04:00 PM	88	29	113	90	44	152	516
04:15 PM	109	39	123	92	24	113	500
04:30 PM	114	33	123	89	27	132	518
04:45 PM	101	39	148	76	33	123	520
Total	412	140	507	347	128	520	2054
05:00 DM	05	24	4.40	70	20	400	400
05:00 PM	95	21	143	70	22	132	483
05:15 PM	102	34	168	77	22	107	510
05:30 PM	85	20	97	71	26	99	398
05:45 PM	57	32	103	52	28	103	375
Total	339	107	511	270	98	441	1766
Grand Total	751	247	1018	617	226	961	3820
Apprch %	75.3	24.7	62.3	37.7	19	81	
Total %	19.7	6.5	26.6	16.2	5.9	25.2	
Cars	747	247	1016	608	225	958	3801
% Cars	99.5	100	99.8	98.5	99.6	99.7	99.5
Trucks	4	0	2	9	1	3	19
% Trucks	0.5	0	0.2	1.5	0.4	0.3	0.5

		Mill St		7	Trapelo Rd			Trapelo Rd		
		From North			From East			From West		
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 04:00 PM to 0	05:45 PM - P	eak 1 of 1		_					
Peak Hour for Entire Inte	rsection Begins	s at 04:00 PN	Л							
04:00 PM	88	29	117	113	90	203	44	152	196	516
04:15 PM	109	39	148	123	92	215	24	113	137	500
04:30 PM	114	33	147	123	89	212	27	132	159	518
04:45 PM	101	39	140	148	76	224	33	123	156	520
Total Volume	412	140	552	507	347	854	128	520	648	2054
% App. Total	74.6	25.4		59.4	40.6		19.8	80.2		
PHF	.904	.897	.932	.856	.943	.953	.727	.855	.827	.988
Cars	409	140	549	506	341	847	128	518	646	2042
% Cars	99.3	100	99.5	99.8	98.3	99.2	100	99.6	99.7	99.4
Trucks	3	0	3	1	6	7	0	2	2	12
% Trucks	0.7	0	0.5	0.2	1.7	0.8	0	0.4	0.3	0.6

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at:

reak Hour for Lacif Appl	ioacii begins	aı.							
	04:00 PM			04:30 PM			04:00 PM		
+0 mins.	88	29	117	123	89	212	44	152	196
+15 mins.	109	39	148	148	76	224	24	113	137
+30 mins.	114	33	147	143	70	213	27	132	159
+45 mins.	101	39	140	168	77	245	33	123	156
Total Volume	412	140	552	582	312	894	128	520	648
% App. Total	74.6	25.4		65.1	34.9		19.8	80.2	
PHF	.904	.897	.932	.866	.876	.912	.727	.855	.827
Cars	409	140	549	581	306	887	128	518	646
% Cars	99.3	100	99.5	99.8	98.1	99.2	100	99.6	99.7
Trucks	3	0	3	1	6	7	0	2	2
% Trucks	0.7	0	0.5	0.2	1.9	0.8	0	0.4	0.3

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 10

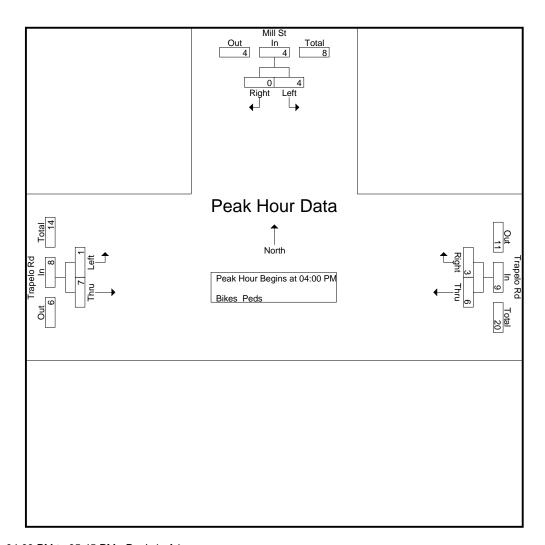
Groups	Printed-	Bikes	Peds
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	Mill St				apelo Rd		Tr	apelo Rd				
	Fr	om North		F	rom East		Fr	om West				
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Exclu. Total	Inclu. Total	Int. Total
04:00 PM	0	0	1	0	0	2	0	4	0	3	4	7
04:15 PM	1	0	0	4	1	0	0	2	0	0	8	8
04:30 PM	1	0	0	1	0	0	1	1	0	0	4	4
04:45 PM	2	0	0	1	2	1	0	0	2	3	5	8
Total	4	0	1	6	3	3	1	7	2	6	21	27
05:00 PM	0	0	ا م	0	0	ا م	0	4	0		1	1
	U	U	0	U	0	U	0	1	0	U	I -	1
05:15 PM	1	1	0	0	0	0	0	0	0	0	2	2
05:30 PM	1	0	0	0	0	0	0	1	0	0	2	2
05:45 PM	0	0	1	0	0	0	0	2	0	1	2	3_
Total	2	1	1	0	0	0	0	4	0	1	7	8
Grand Total	6	1	2	6	3	3	1	11	2	7	28	35
Apprch %	85.7	14.3		66.7	33.3		8.3	91.7				
Total %	21.4	3.6		21.4	10.7		3.6	39.3		20	80	

		Mill St			Trapelo Rd					
	I	From North			From East					
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis Fron	n 04:00 PM to 0)5:45 PM - P	eak 1 of 1		_					
Peak Hour for Entire Inte	rsection Begins	at 04:00 PN	И .							
04:00 PM	0	0	0	0	0	0	0	4	4	4
04:15 PM	1	0	1	4	1	5	0	2	2	8
04:30 PM	1	0	1	1	0	1	1	1	2	4
04:45 PM	2	0	2	1	2	3	0	0	0	5_
Total Volume	4	0	4	6	3	9	1	7	8	21
% App. Total	100	0		66.7	33.3		12.5	87.5		
PHF	.500	.000	.500	.375	.375	.450	.250	.438	.500	.656

N/S Street : Trapelo Road E/W Street : Mill Street City/State : Belmont, MA Weather : Clear

File Name: 80580001 Site Code : 80580001 Start Date : 11/10/2020 Page No : 11



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1

Peak Hour for Each Approach Begins at:

I can riour for Edon Appr	Cach Bogine	и							
	04:30 PM			04:00 PM			04:00 PM		
+0 mins.	1	0	1	0	0	0	0	4	4
+15 mins.	2	0	2	4	1	5	0	2	2
+30 mins.	0	0	0	1	0	1	1	1	2
+45 mins.	1	11	2	1	2	3	0	0	0
Total Volume	4	1	5	6	3	9	1	7	8
% App. Total	80	20		66.7	33.3		12.5	87.5	
PHF	.500	.250	.625	.375	.375	.450	.250	.438	.500



Crash Number	City Town Name	Crash Date	Weekday	Crash Severity	Crash Time	Number of Vehicles	Age of Driver - Youngest Known	Driver Contributing Circumstances (All Drivers)	First Harmful Event	Light Conditions	Manner of Collision	Road Surfac e Condi tion	Vehicle Actions Prior to Crash (All Vehicles)	Vehicle Travel Directions (All Vehicles)	Weather Conditions	Latitude	Longitude	Street Numb er	Roadway	Near Intersection Roadway
									PLEASANT ST /	TRAPELO	RD									
			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Property damage				04 (11) (00	a 10 - 1 - 1 - 1 - 1 - 1		Sideswipe,			= /				p		
3372025	BELMONT	03/08/2013	Friday	injured)	7:55 AM	2	55-64	D1: (Unknown) / D2: (Unknown)	Collision with motor vehicle in traffic	Daylight	same direction	Snow	V1: Slowing or stopped in traffic / V2: Travelling straight ahead	V1: E / V2:	Snow/Blowing sand, snow	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3414798	BELMONT	05/01/2013	Wednesday	Property damage only (none injured)	5:25 PM	2	21-24	D1: (No improper driving) / D2: (Followed too closely)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Entering traffic lane / V2: Entering traffic lane	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3414802	BELMONT	05/05/2013	Sunday	Not Reported	4:02 PM	2	35-44	D1: (Inattention) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Slowing or stopped in traffic	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3430036	BELMONT	05/26/2013	Sunday	Not Reported	7:27 PM	2	18-20	D1: (Failed to yield right of way),(Glare) / D2: (No improper driving)	Collision with motor vehicle in traffic	Dusk	Angle	Dry	V1: Turning left / V2: Travelling straight ahead	V1: S / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3573487	BELMONT	07/21/2013	Sunday	Non-fatal injury	10:26 AM	1	65-74	D1: (No improper driving)	Collision with pedalcycle (bicycle, tricycle, unicycle, pedal car)	Daylight	Sideswipe, same direction	Dry	V1: Travelling straight ahead	V1: S	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3605559	BELMONT	09/25/2013	Wednesday	Property damage only (none injured)	7:11 PM	2	18-20	D1: (Inattention),(Disregarded traffic signs, signals, road markings) / D2: (No improper driving)	Collision with motor vehicle in traffic	Dark - lighted roadway	Angle	Dry	V1: Turning left / V2: Travelling straight ahead	V1: S / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3700131	BELMONT	12/16/2013	Monday	Non-fatal injury	6:54 PM	3	16-17	D1: (Followed too closely) / D2: (No improper driving) / D3: (No improper driving)	Collision with motor vehicle in traffic	Dark - lighted roadway	Rear-end	Ice	V1: Slowing or stopped in traffic / V2: Slowing or stopped in traffic / V3: Slowing or stopped in traffic	V1: E / V2: E / V3: E	Clear	42.38890721	-71.19298671	576	TRAPELO RD	
3731020	BELMONT	01/05/2014	Sunday	Property damage only (none injured)	4:13 PM	2	21-24	D1: (Followed too closely) / D2: (No improper driving)	Collision with motor vehicle in traffic	Dusk	Rear-end	Wet	V1: Travelling straight ahead / V2: Slowing or stopped in traffic	V1: W / V2: W	Rain/Cloudy	42.38875325	-71.19264741		PLEASANT ST Rte 60 W / TRAPELO RD	
3731027	BELMONT	01/14/2014	Tuesday	Property damage only (none injured)	5:41 PM	2	55-64	D1: (Failed to yield right of way) / D2: (No improper driving)	Collision with motor vehicle in traffic	Dark - lighted roadway	Sideswipe, same direction	Wet	V1: Changing lanes / V2: Travelling straight ahead	V1: E / V2: E	Rain	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3733273	BELMONT	02/05/2014	Wednesday	Property damage only (none injured)	12:32 PM	1	35-44	D1: (Unknown)	Overturn/rollover	Daylight	Single vehicle crash	Slush	V1: Entering traffic lane	V1: W	Snow/Blowing sand, snow	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	
3784412	BELMONT	03/21/2014	Friday	Property damage only (none injured)	7:53 AM	2	25-34	D1: (Unknown) / D2: (Unknown)	Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Turning left / V2: Travelling straight ahead	V1: S / V2: W	Cloudy	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3799403	BELMONT	04/16/2014	Wednesday	Property damage only (none injured)	6:50 PM	2	25-34	D1: (Glare),(Inattention) / D2: (No improper driving)	Collision with motor vehicle in traffic	Dusk	Rear-end	Dry	V1: Entering traffic lane / V2: Slowing or stopped in traffic	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3799409	BELMONT	05/01/2014	Thursday	Non-fatal injury	5:37 PM	2	55-64	D1: (Failed to yield right of way) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Head-on	Dry	V1: Turning left / V2: Travelling straight ahead	V1: E / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3869503	BELMONT	06/11/2014	Wednesday	Property damage only (none injured)	10:48 AM	2	25-34	D1: (No improper driving),(No improper driving) / D2: (Failed to yield right of way),(Failed to yield right of way)	Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Turning left	V1: W / V2: W	Clear/Clear	42.38875325	-71.19264741		PLEASANT ST Rte 60 / TRAPELO RD	
3910702	BELMONT	08/13/2014	Wednesday	injured)	11:38 AM	2	16-17	D1: (No improper driving) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Wet	V1: Slowing or stopped in traffic / V2: Travelling straight ahead	V1: W / V2: W	Cloudy/Rain	42.38870801	-71.19256262	563	TRAPELO RD	
3911039	BELMONT	07/08/2014	Tuesday	injured)	9:53 AM	2	25-34	D1: (Unknown) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Turning right / V2: Travelling straight ahead	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	
3950575	BELMONT	08/21/2014	Thursday	injured)	8:12 AM	2	35-44	D1: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Backing	V1: S / V2: N	Cledi	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3988936	BELMONT	10/30/2014	Thursday	Property damage only (none injured)	8:32 AM	2	45-54	D1: (Failed to yield right of way),(Glare) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Sideswipe, same direction	Dry	V1: Entering traffic lane / V2: Travelling straight ahead	V1: E / V2: E	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3988944	BELMONT	11/13/2014	Thursday	Property damage only (none injured)	7:50 AM	2	35-44	markings),(Failure to keep in proper lane or running off road)	Collision with motor vehicle in traffic	Daylight	Sideswipe, same direction	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead		Clear/Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
3990023	BELMONT	10/10/2014	Friday		5:30 PM	2	45-54	D1: (Unknown) / D2: (No improper driving),(No improper driving)	Collision with motor vehicle in traffic	Daylight	Sideswipe, same direction	Dry	V1: Other / V2: Travelling straight ahead	V1: E / V2: Not Reported	Clear	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	
4060665	BELMONT	01/29/2015	Thursday	Property damage only (none injured)	12:02 PM	2	35-44	D1: (Inattention) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Wet	V1: Entering traffic lane / V2: Entering traffic lane	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4060679	BELMONT	02/09/2015	Monday	Not Reported	5:00 PM	3	18-20		Collision with motor vehicle in traffic	Dark - lighted roadway	Rear-end	Snow	V1: Travelling straight ahead / V2: Slowing or stopped in traffic / V3: Entering traffic lane	V1: W / V2: W / V3: Not Reported	Snow	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	

4																				
Crash Number	City Town Name	Crash Date	Weekday	Crash Severity	Crash Time	Number of Vehicles	of Age of Driver - Youngest Known	Driver Contributing Circumstances (All Drivers)	First Harmful Event	Light Conditions	Manner of Collision	Road Surfac e Condi tion	Vehicle Actions Prior to Crash i (All Vehicles)	Vehicle Travel Directions (All Vehicles)	Weather Conditions	Latitude	Longitude	Street Numb er	Roadway	Near Intersection Roadway
4060683	BELMONT	02/12/2015		Property damage only (none injured)	10:50 AM	2			Collision with motor vehicle in traffic	Daylight	Sideswipe, same direction	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead	V1: E / V2: E	Clear	42.38875325	-71.19264741	560	TRAPELO ROAD	PLEASANT STREET
4060695	BELMONT	02/20/2015	Friday	Property damage only (none injured)	e 4:07 PM	2		D1: (Followed too closely) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Slowing or stopped in traffic	V1: W / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4060702	BELMONT	02/27/2015		Property damage	e 10:37 AM	1	1 55-64	D1: (No improper driving)	Collision with motor vehicle in traffic	Unknown	Rear-end	Unkno wn	V1: Slowing or stopped in traffic	V1: E	Unknown	42.38875325	-71.19264741	0	PLEASANT ST / TRAPELO RD	
4060747	BELMONT	04/23/2015	Thursday	Not Reported	7:39 AM	1	-		Collision with pedalcycle (bicycle, tricycle, unicycle, pedal car)	Daylight	Sideswipe, same direction	Dry	V1: Turning right	V1: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4060783	BELMONT	06/04/2015	Thursday	Not Reported	7:29 AM	2	2 25-34		Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead	V1: S / V2: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4062484	BELMONT	05/29/2015			1:09 PM	2	2 45-54	D2: (Inattention)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Slowing or stopped in traffic / V2: Travelling straight ahead	V2: W	Clear	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	ļ
4076411	BELMONT	08/05/2015		Property damage only (none injured)	e 8:24 AM	3	3 25-34	improper driving) / D3: (No improper driving)	in trainc	Daylight	Angle	Dry	stopped in traffic	V1: E / V2: W / V3: S	1	42.38875325	-71.19264741		TRAPELO ROAD / TRAPELO ROAD / PLEASANT STREET	
4076418	BELMONT	07/07/2015	Tuesday		2:36 PM	2		D1: (Followed too closely) / D2: (No improper driving)	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead	V1: S / V2: S	.i	42.38875325	-71.19264741	ļ	PLEASANT STREET	TRAPELO ROAD
4177563	BELMONT	09/16/2015	5 Wednesday	injured)	8:58 AM	2	2 45-54		in trainc	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Entering traffic lane	V1: E / V2: E	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4177603	BELMONT	11/03/2015	Tuesday	injured)	9:46 PM	1	1 35-44	D1: (Fatigued/asleep)	Collision with utility pole	Dark - lighted roadway	Single vehicle crash	Dry	V1: Travelling straight ahead		Clear	42.38875325	-71.19264741		TRAPELO RD / PLEASANT ST	ļ
4178511	BELMONT	09/24/2015	Thursday	injured)	12:37 PM	3	3 23-34		Collision with motor vehicle in traffic	Daylight	Sideswipe, opposite direction	Dry	V1: Turning right / V2: Turning right / V3: Travelling straight ahead	V1: N / V2: N / V3: W	Clear	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	
4178931	BELMONT	09/17/2015	Thursday	Property damage only (none injured)	e 12:34 PM	2	2 21-24	D2: (Failed to yield right of way)	Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Turning left	V1: S / V2: W	Clear	42.38875325	-71.19264741		PLEASANT STREET / TRAPELO ROAD	
4190363	BELMONT	03/11/2016		Property damage only (none injured)	e 3:17 PM	2	2 35-44	D1: (Failure to keep in proper lane or running off road),(Failed to yield right of way) / D2: (No improper driving),(No improper driving)	Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Changing lanes / V2: Travelling straight ahead	V2: W	Clear/Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4190365	BELMONT	03/15/2016	Tuesday	Non-fatal injury 4	4:56 PM	2	2 18-20	D1: (No improper driving) / D2: (Failed to yield right of way),(Inattention)	Not reported	Daylight	Head-on	Wet	V1: Travelling straight ahead / V2: Turning left	V1: W / V2: E	Rain/Cloudy	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4190407	BELMONT	04/27/2016	Į		3:11 PM	3	3 55-64		Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Turning left / V2: Travelling straight ahead / V3: Travelling straight ahead	V1: E / V2: W / V3: S	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4190408	BELMONT	04/28/2016		Property damage only (none injured)	5:31 PM	2			in traffic	Daylight	Rear-end	Dry	V1: Turning left / V2: Travelling straight ahead	V1: N / V2: E	Cloudy	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4190409	BELMONT	04/30/2016			4:25 PM	2	2 21-24	ľ	Collision with motor vehicle in traffic	Daylight	Head-on	Dry	V1: Travelling straight ahead / V2: Turning left	V1: W / V2: E	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4302300	BELMONT	09/29/2016		injured)	5:35 PM	2	2 25-34		Collision with motor vehicle in traffic	Daylight	Angle	Dry	straignt anead	V1: E / V2: W		42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4302314	BELMONT	10/24/2016	Monday	Property damage only (none injured)	9:00 AM	2	2 21-24	1	Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead	V1: E / V2: E	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4302347	BELMONT	12/16/2016		Property damage only (none injured)	e 7:44 AM	3	3 25-34		Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Slowing or stopped in traffic / V2: Slowing or stopped in traffic / V3: Travelling straight ahead	V2: W / V3: W	Clear	42.38875325	-71.19264741		PLEASANT ST / TRAPELO RD	
4380481	BELMONT	05/26/2017	/ Friday	Non-fatal injury	12:04 PM	2	2 55-64		Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Travelling straight ahead	V1: E / V2: E	Clear	42.38900679	-71.19319269		PLEASANT ST / TRAPELO RD	
4380495	BELMONT	06/05/2017		Property damage only (none injured)	7:52 AM	2	2 35-44		Collision with motor vehicle in traffic	Daylight	Rear-end	Wet	V1: Turning right / V2: Travelling straight ahead	V1: N / V2: W	Rain	42.38900679	-71.19319269		PLEASANT ST / TRAPELO RD	
4380622	BELMONT	04/14/2017		Property damage only (none injured)	e 1:27 PM	2	2 35-44		Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Slowing or stopped in traffic / V2: Backing	V1: E / V2: N	Clear/Clear	42.38900679	-71.19319269		TRAPELO RD / PLEASANT ST	
4380633	BELMONT	05/19/2017	7 Friday	injured)	8:01 AM	2	2 25-34		Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Travelling straight ahead / V2: Entering traffic lane	V1: W / V2: S	Clear/Clear	42.38900679	-71.19319269		PLEASANT ST / TRAPELO RD	
4381828	BELMONT	06/21/2017	7 Wednesday	injured)	8:57 AM	2	2 21-24		Collision with motor vehicle in traffic	Daylight	Rear-end	Dry	V1: Travelling straight ahead / V2: Slowing or stopped in traffic	V1: E / V2: E	Clear/Clear	42.38909769	-71.19352015	590	TRAPELO RD	
4399849	BELMONT	07/28/2017		Property damage only (none injured)	e 12:10 PM	2	2 35-44		Collision with motor vehicle in traffic	Daylight	Angle	Dry	V1: Turning left / V2: Travelling straight ahead	V1: N / V2: W	Clear/Clear	42.38900679	-71.19319269		PLEASANT ST Rte 60 N / TRAPELO RD	
4399849	BELMONT	07/28/2017		only (none		2	2 35-44	way),(Unknown) / D2:		Daylight	Angle	Dry			Clear/Clear	42.38900679	-71.19319269		PLEASANT ST Rte 60 N / TRAPELO RD	<u></u>

Crash Number	City Town Name	Crash Date	Weekday	Crash Severity	Crash Time	Number of Vehicles		Driver Contributing Circumstances (All Drivers)	First Harmful Event	Light Conditions	Manner of Collision	Road Surfac e Condi tion	(All Vehicles)	Vehicle Travel Directions (All Vehicles)	Weather Conditions	Latitude	Longitude	Street Numb er	Roadway	Near Intersection Roadway
4415759	BELMONT	08/31/2017	Thursday	Property damage only (none injured)	4:03 PM	2	25-34		Collision with motor vehicle in traffic	Daylight	Sideswipe, same direction		V1: Slowing or stopped in traffic / V2: Travelling straight ahead	V1: W / V2: W	Clear	42.38900679	-71.19319269		PLEASANT ST / TRAPELO RD	
									PLEASANT ST / O	LMSTED	DRIVE									
4380932	BELMONT	02/24/2017	Friday	Non-fatal injury	3:47 PM	1	35-44	D1: (Glare)	Collision with pedalcycle (bicycle, tricycle, unicycle, pedal car)	Dusk	Angle	Dry	V1: Turning left	V1: W	Clear	42.38902647	-71.19046006		PLEASANT STREET	OLMSTED DRIVE



CRASH RATE WORKSHEET

CITY/TOWN : Belmont				COUNT DA	TE:	2020	MHD USE ONLY
DISTRICT: 4	UNSIGN	ALIZED :		SIGNA	LIZED :	х	Source #
		INT	TEDOLOTIC	NI DATA			_
		~ IN	TERSECTION	ON DATA ~	,		annum
MAJOR STREET :	Trapello Ro	ad					ST#
MINOR STREET(S): F	Pleasant St						ST#
_							ST#
_							ST#
_							ST#
INTERSECTION DIAGRAM (Label Approaches)	North	1,292	583 2 3	1 0	1,098		INTERSECTION REF #
			Peak Hou	r Volumes			
APPROACH:	1	2	3	4	5	Total Entering	
DIRECTION:	NB	SB	EB	WB		Vehicles	
VOLUMES (PM):	0	583	1,292	1,098		2,973	
"K" FACTOR:	0.080	APPROA	CH ADT :	37,163	ADT = TOTA	L VOL/"K" FACT	г.
TOTAL # OF ACCIDENTS :	49	# OF YEARS :	5		GE#OF NTS(A):	9.80	
CRASH RATE CALCU	LATION :	0.72	RATE =	<u>(A * 1,0</u> (ADT	000,000) * 365)		
Comments : Accident				ections = 0.7			-

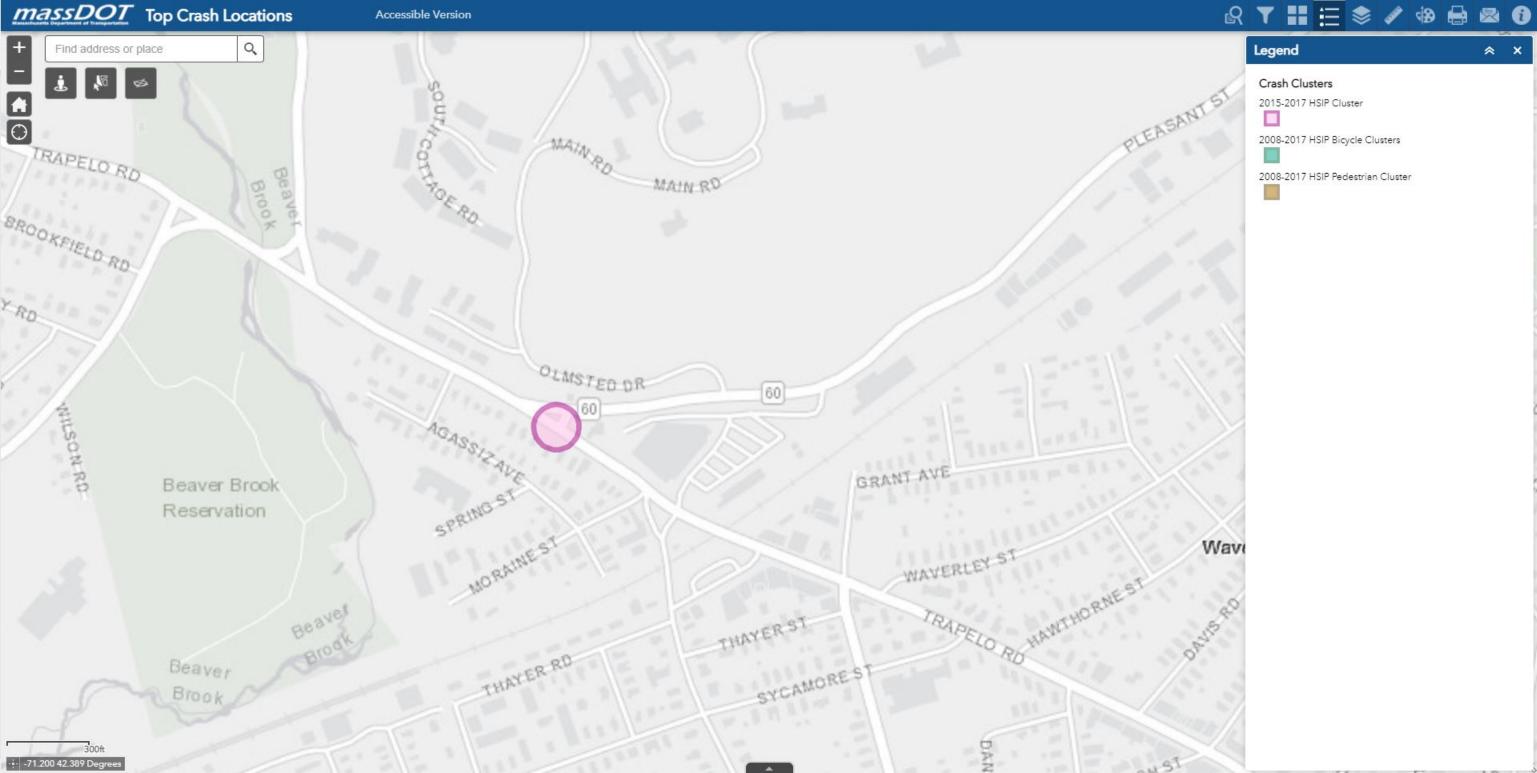
Statewide Accident Rate for Signalized Inteserction = 0.78 and Unsignalized/Inteserction = 0.57



CRASH RATE WORKSHEET

CITY/TOWN : Belmon	ı <u>t</u>			COUNT DA	NTE :	2020	MHD USE ONLY
DISTRICT: 4	UNSIGN	ALIZED :	х	SIGNA	LIZED :		Source #
		~ IN	TERSECTION	N DATA ~			
			ILICEOTIC				
MAJOR STREET :	Pleasant St	reet					ST#
MINOR STREET(S):	Olmsted Dr	ve					ST#
							ST#
							ST#
							ST#
INTERSECTION	Ala atta		6	l			INTERRECTION
INTERSECTION DIAGRAM	North		6 2	4	592		REF #
(Label Approaches)			<u> </u>	١			
		552	3				
			Peak Hou	r Volumes			
APPROACH:	1	2	3	4	5	Total Entering	
DIRECTION:	NB	SB	EB	WB		Vehicles	
VOLUMES (PM):	0	6	552	592		1,150	
"K" FACTOR:	0.080	APPROA	CH ADT :	14,375	ADT = TOTA	L VOL/"K" FAC	т.
TOTAL # OF ACCIDENTS :	1	# OF YEARS :	5		GE#OF NTS(A):	0.20	
CRASH RATE CALC	ULATION :	0.04	RATE =	(A * 1,0 (ADT	000,000) * 365)		
	nt Rate for Di						-

Statewide Accident Rate for Signalized Inteserction = 0.78 and Unsignalized/Inteserction = 0.57





FITCHBURG LINE

Fall/Winter Schedule Effective November 2 2020

Ride Safer. Practice @ a healthy a face good hygiene **(T**) covering distance

Saturday & Sunday Inbound to Boston

Bikes Allowed

North Leominster

Littleton/Rte495

West Concord

Kendal Green

2 Brandeis/Roberts

Wachusett

Fitchburg

Shirley

6 South Acton

Concord

Waltham

1A Porter Square

1A North Station

Saturday & Sunday Outbound from Boston

1 Waverley

1 Belmont

1400

2400

Ø₽

6:30

6:38

f 6:53

6:58

7:06

7:12

f 7:17

7:21

7:26 9:41

f 7:31

f 7:35

7:51

8:01

b

8 6:45

b

ል 7:39

SATURDAY TRAIN # ZONE STATION SUNDAY TRAIN #

1402

2402 2404

640

8:45 10:50

8:53

9:00 11:05

f 9:08 f 11:13

9:13

9:21

9:27

f 9:32

9:36

f 9:46 f 11:51

9:54

f 7:43 f 9:58 f 12:03

f 7:46 | f 10:01 | f 12:06

10:06

10:16

1404

66

10:58

11:18

11:26

11:32

f 11:37

11:41

11:46

11:59

12:11

12:21

1406

2406

₫**₽**

1:15

1:23

1:30 3:55

f 1:38 f 4:03

1:43

1:51

1:57

£2:02 f 4:27

2:06

2:11

f 2:16 f 4:41

2:24

f 2:31

2:36

2:46

1408

2408

60

3:40

3:48

4:08

4:16

4:22

4:31

4:36

4:49

f 4:56 f 7:26

5:01

5:11

1410

2410

50

6:10

6:18

6:25

f 6:33

6:38

6:46

6:52

f 6:57 f 10:32

7:01

7:06

f 7:11 f 10:46

7:19

7:31

7:41 11:16

f 2:28 f 4:53 f 7:23 f 10:58

1412

64

9:45

9:53

10:00

f 10:08

10:13

10:21

10:27

10:36

10:41

10.54

f 11:01

11:06

Mo	nday to Friday						:nec	LIVC	1404	-111100	- I — ,	202	•								
	ound to Boston						AM									F	PM				
ZON	E STATION	TRAIN#	400	402	404	492	406	408	410	412	414	416	418	420	422	494	424	426	428	430	432
	Bikes Allowed		40	<i>₫</i> ₽	640	₫\$	646	6%	6%	₫6	₫\$	66	<i>6</i> 46	<i>6</i> %	5%	66	6%	66	40	49	40
8	Wachusett	8	4:25	5:25	6:20	-	7:10	8:10	9:25	10:25	11:25	12:45	2:05	3:25	4:42	-	5:35	6:57	7:47	8:45	10:10
8	Fitchburg	8	4:33	5:33	6:28	-	7:18	8:18	9:33	10:33	11:33	12:53	2:13	3:33	4:50	-	5:43	7:05	7:55	8:53	10:18
8	North Leominster	8	4:40	5:40	6:35	-	7:25	8:25	9:40	10:40	11:40	1:00	2:20	3:40	4:57	-	5:50	7:12	8:02	9:00	10:2
8	Shirley		4:48	5:48	6:43	-	7:33	8:33	f 9:48	f 10:48	f 11:48	f 1:08	f 2:28	f 3:48	f 5:05	-	f 5:58	f 7:20	f 8:10	f 9:08	f 10:3
8	Ayer		4:53	5:53	6:48	-	7:38	8:38	f 9:53	f 10:53	f 11:53	f 1:13	f 2:33	f 3:53	f 5:10	-	f 6:03	f 7:25	f 8:15	f 9:13	f 10:
7	Littleton/Rte 495	8	5:01	6:01	6:56	7:05	7:46	8:46	f 10:01	f 11:01	f 12:01	f 1:21	f 2:41	f 4:01	5:19	5:38	f 6:11	f 7:33	f 8:23	f 9:21	f 10:4
6	South Acton	8	5:08	6:08	7:03	7:12	7:53	8:53	10:07	11:07	12:07	1:27	2:47	4:07	5:26	5:44	6:17	7:39	8:29	9:27	10:5
5	West Concord	8	5:12	6:12	-	7:17	7:57	8:57	f 10:11	f 11:11	f 12:11	f 1:31	f 2:51	f 4:11	-	f 5:48	f 6:21	f 7:43	f 8:33	f 9:31	f 10:
5	Concord		5:17	6:17	-	7:22	8:02	9:02	f 10:15	f 11:15	f 12:15	f 1:35	f 2:55	f 4:15	-	f 5:52	f 6:25	f 7:47	f 8:37	f 9:35	f 11:0
4	Lincoln		5:23	6:24	-	7:29	8:09	9:08	f 10:21	f 11:21	f 12:21	f 1:41	f 3:01	f 4:21	-	f 5:58	f 6:31	f 7:53	f 8:43	f 9:41	f 11:0
3	Silver Hill		-	f 6:27	-	-	f 8:12	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3	Hastings		5:27	6:30	-	-	8:15	-	f 10:25	-	-	-	-	-	-	-	-	-	-	-	-
3	Kendal Green		5:30	6:33	-	7:36	8:18	9:14	f 10:28	f 11:27	f 12:27	f 1:47	f 3:07	f 4:27	-	f 6:04	f 6:37	f 7:59	f 8:49	f 9:47	f 11:1
2	Brandeis/Roberts	8	5:33	6:37	-	7:40	8:22	9:17	f 10:31	f 11:30	f 12:30	f 1:50	f 3:10	f 4:30	-	f 6:07	f 6:41	f 8:03	f 8:53	f 9:51	f 11:
2	Waltham	8	5:38	6:41	-	7:44	8:26	9:22	10:35	11:34	12:34	1:53	3:13	4:33	5:43	6:12	6:44	8:06	8:56	9:54	11:19
1	Waverley		5:43	6:46	-	7:49	8:31	9:27	f 10:40	f 11:39	f 12:39	-	-	-	-	f 6:17	-	-	f 9:01	-	-
1	Belmont		5:46	6:49	-	7:52	8:34	9:30	f 10:42	f 11:41	f 12:41	f 2:00	f 3:20	f 4:40	-	f 6:19	f 6:50	f 8:12	f 9:03	f 10:00	f 11:2
1A	Porter Square	\$	5:51	6:54	7:31	7:57	8:39	9:35	10:47	11:46	12:46	2:05	3:25	4:45	5:54	6:24	6:55	8:17	9:08	10:05	11:30
1A	North Station	8	6:02	7:05	7:42	8:08	8:50	9:46	10:57	11:56	12:56	2:15	3:35	4:56	6:05	6:35	7:05	8:27	9:18	10:15	11:40

Mo	nday to Friday																				
0	utbound from Bosto	n)			Α	М								P	М						AM
ZON	IE STATION	TRAIN #	491	401	403	405	407	409	411	413	415	417	493	419	421	423	425	427	429	431	433
	Bikes Allowed		56	66	<i>6</i> 46	640	€	€	<i>6</i> 46	6%	646	640	640	640	640	640	6%	646	640	<i>6</i> 46	640
1.4	North Station	8	5:55	6:30	7:30	8:35	9:35	10:35	11:55	1:15	2:30	3:30	4:15	4:45	5:00	5:45	6:40	7:40	9:15	10:40	12:10
1.4	Porter Square	8	6:05	6:40	7:40	8:45	9:45	10:45	12:05	1:25	2:40	3:40	4:25	4:56	5:10	5:55	6:50	7:50	9:25	10:50	12:20
1	Belmont		f 6:09	-	f 7:45	f 8:50	f 9:50	f 10:50	f 12:10	f 1:30	f 2:45	3:45	4:30	-	5:15	6:00	6:55	f 7:55	f 9:30	f 10:55	f 12:25
1	Waverley		f 6:11	-	f 7:47	f 8:52	f 9:52	-	-	f 1:32	f 2:47	3:48	4:33	-	5:18	6:03	6:58	f 7:57	f 9:32	f 10:57	f 12:27
2	Waltham	8	6:16	-	7:52	8:57	9:57	10:55	12:15	1:37	2:52	3:53	4:38	-	5:23	6:08	7:03	8:02	9:37	11:02	12:32
2	Brandeis/Roberts	8	f 6:19	-	f 7:56	f 9:00	f 10:00	f 10:58	f 12:18	f 1:40	f 2:56	3:57	4:42	-	5:27	6:12	7:07	f 8:05	f 9:40	f 11:05	f 12:35
3	Kendal Green		f 6:23	-	f 8:00	f 9:04	f 10:04	f 11:02	f 12:22	f 1:44	f 3:00	4:01	4:46	-	5:31	6:16	7:11	f 8:09	f 9:44	f 11:09	f 12:39
3	Hastings		-	-	-	-	-	-	-	-	-	-	f 4:49	-	f 5:34	f 6:19	f 7:14	f 8:12	-	-	-
3	Silver Hill		-	-	-	-	-	-	-	-	-	-	-	-	-	f 6:21	f 7:16	f 8:14	-	-	-
4	Lincoln		f 6:27	-	f 8:05	f 9:09	f 10:09	f 11:07	f 12:27	f 1:49	f 3:05	4:06	4:53	-	5:40	6:25	7:20	8:18	f 9:49	f 11:14	f 12:44
5	Concord		f 6:32	-	f 8:10	f 9:14	f 10:14	f 11:12	f 12:32	f 1:54	f 3:10	4:11	4:58	-	5:45	6:30	7:25	8:23	f 9:54	f 11:19	f 12:49
5	West Concord	8	f 6:36	-	f 8:15	f 9:18	f 10:18	f 11:16	f 12:36	f 1:58	f 3:15	4:16	5:03	-	5:50	6:35	7:30	8:28	f 9:58	f 11:23	f 12:53
6	South Acton	8	6:42	7:04	8:20	9:23	10:23	11:21	12:41	2:03	3:20	4:21	5:08	5:23	5:55	6:40	7:35	8:33	10:03	11:28	12:58
7	Littleton/Rte 495	8	6:50	7:12	8:27	9:30	10:30	11:28	12:48	2:10	3:27	4:28	5:16	5:30	6:02	6:47	7:42	8:40	10:10	11:35	1:05
8	Ayer		-	f 7:20	f 8:35	f 9:38	f 10:38	f 11:36	f 12:56	f 2:18	f 3:35	4:36	-	5:38	6:10	6:55	7:50	8:48	f 10:18	f 11:43	f 1:13
8	Shirley		-	f 7:25	f 8:40	f 9:43	f 10:43	f 11:41	f 1:01	f 2:23	f 3:41	4:42	-	5:43	6:16	7:01	7:56	8:54	f 10:23	f 11:48	f 1:18
8	North Leominster	8	-	7:34	8:50	9:52	10:52	11:50	1:10	2:32	3:50	4:51	-	5:52	6:25	7:10	8:05	9:03	10:32	11:57	1:27
8	Fitchburg	8	-	L 7:44	L 8:58	L 9:59	L 10:59	L 11:59	L 1:19	L 2:40	L 3:57	L 4:58	-	L 5:59	L 6:32	L 7:17	L 8:12	L 9:10	L 10:39	L 12:04	L 1:34
8	Wachusett	8	-	7:54	9:08	10:09	11:09	12:09	1:29	2:50	4:07	5:08	-	6:10	6:42	7:27	8:22	9:20	10:49	12:14	1:44

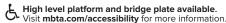
1401 1403 1405 1407 1409 1411 1413 SATURDAY TRAIN # STATION 2401 2403 2405 2407 2409 2411 2413 SUNDAY TRAIN # Bikes Allowed 640 1A North Station 8:35 10:45 1:10 3:30 5:45 7:55 11:30 8:45 1:20 3:40 10:55 5:55 8:05 1 Belmont f 8:49 f 10:59 f 1:24 f 3:44 f 5:59 f 8:09 f 11:44 1 Waverley f 8:52 f 11:02 f 1:27 f 6:02 f 8:12 f 11:47 f 3:47 2 Waltham 8:57 11:07 1:32 3:52 6:07 8:17 11:52 f 9:00 f 11:10 f 1:35 2 Brandeis/Roberts f 3:55 f 6:10 f 8:20 f 11:55 3 Kendal Green f 9:04 f 11:14 1:39 f 3:59 f 6:14 f 8:24 f 11:59 9:09 11:19 1:44 4:04 6:19 8:29 12:04 9:15 1:50 4:10 Concord 11:25 6:25 8:35 12:10 6 f 9:18 f 11:28 5 West Concord f 1:53 f 4:13 f 6:28 f 8:38 f 12:13 6 South Acton 9:24 11:34 1:59 4:19 6:34 8:44 12:19 7 Littleton/Rte495 9:31 11:41 2:06 4:26 6:41 8:51 12:26 2:13 8 Ayer 9:38 11:48 4:33 6:48 8:58 12:33 8 Shirley f 9:43 f 11:53 f 2:18 f 4:38 f 6:53 f 9:03 f 12:38 North Leominster 9:51 12:01 2:26 4:46 7:01 9:11 12:46 8 Fitchburg 10:01 12:11 2:36 4:58 7:11 9:21 12:56

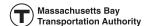
12:21

Times in purple with "f" indicate a flag stop: Passengers must tell the conductor that they wish to leave. Passengers waiting to board must be visible on the platform for the train to stop.



Bikes: Bicycles are allowed on trains with the bicycle symbol shown below the train number.









This schedule will be effective from November 2, 2020 and will replace the schedule of June 22, 2020.

New Year's Day, Thanksgiving Day, and Christmas Day operate on a Sunday schedule. For all holiday schedules, visit MBTA.com or call 617-222-3200.

Ski Train: Wachusett Mountain Ski Area operates a shuttle between the Ski Area and Wachusett Station during winter months to connect to these trains, which feature a specially modified coach equipped with racks for ski and snowboard equipment. Please visit MBTA.com for updated information about this service.

8 10:11

Schedules may change in the event of severe weather

During weather events, the symbols to the right will communicate service level and impact on passengers. These modified schedules are available on MBTA.com.



REGULAR SCHEDULE

5:08

Trains will operate on a normal schedule

7:21

9:31

1:06

STORM SCHEDULE

2:46

Major changes to the regular schedule. Schedules will be available on mbta.com, and in Boston stations.













No passenger service on Commuter Rail.

8 Wachusett



Ride Safer. Wear

mbta.com/



Maintain a healthy distancé

Practice good hygiene

Reduced service schedule in effect when declared in advance by the MBTA. In most cases, announcement made late in the afternoon on the prior day. Stay connected to MBTA.com for upto-the minute information.

FITCHBURG LINE

Monday to Friday

Monday to Friday

Inb	ound to Boston			А	М			Р	M		Out	bound from Boston		A	М			PM		
ZONE	STATION TRAI	N #	7400	1400	1402	1404	1406	1408	1410	1412	ZONE	STATION TE	RAIN#	1401	1403	1405	1407	1409	1411	1413
8	Wachusett	8	4:50	6:30	8:45	10:50	1:15	3:40	6:10	9:45	1A	North Station	8	8:35	10:45	1:10	3:30	5:45	7:55	11:30
8	Fitchburg	8	4:58	6:38	8:53	10:58	1:23	3:48	6:18	9:53	1A	Porter Square	8	8:45	10:55	1:20	3:40	5:55	8:05	11:40
8	North Leominster	8	5:05	6:45	9:00	11:05	1:30	3:55	6:25	10:00	1	Belmont		8:49	10:59	1:24	3:44	5:59	8:09	11:44
8	Shirley		5:13	6:53	9:08	11:13	1:38	4:03	6:33	10:08	1	Waverley		8:52	11:02	1:27	3:47	6:02	8:12	11:47
8	Ayer		5:18	6:58	9:13	11:18	1:43	4:08	6:38	10:13	2	Waltham	8	8:57	11:07	1:32	3:52	6:07	8:17	11:52
7	Littleton/Route 495	8	5:26	7:06	9:21	11:26	1:51	4:16	6:46	10:21	2	Brandeis/Roberts	8	9:00	11:10	1:35	3:55	6:10	8:20	11:55
6	South Acton	b	5:33	7:12	9:27	11:32	1:57	4:22	6:52	10:27	3	Kendal Green		9:04	11:14	1:39	3:59	6:14	8:24	11:59
5	West Concord	8	5:37	7:17	9:32	11:37	2:02	4:27	6:57	10:32	4	Lincoln		9:09	11:19	1:44	4:04	6:19	8:29	12:04
5	Concord		5:42	7:21	9:36	11:41	2:06	4:31	7:01	10:36	5	Concord		9:15	11:25	1:50	4:10	6:25	8:35	12:10
4	Lincoln		5:49	7:26	9:41	11:46	2:11	4:36	7:06	10:41	5	West Concord	8	9:18	11:28	1:53	4:13	6:28	8:38	12:13
3	Kendal Green		5:57	7:31	9:46	11:51	2:16	4:41	7:11	10:46	6	South Acton	8	9:24	11:34	1:59	4:19	6:34	8:44	12:19
2	Brandeis/Roberts	8	6:01	7:35	9:50	11:55	2:20	4:45	7:15	10:50	7	Littleton/Route 49	5 &	9:31	11:41	2:06	4:26	6:41	8:51	12:26
2	Waltham	b	6:05	7:39	9:54	11:59	2:24	4:49	7:19	10:54	8	Ayer		9:38	11:48	2:13	4:33	6:48	8:58	12:33
1	Waverley		6:10	7:43	9:58	12:03	2:28	4:53	7:23	10:58	8	Shirley		9:43	11:53	2:18	4:38	6:53	9:03	12:38
1	Belmont		6:13	7:46	10:01	12:06	2:31	4:56	7:26	11:01	8	North Leominster	8	9:51	12:01	2:26	4:46	7:01	9:11	12:46
1A	Porter Square	8	6:18	7:51	10:06	12:11	2:36	5:01	7:31	11:06	8	Fitchburg	8	10:01	12:11	2:36	4:58	7:11	9:21	12:56
1A	North Station	8	6:30	8:01	10:16	12:21	2:46	5:11	7:41	11:16	8	Wachusett	8	10:11	12:21	2:46	5:08	7:21	9:31	1:06

Reduced Schedule advisory system The MBTA and Keolis closely monitor events to determine if changes to the Commuter Rail schedule are needed. During this time, the symbols to the right will communicate service level and impact on passengers.



REGULAR SCHEDULE Trains will operate on a normal schedule.

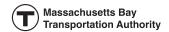


REDUCED SCHEDULE

Major changes to the regular schedule. Schedules also available on MBTA.com and in Boston stations.



NO SERVICE No passenger service on Commuter Rail.



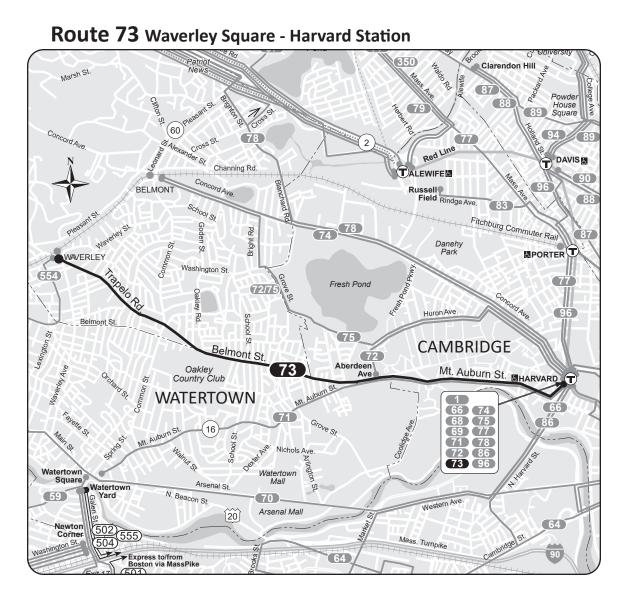












Effective December 20, 2020 Waverley Square-Harvard Station Serving • Mount Auburn Hospital • Cushing Square Harvard University • Red Line • Fitchburg Commuter Rail Massachusetts Bay Transportation Authority Massachusetts Department of Transportation Information 617-222-3200 • 1-800-392-6100 (TTY) 617-222-5146 • www.mbta.com

73		Wee	kday			73		Satu	rday			73			Sunda	ıy		
	Inbound			Outbound			Inbound			Outbound			Inbound	d			Outbound	
Leave Waverley Square	Arrive Mt. Auburn Bridge	Arrive Harvard	Leave Harvard	Arrive Mt. Auburn Bridge	Arrive Waverley Square	Leave Waverley Square	Arrive Mt. Auburn Bridge	Arrive Harvard	Leave Harvard	Arrive Mt. Auburn Bridge	Arrive Waverley Square	Leave Waverley Square	Arrive Mt. Aubu Bridge	rn Arriv Harva		eave arvard	Arrive Mt. Auburn Bridge	Arı n Wav Sqı
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7:35 7:41	7:45 7:51	7:52 7:58	7:43 7:59	7:52 8:08	8:03 8:19	12:55	1:03	1:10	12:27	12:35	12:43	Charlie	eTicket	\$1.70	\$1.70	\$2.4	0 \$4.	10*
7:47	7:57	8:04	8:14	8:23	8:34	1:10	1:18	1:25	12:47	12:55	1:03	Cash-c	on-Board	\$1.70	\$3.40	\$2.4	0 \$4.	10
7:56	8:06	8:13	8:31	8:40	8:51	1:18	1:26	1:33	w 1:02	1:10	1:18	Stude	nt/Youth*	* \$0.85	\$0.85	\$1.1	0 \$1.	10
8:06 8:22	8:16 8:32	8:23 8:38	8:48 9:08	8:57 9:17	9:08 9:28		w - Waits fo	or last train to	arrive at Ha	rvard Station	١.	Senior	/TAP***	\$0.85	\$0.85	\$1.1	0 \$1.	10
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1:41

1:26

1:35

w 1:08

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12:23 12:36 17 Mins. or Less

Arrive Waverley Square

6:47A

7:07 7:27

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11:09 11:26

11:44 12:01P 12:19

9:00 9:17 9:34 9:52 10:06 10:27

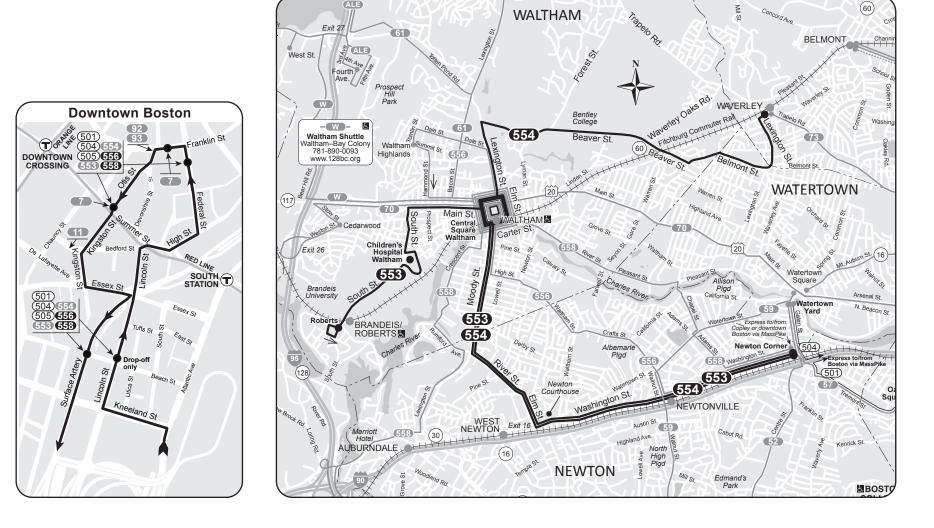
10:46 11:06

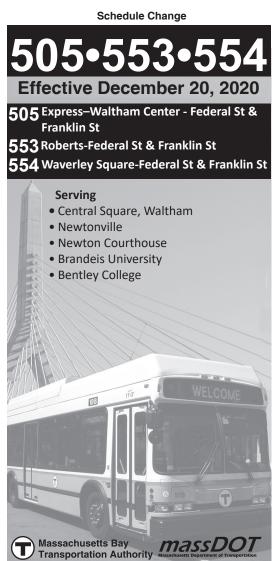
11:26

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12:08A 12:28 12:48 1:08 1:27

Route 505 Express – Waltham Center - Federal St & Franklin St Route 553 Roberts - Federal St & Franklin St Route 554 Waverley Square - Federal St & Franklin St





Information 617-222-3200 • 1-800-392-6100 (TTY) 617-222-5146 • www.mbta.com

Route 505 service may be limited or suspended.

For schedules, alerts and updates, visit: mbta.com/schedules/505

Route 505
Outer Express BusCentral Square, WalthamDowntown Boston

Inbound bus stops before the route enters the highway are pickup only. Outbound bus stops after the route exits the highway are dropoff only. For local travel, please use Route 553, 554, or 558.

553 &	554	Inbound		Week	kday	Ó	Outbound		553	Satur	day
Leave Wa	∟eave averley quare	Lv/Arrive Central Sq. Waltham	Arrive Newton Corner		Leave Newton Corner	Arrive Central Sq. Waltham	Arrive Waverley Square	Arrive Roberts	Leave Roberts	Arrive Central Sq. Waltham	Arrive Newton Corner
7:10 7:55 8 8:40 9:25 9 10:10 10:55 11:40 12:25P 1:10 1:55 2:40 3:25	6:44A 8:17 9:43 	6:34A 6:56 7:19 8:08 8:33 8:50 9:35 9:56 10:20 11:05 11:50 12:36P 1:21 2:06 2:51 3:36	6:58A 7:23 7:45 8:34 8:57 9:13 9:57 10:18 10:42 11:25 12:10P 12:59 1:44 2:29 3:14 3:59		6:00A 6:30 7:05 7:30 7:55 8:40 9:05 9:20 10:05 10:50 11:35 12:15P 1:05 1:50 2:35 3:20	6:21A 6:46 7:21 7:51 8:14 8:59 9:24 9:41 10:26 11:11 11:55 12:34P 1:24 2:09 2:59 3:44	6:39A 8:09 9:37 	7:00A 7:36 8:30 9:17 9:59 10:44 11:29 12:12P 12:54 1:44 2:29 3:21 4:06	6:30A 7:30 8:40 9:50 11:15 12:45P 2:15 3:40 5:10 6:35	6:38A 7:38 8:50 10:00 11:27 12:58P 2:27 3:52 5:22 6:45	6:56A 7:56 9:09 10:23 11:50 1:22P 2:49 4:14 5:45 7:06
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7:55 8:40	7:25 	7:40 8:05 8:50	8:01 8:25 9:10		7:00 7:45 8:30	7:27 8:12 8:57		7:43 8:28 9:13	7:00A 8:05 9:15 10:35 12:00N 1:30 3:00 4:25 5:55 7:15	7:13A 8:18 9:30 10:53 12:21P 1:50 3:19 4:43 6:13 7:31	7:26A 8:31 9:44 11:09 12:37P 2:05 3:33 4:57 6:27 7:45
				indicated	d by shad				7.13	7.01	7.45
	erts	Route 59 or Centi Downto	al Squa			Wave	Route 59 erley So ntown E	quare-			

No Route 553 service on Sunday

No Route 554 service on weekends

All buses are accessible to persons with disabilities

Outer Express-Route 505 fares

Fare	Outer Express	Outer Express + Local Bus	Outer Express + Subway
CharlieCard	\$5.25	\$5.25	\$5.25
CharlieTicket	\$5.25	\$6.95	\$7.65
Cash-on-Board	\$5.25	\$6.95	\$7.65
Student/Youth*	\$2.60	\$2.60	\$2.60
Senior/TAP**	\$2.60	\$2.60	\$2.60

FREE FARES: Children 11 and under ride free when accompanied by a paying customer; Blind Access CharlieCard holders ride free and if using a guide, the guide rides free.

* Requires Student CharlieCard or Youth CharlieCard. Student CharlieCards available

- Requires Student CharlieCard or Youth CharlieCard. Student CharlieCards available to students through participating middle and high schools. Youth CharlieCards available through community partners across Greater Boston.
- ** Requires Senior/TAP CharlieCard, available to Medicare cardholders, seniors 65+, and persons with disabilities.

NO local fare applies for any portion of Route 505

Route 553 & 554 fares

Fare	Inner Express	Inner Express + Local Bus	Inner Express + Subway
CharlieCard	\$4.25	\$4.25	\$4.25
CharlieTicket	\$4.25	\$5.95	\$6.65
Cash-on-Board	\$4.25	\$5.95	\$6.65
Student/Youth*	\$2.10	\$2.10	\$2.10
Senior/TAP**	\$2.10	\$2.10	\$2.10

FREE FARES: Children 11 and under ride free when accompanied by a paying customer; Blind Access CharlieCard holders ride free and if using a guide, the guide rides free.

* Requires Student CharlieCard or Youth CharlieCard. Student CharlieCards available

- Requires Student CharlieCard or Youth CharlieCard. Student CharlieCards availab to students through participating middle and high schools. Youth CharlieCards available through community partners across Greater Rotton.
- available through community partners across Greater Boston.

 ** Requires Senior/TAP CharlieCard, available to Medicare cardholders, seniors 65+, and persons with disabilities.

Winter 2021 Holidays 12/25/20 & 1/1/21 Sun; 1/18/21 & 2/15/21: Sat



WEEKDAY EVENING PEAK HOUR SITE DRIVE SITE DRIVE B SITE DRIVE A OLMSTED 60 PRAPELO STREET 60 **PLEASANT** 17-POAD **SATURDAY MIDDAY PEAK HOUR** SITE DRIVE SITE DRIVE B SITE DRIVE A OLMSTED 60 STREET **PLEASANT** 26-



Marijuana Dispensary 1010 Pleasant Street Peak Hour Traffic Volumes

Figure A1



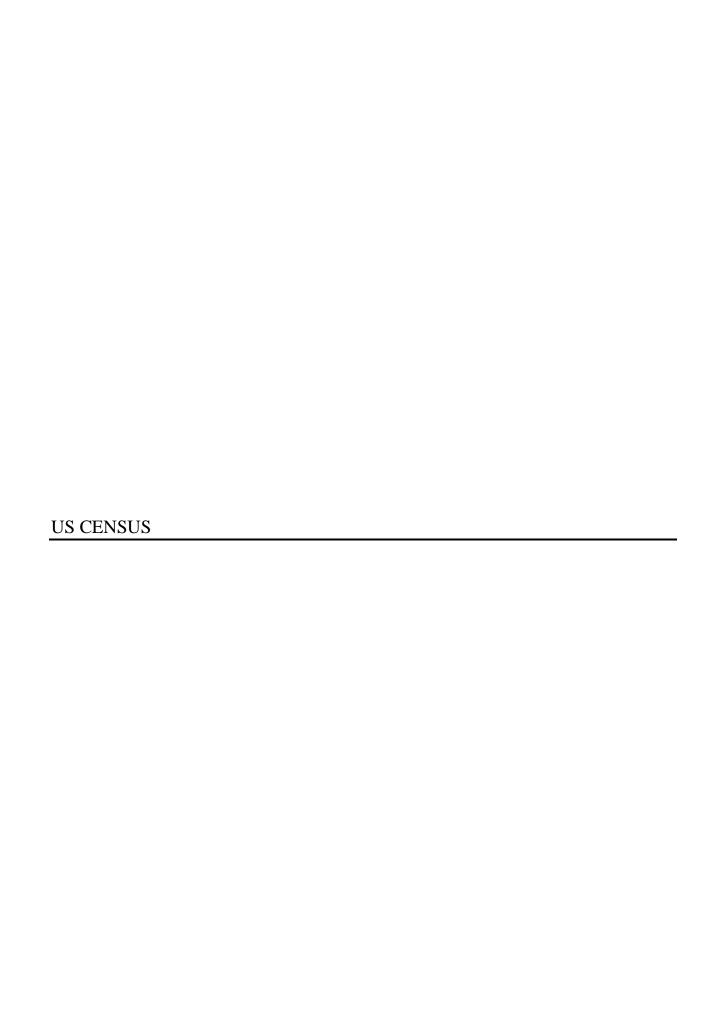
General Background Traffic Growth - Daily Traffic Volumes

LOCATION							Average
ID	Location	2015	2016	2017	2018	2019	Annual Growth
4119	YANKEE DIVISION HIGHWAY SOUTH OF WINTER ST.INTERSTATE 95		164,855	168,019	172,064	171,892	1.49
8098	INTERSTATE 93 AT MEDFORD	148,021	152,241	151,784	155,803	147,366	0.13

0.81

Say 1%

2015	2015	2015	2015	2016	2016	2016	2017	2017	2018	
	2017					•				
1.02851	1.01263	1.01723	0.99889							1.01489818 1.00127636



COMMUTING CHARACTERISTICS BY SEX



Note: This is a modified view of the original table produced by the U.S. Census Bureau. This download or printed version may have missing information from the original table.

	Census Tract 3577, Middlesex County, Ma	ssachusetts
	Total	
Label	Estimate	Margin of Error
➤ Workers 16 years and over	1,892	±195
▼ MEANS OF TRANSPORTATION TO WORK		
➤ Car, truck, or van	67.1%	±6.7
Drove alone	60.6%	±6.6
∨ Carpooled	6.5%	±3.8
In 2-person carpool	6.0%	±3.5
In 3-person carpool	0.5%	±0.8
In 4-or-more person carpool	0.0%	±1.8
Workers per car, truck, or van	1.05	±0.03
Public transportation (excluding taxicab)	20.5%	±5.2
Walked	2.3%	±2.4
Bicycle	3.1%	±2.3
Taxicab, motorcycle, or other means	2.6%	±2.1
Worked from home	4.4%	±2.3
➤ PLACE OF WORK		
➤ Worked in state of residence	100.0%	±1.8
Worked in county of residence	59.2%	±7.4
Worked outside county of residence	40.8%	±7.4
Worked outside state of residence	0.0%	±1.8
✓ Living in a place	100.0%	±1.8
Worked in place of residence	15.3%	±5.0
Worked outside place of residence	84.7%	±5.0
Not living in a place	0.0%	±1.8
➤ Living in 12 selected states	100.0%	±1.8
Worked in minor civil division of residence	15.3%	±5.0
Worked outside minor civil division of residence	84.7%	±5.0
Not living in 12 selected states	0.0%	±1.8
➤ Workers 16 years and over who did not work from home	1.809	+193

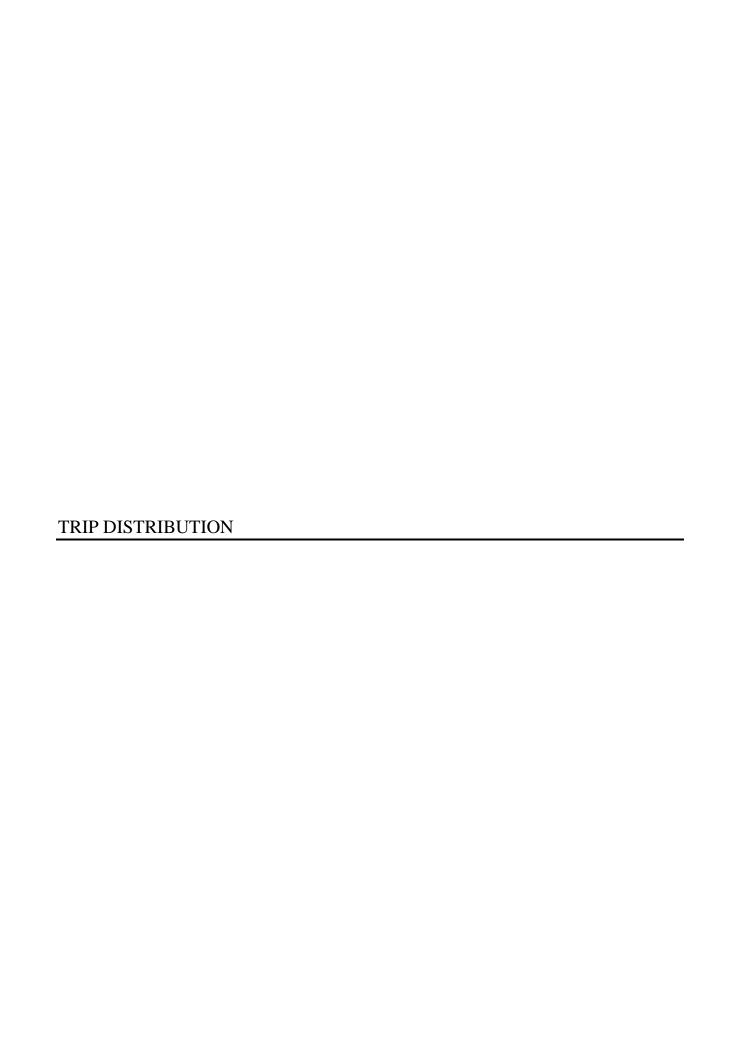


Table 3. Residence MCD/County to Workplace MCD/County Commuting Flows for the United States and Puerto Rico Sorted by Residence Geography: 5-Year ACS, 2011-2015

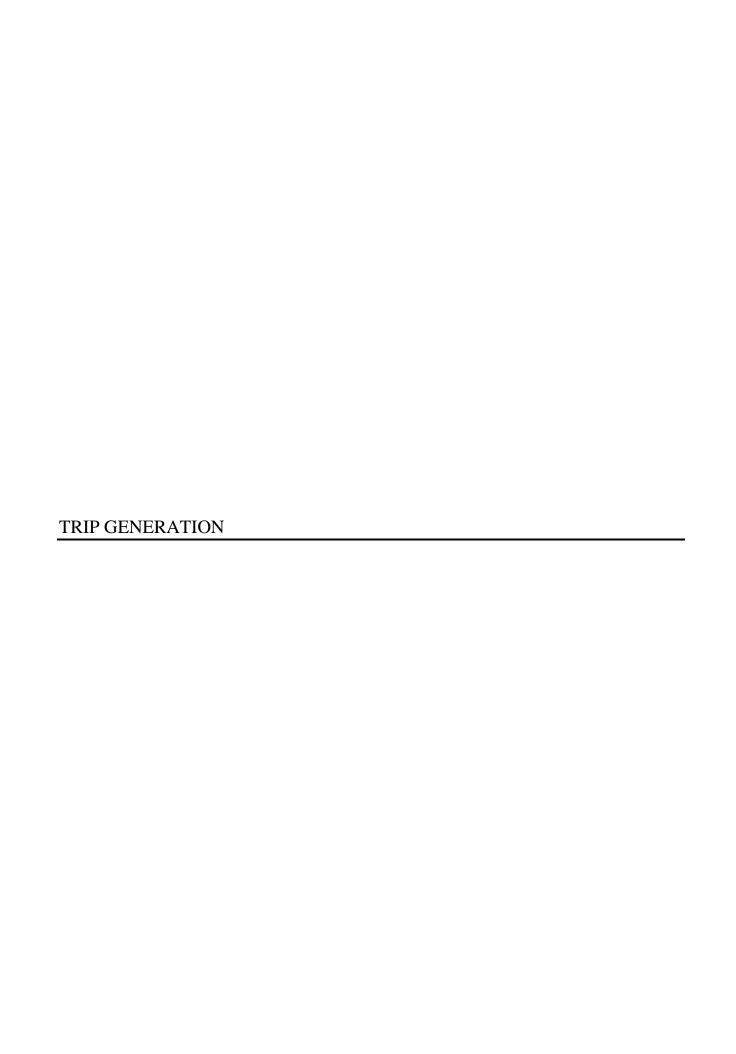
For more information on sampling and estimation methods, confidentiality protection, and sampling and nonsampling errors, see http://www2.census.gov/programs-surveys/acs/tech_docs/accuracy/MultiyearACSAccuracyofData2015.pdf. Universe: Workers 16 years and over.

Commuting flows are sorted by residence state, residence county, and residence minor civil division.

Residence	Place of Work	Commuting F	low
Minor Civil Division Name	Minor Civil Division Name	Workers in Commuting Flow	Margin of Error
Belmont town	Boston city	3,254	268
Belmont town	Belmont town	2,410	283
Belmont town	Cambridge city	2,286	202
Belmont town	Watertown Town city	562	128
Belmont town	Waltham city	557	116
Belmont town	Newton city	453	99
Belmont town	Lexington town	334	96
Belmont town	Arlington town	239	99
Belmont town	Burlington town	223	64
Belmont town	Somerville city	150	63
Belmont town	Framingham town	123	60
Belmont town	Needham town	114	61
Belmont town	Bedford town	104	43
Belmont town	Medford city	94	47
Belmont town	Woburn city	92	56
Belmont town	Brookline town	88	34
Belmont town	Concord town	87	52
Belmont town	Everett city	83	70
Belmont town	Billerica town	73	42
		12,779	

				Matri	K %		
_		Exit				Enter	
	Trapelo Road (West/ North)	Trapelo Road (East/ South)	Pleassant Street (East)	%	Trapelo Road (West/ North)	Trapelo Road (East/ South)	Pleassant Street (East)
25.5%		0.5	0.5	1		0.5	0.5
18.9%		0.5	0.5	1		0.5	0.5
17.9%		0.8	0.2	1		0.5	0.5
4.4%		1		1		1	
4.4%	0.7	0.3		1	0.7	0.3	
3.5%	0.3	0.7		1	0.3	0.7	
2.6%	0.9		0.1	1	0.9		0.1
1.9%			1	1			1
1.7%	1			1	1		
1.2%			1	1			1
1.0%	0.8	0.2		1	0.8	0.2	
0.9%	0.8	0.2		1	0.8	0.2	
0.8%	1			1	1		
0.7%			1	1			1
0.7%	1			1	1		
0.7%		1		1		1	
0.7%	1			1	1		
0.6%			1	1			1
0.6%	1			1	1		

				Trips				
		Exit		_		Enter		_
%	Trapelo Road (West/ North)	Trapelo Road (East/ South)	Pleassant Street (East)	total	Trapelo Road (West/ North)	Trapelo Road (East/ South)	Pleassant Street (East)	total
1	0	1627	1627	3254	0	1627	1627	3254
1	0	1205	1205	2410	0	1205	1205	2410
1	0	1829	457	2286	0	1143	1143	2286
1	0	562	0	562	0	562	0	562
1	390	167	0	557	390	167	0	557
1	136	317	0	453	136	317	0	453
1	301	0	33	334	301	0	33	334
1	0	0	239	239	0	0	239	239
1	223	0	0	223	223	0	0	223
1	0	0	150	150	0	0	150	150
1	98	25	0	123	98	25	0	123
1	91	23	0	114	91	23	0	114
1	104	0	0	104	104	0	0	104
1	0	0	94	94	0	0	94	94
1	92	0	0	92	92	0	0	92
1	0	88	0	88	0	88	0	88
1	87	0	0	87	87	0	0	87
1	0	0	83	83	0	0	83	83
1	73	0	0	73	73	0	0	73
	1595	5842	3889	11326	1595	5157	4574	11326
	14%	52%	34%		14%	46%	40%	
۱Y	15	45	40	100	15	45	40	100





Trip Generation Handbook 3rd Edition

SEPTEMBER 2017

INSTITUTE OF TRANSPORTATION ENGINEERS

4.3 Basis for Recommended Process

The recommended approach for using information from *Trip Generation Manual* data pages to estimate trip generation for a study site is based on the following statements:

- The value of the independent variable for the study site must be within the range of data included to use the data plot:
- When the data plot has at least 20 data points and a fitted curve equation are provided, the fitted curve equation should be used;
- A fitted curve equation with an R² of at least 0.75 is appropriate to use because it indicates the
 recommended acceptable level of correlation between trips generated by a site and the value
 measured for an independent variable;
- A weighted average rate is appropriate to use when the weighted standard deviation is less than
 or equal to 55 percent of the weighted average rate;
- The use of supplemental local data is suggested when the data plot has fewer than six data points; and
- The number of trips determined by either the rate or the equation should be within the cluster of data points (that is, the range of trip values) found at the study site's independent variable value.
 Otherwise, collecting and using additional local data is suggested.

A detailed step-by-step approach for using *Trip Generation Manual* data is presented in Section 4.4 of this chapter.

4.4 Process for Selecting Average Rate or Equation in Trip Generation Manual Data

A step-by-step procedure is described below for determining how best to estimate trip generation using data contained in *Trip Generation Manual*. These guidelines are merely tools to help the analyst estimate trip generation. These tools are by design straightforward and uncomplicated. They do not include all considerations that could be relevant to a particular situation. Thus, professional judgment must be applied at all stages in this analysis process. The procedure is also outlined with simplified text in the flow chart in Figure 4.2.

- 4.4.1—Step 1: Determine if the study site is consistent with the description of a land use code in *Trip Generation Manual* and with the described or presumed characteristics of development sites for which data points are provided.
 - If the answer is yes, proceed to Step 2.
 - If the answer is no, collect local data for the land use being analyzed and establish a local or consolidated rate. Refer to Chapter 9 for guidance.
- 4.4.2—Step 2: Determine if the size of the study site (in terms of the unit of measurement of the independent variable) is within the range of the data shown in the data plot.
 - If the answer is yes, proceed to Step 3.
 - If the answer is no, either (1) consider the use of a different independent variable and its associated data pages, or (2) collect local data and establish a local or consolidated rate. Refer to Chapter 9 for guidance.

Time Period/Direction	ITE Senior housing (93 units) a	Transit Reduction Trips 0.05	Total	ITE Multifamily Housing (57 units) b	Transit Reduction Trips 0.1	Total	TOTAL	ТММА
Average Weekday	348.49	17.42	331.07	308.90	30.89	278.01	609.08	1,148
Weekday Morning Peak Hour								
Entering	6.45	0.32	6.13	5.34	0.53	4.81	10.94	
<u>Exiting</u>	<u>11.97</u>	<u>0.60</u>	<u>11.37</u>	<u>15.18</u>	<u>1.52</u>	<u>13.66</u>	<u>25.03</u>	
Total	18.42	0.92	17.50	20.52	2.05	18.47	35.97	36
Weekday Evening Peak Hour								
Entering	13.27	0.66	12.61	15.3	1.53	13.77	26.38	
Exiting	<u>11.31</u>	<u>0.57</u>	<u>10.74</u>	<u>9.78</u>	<u>0.98</u>	<u>8.80</u>	<u>19.54</u>	
Total	24.58	1.23	23.35	25.08	2.51	22.57	45.92	92

Institute of Transportation Engineers (ITE) Trip Generation, 10th Edition

Land Use Code (LUC) 221 - Multifamily Housing (Mid-Rise)

```
'ehicle Trips Ends vs:
                                              Dwelling Units
ndent Variable (X):
                                         57
   R^2
                 Equation
   0.77
          AVERAGE WEEKDAY DAILY
             T = 5.45 * (X) - 1.75
             T = 5.45 *
                            57
                                   -(1.75)
             T = 308.90
             T = 308
                         vehicle trips
               with 50% ( 154.45 vpd) entering and 50% ( 154.45 vpd) exiting.
                 Rate
          WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC
   0.67
             T = 0.36 * (X)
             T = 0.36 *
                            57
             T = 20.52
             T = 21
                        vehicle trips
               with 26% ( 5.34 vph) entering and 74% ( 15.18 vph) exiting.
                Rate
  0.72
          WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC
          T =
                0.44 * (X)
          T =
                0.44 *
                            57
          T =
                25.08
            T = 25
                        vehicle trips
               with 61% ( 15.30 vph) entering and 39% ( 9.78 vph) exiting.
```

Institute of Transportation Engineers (ITE) *Trip Generation,* 10th Edition

Land Use Code (LUC) 252 - Senior Adult Housing - Attached

Average Vehicle Trips Ends vs: Dwelling Units Independent Variable (X): 93

R² Equation

0.99 AVERAGE WEEKDAY DAILY

T = 4.02 * (X)-25.37

T = 4.02 * 93 -25.370

T = 348.49

T = 348 vehicle trips

with 50% (174 vph) entering and 50% (174 vph) exiting.

Equation

0.98 WEEKDAY MORNING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 0.20 * (X) -0.18

T = 0.20 * 93 -0.18

T = 18.42

T = 18 vehicle trips

with 35% (6.45 vph) entering and 65% (11.97 vph) exiting.

Equation

WEEKDAY EVENING PEAK HOUR OF ADJACENT STREET TRAFFIC

T = 0.24*(X)+2.26

T = 0.24 * 93 +2.26

T = 24.58

0.96

T = 25 vehicle trips

with 54% (13.27 vph) entering and 46% (11.31 vph) exiting.



Start Time: 12:00:00 AM Site Code: 82890001 Location: Boston Post Road Location: West of Village Drive City/State: Marlborough, MA

Start Date: 6/13/2019

2020 Raw

	Olmsted Dr From North			ant St East	Pleasant St From West		
Start Time	Left Right		Thru	Right	Left	Thru	
7-8 AM		9	307	1	2	283	
8-9 AM	2	2	344	2	2	272	
9-10 AM	3	7	293	2	3	229	
10-11 AM	3	10	252	6	6	241	
11-12 PM	3	3	244	5	5	252	
12-1 PM	3	8	291	2	3	256	
1-2 PM	6	4	260	7	8	309	
2-3 PM	2	5	355	0	5	294	
3-4 PM	6	10	357	8	11	354	
4-5 PM	2	2 5		1	6	387	
5-6 PM	1	4	358	3	3	339	
6-7 PM	7	7	243	5	9	232	

Covid Adj - 2020 Baseline Condition COVID adj 1.4

1% ab	ove no sea	isonal adju	stment			
		ted Dr North		ant St East		ant St West
Time	Left Right		Thru	Right	Left	Thru
7-8 AM	4	13	430	1	3	396
8-9 AM	3	3	482	3	3	381
9-10 AM	4	10	410	3	4	321
10-11 AM	4	14	353	8	8	337
11-12 PM	4	4	342	7	7	353
12-1 PM	4	11	407	3	4	358
1-2 PM	8	6	364	10	11	433
2-3 PM	3	7	497	0	7	412
3-4 PM	8	14	500	11	15	496
4-5 PM	3	7	547	1	8	542
5-6 PM	1	6	501	4	4	475
6-7 PM	10	10	340	7	13	325

2021 Adjustment

1 year 1.01

					jou.		
	1 percent	per year co	ompounde	d annual b	ackground		
		ted Dr North		ant St East	Pleasant St From West		
Time	Left	Right	Thru	Right	Left	Thru	
7-8 AM	4	13	434	1	3	400	
8-9 AM	3	3	487	3	3	385	
9-10 AM	4	10	414	3	4	324	
10-11 AM	4 14		357	8	8	340	
11-12 PM	4	4	345	7	7	357	
12-1 PM	4	11	411	3	4	362	
1-2 PM	8	6	368	10	11	437	
2-3 PM	3	7	502	0	7	416	
3-4 PM	8	14	505	11	15	501	
4-5 PM	3 7		552	1	8	547	
5-6 PM	1 6		506	4	4	480	
6-7 PM	10	10	343	7	13	328	



Source: ITE Trip Generation Manual, 10th Edition

Land Use Code 882

Land Use Marijuana Dispensary Setting General Urban/Suburban

Time Period Weekday

Time Period	wee	ekuay				
Trip Type	Vel	nicle	Daily		Daily	
# Data Sites		4	303	303	303	303
	% of 24-H	lour Traffic	Entering	Exiting	Entering	Exiting
Time	Entering	Exiting				
12-1 AM	0	0	0.0	0.0	0	0
1-2 AM	0	0	0.0	0.0	0	0
2-3 AM	0	0	0.0	0.0	0	0
3-4 AM	0	0	0.0	0.0	0	0
4-5 AM	0			0.0	0	0
5-6 AM	0.1	0.1	0.3	0.3	0	0
6-7 AM	0.4	0.1	1.2	0.3	1	0
7-8 AM	1.4	0.2	4.2	0.6	4	1
8-9 AM	4.1	3.6	12.4	10.9	12	11
9-10 AM	5.3	4.3	16.1	13.0	16	13
10-11 AM	8.2	7.5	24.8	22.7	25	23
11-12 PM	8.1	7.9	24.5	23.9	25	24
12-1 PM	9.1	8.9	27.6	27.0	28	27
1-2 PM	8.3	8.0	25.1	24.2	25	24
2-3 PM	8.4	9.3	25.5	28.2	26	28
3-4 PM	9.9	9.6	30.0	29.1	30	29
4-5 PM	11.3	11.4	34.2	34.5	34	34
5-6 PM	12.4	12.4	37.6	37.6	38	38
6-7 PM	12.5	14.7	37.9	44.5	38	45
7-8 PM	0.2	1.6	0.6	4.8	1	5
8-9 PM	0.1	0.2	0.3	0.6	0	1
9-10 PM	0	0.1	0.0	0.3	0	0
10-11 PM	0	0	0.0	0.0	0	0
11-12 AM	0	0	0.0	0.0	0	0
					303	303
Data obtaine	d from the IT	F Canaral I	Irhan/Suhu	rhan - T	rine by tim	of day

Background Development											
	out										
	From	North	From West								
Start Time	Left	Right	Thru	Right	Left	Thru					
%	0	0	0.65	0	0	0.65					
7-8 AM	0	0	1	0	0	3					
8-9 AM	0	0	7	0	0	8					
9-10 AM	0	0	8	0	0	10					
10-11 AM	0	0	15	0	0	16					
11-12 PM	0	0	16	0	0	16					
12-1 PM	0	0	18	0	0	18					
1-2 PM	0	0	16	0	0	16					
2-3 PM	0	0	18	0	0	17					
3-4 PM	0	0	19	0	0	20					
4-5 PM	0	0	22	0	0	22					
5-6 PM	0	0	25	0	0	25					
6-7 PM	0	0	29	0	0	25					

2028 No- Build												
7 year 1.01												
1 percent per year compounded annual background												
		ted Dr North		ant St East		ant St West						
Start Time	Left	Right	Thru	Right	Left	Thru						
7-8 AM	4	13	466	1	3	432						
8-9 AM	3	3	529	3	3	421						
9-10 AM	4	10	452	3	4	357						
10-11 AM	4	14	398	8	8	381						
11-12 PM	4	4	386	7	7	399						
12-1 PM	4	11	459	3	4	406						
1-2 PM	8	6	411	10	11	485						
2-3 PM	3	7	556	0	7	463						
3-4 PM	8	14	560	11	15	557						
4-5 PM	3	7	614	1	8	608						
5-6 PM	1	6	568	4	4	540						
6-7 PM	10	10	397	7	13	377						

Data obtained from the ITE General Urban/Suburban - Trips by time of day



La

Hourly Distribution of Entering and Exiting Vehicle

Source: ITE Trip Generation Manual, 10th Edition

	221 Multifamily Ho General Urba Wee	an/Suburban	ise) 332				Land Use Code Land Use t Setting U Time Period /	Housin Jrban/S	Suburba		278				610
# Data Sites	Veh	iolo					Trip Type '	Vohiolo						305	305
# Data Sites	Ven		Da	ilv	Da	ilv	# Data Sites	verlicie	1	D	aily	Da	ilv		otal
	% of 24-H		166	166	166	166		4-Hour	ı Traffic	139	139	139	139	10	itai
	70 OI 24-II	oui manic	100	100	100	100	70 OI 2-	+-1 10ui	Trailic	100	100	100	100		
Time	Entering	Exiting	Entering	Exitina	Entering	Exiting	Time F	nterino	Exiting	Enterino	Exiting	Entering	Exiting	Entering	Exiting
11110	Lintoning	Exiting	Lintolling	Extung	Lintoning	LAiting	11110 2		Landing	Lincolnie	LXIIII	Lintoning	Exiting	Lintorning	Exiting
12-1 AM	0.7	0.3	1.2	0.5	1	0	12-1 AM	0.3	0.4	0.4	0.6	0	1	1	1
1-2 AM	0.3	0.2	0.5	0.3	0	0	1-2 AM	0.2	0.5	0.3	0.7	0	1	0	1
2-3 AM	0.2	0.2	0.3	0.3	0	0	2-3 AM	0	0.1	0.0	0.1	0	0	0	0
3-4 AM	0.4	0.3	0.7	0.5	1	0	3-4 AM	0.1	0.2	0.1	0.3	0	0	1	0
4-5 AM	0.3	8.0	0.5	1.3	0	1	4-5 AM	0.2	0.5	0.3	0.7	0	1	0	2
5-6 AM	0.6	2.7	1.0	4.5	1	4	5-6 AM	0.9	2.2	1.3	3.1	1	3	2	7
6-7 AM	1.5	6.5	2.5	10.8	2	11	6-7 AM	1.3	3.0	1.8	4.2	2	4	4	15
7-8 AM	2.8	12.1	4.6	20.1	5	20	7-8 AM	2.1	5.1	2.9	7.1	3	7	8	27
8-9 AM	3.5	8.8	5.8	14.6	6	15	8-9 AM	3.9	6.3	5.4	8.8	5	9	11	24
9-10 AM	2.9	5.7	4.8	9.5	5	10	9-10 AM	4.7	6.7	6.5	9.3	7	9	12	19
10-11 AM	2.7	4.7	4.5	7.8	5	8	10-11 AM	6.4	7.5	8.9	10.4	9	10	14	18
11-12 PM	4.5	4.5	7.5	7.5	7	8	11-12 PM	6.8	6.5	9.5	9.0	10	9	17	17
12-1 PM	4.8	4.6	8.0	7.6	8	8	12-1 PM	8.5	9.0	11.8	12.5	12	12	20	20
1-2 PM	4.1	4.8	6.8	8.0	7	8	1-2 PM	7.7	8.0	10.7	11.1	11	11	18	19
2-3 PM	5.8	5.0	9.6	8.3	10	8	2-3 PM	9.1	6.7	12.6	9.3	13	9	23	17
3-4 PM	6.7	4.9	11.1	8.1	11	8	3-4 PM	8.7	5.7	12.1	7.9	12	8	23	16
4-5 PM	10.6	6.2	17.6	10.3	18	10	4-5 PM	8.3	6.3	11.5	8.8	12	9	30	19
5-6 PM	12.6	7.7	20.9	12.8	21	13	5-6 PM	7.3	5.7	10.1	7.9	10	8	31	21
6-7 PM	9.3	6.6	15.4	11.0	15	11	6-7 PM	6.3	5.2	8.8	7.2	9	7	24	18
7-8 PM	7.8	4.8	12.9	8.0	13	8	7-8 PM	5.8	5.2	8.1	7.2	8	7	21	15
8-9 PM	7.0	3.3	11.6	5.5	12	6	8-9 PM	4.8	4.1	6.7	5.7	7	6	19	12
9-10 PM	5.5	2.2	9.1	3.7	9	4	9-10 PM	3.1	2.2	4.3	3.1	4	3	13	7
10-11 PM	3.6	1.9	6.0	3.2	6	3	10-11 PM	2.5	1.8	3.5	2.5	3	3	9	6
11-12 AM	2.0	1.1	3.3	1.8	3	2	11-12 AM	8.0	1.1	1.1	1.5	1	2	4	4
					166	166						139	139	305	305

Data obtained from the ITE General Urban/Suburban - Trips by time of day

e Trips by Land Use

Trip Generation

	out	out		ın	in		
	From	North	From	ı East	From West		
Time	Left	Right	Thru	Right	Left	Thru	
%	0.4	0.6	0	0.4	0.6	0	
7-8 AM	11	16	0	3	5	0	
8-9 AM	10	14	0	4	7	0	
9-10 AM	8	11	0	5	7	0	
10-11 AM	7	11	0	6	8	0	
11-12 PM	7	10	0	7	10	0	
12-1 PM	8	12	0	8	12	0	
1-2 PM	8	11	0	7	11	0	
2-3 PM	7	10	0	9	14	0	
3-4 PM	6	10	0	9	14	0	
4-5 PM	8	11	0	12	18	0	
5-6 PM	8	13	0	12	19	0	
6-7 PM	7	11	0	10	14	0	

2028 Build Zone 3

	Olmsted Dr From North			ant St East	Pleasant St From West		
Start Time	Left Right		Thru	Right	Left	Thru	
7-8 AM	15	29	466	4	8	432	
8-9 AM	13	17	529	7	10	421	
9-10 AM	12	21	452	8	11	357	
10-11 AM	11	25	398	14	16	381	
11-12 PM	11	14	386	14	17	399	
12-1 PM	12	23	459	11	16	406	
1-2 PM	16	17	411	17	22	485	
2-3 PM	10	17	556	9	21	463	
3-4 PM	14	24	560	20	29	557	
4-5 PM	11	18	614	13	26	608	
5-6 PM	9	19	568	16	23	540	
6-7 PM	17	21	397	17	27	377	



Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use

Source: ITE Trip Generation Manual, 10th Edition

Land Use Code 710

Land Use General Office Building Setting General Urban/Suburbar

Daily number obtaneid from TMA

Time Period Weekday

Trip Type							
# Data Sites			Da	ily	Da	aily	
			892	892	892	892	
	Entering Exiting						
Time	Entering	Exiting	Entering	Exiting	Entering	Exiting	
	Ü	Ū		Ü			
12-1 AM	0.2	0.1	1.8	0.9	2	1	
1-2 AM	0	0.1	0.0	0.9	0	1	
2-3 AM	0	0	0.0	0.0	0	0	
3-4 AM	0	0.1	0.0	0.9	0	1	
4-5 AM	0.1	0.2	0.9	1.8	1	2	
5-6 AM	0.4	0.1	3.6	0.9	4	1	
6-7 AM	4.6	0.5	41.0	4.5	41	4	
7-8 AM	13.1	1.9	116.9	16.9	117	17	
8-9 AM	14.4	3.5	128.4	31.2	129	31	
9-10 AM	6.4	4.3	57.1	38.4	57	38	
10-11 AM	5.4	5.9	48.2	52.6	48	52	
11-12 PM	6.2	10.3	55.3	91.9	55	92	
12-1 PM	10.2	10.4	91.0	92.8	91	93	
1-2 PM	9.0	6.7	80.3	59.8	81	60	
2-3 PM	8.2	6.5	73.1	58.0	73	58	
3-4 PM	7.4	8.5	66.0	75.8	66	76	
4-5 PM	5.5	15.2	49.1	135.6	49	135	
5-6 PM	4.2	15.6	37.5	139.2	38	139	
6-7 PM	1.7	2.9	15.2	25.9	15	26	
7-8 PM	0.9	2.2	8.0	19.6	8	20	
8-9 PM	0.7	1.3	6.2	11.6	6	12	
9-10 PM	0.5	1.5	4.5	13.4	4	13	
10-11 PM	0.3	2.0	2.7	17.8	3	18	
11-12 AM	0.4	0.2	3.6	1.8	4	2	
					000	000	

	Trip Generation											
	out	out		in	in							
	From	North	From	East	From West							
Time	Left	Right	Thru	Right	Left	Thru						
%	0.4	0.6	0	0.4	0.6	0						
7-8 AM	7	10	0	47	70	0						
8-9 AM	12	19	0	52	77	0						
9-10 AM	15	23	0	23	34	0						
10-11 AM	21	31	0	19	29	0						
11-12 PM	37	55	0	22	33	0						
12-1 PM	37	56	0	36	55	0						
1-2 PM	24	36	0	32	49	0						
2-3 PM	23	35	0	29	44	0						
3-4 PM	30	46	0	26	40	0						
4-5 PM	54	81	0	20	29	0						
5-6 PM	56	83	0	15	23	0						
6-7 PM	10	16	0	6	9	0						

Note: Same distribution of the zone 3 was assumed for zone 4 Developments

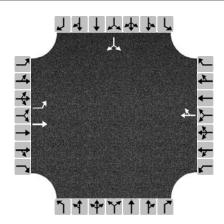
2028 Build - w/ Zone 3 and 4

		ted Dr North		ant St East	Pleasant St From West		
Start Time	Left	Right	Thru	Right	Left	Thru	
7-8 AM	22	39	466	51	78	432	
8-9 AM	25	36	529	59	87	421	
9-10 AM	27	44	452	31	45	357	
10-11 AM	32	56	398	33	45	381	
11-12 PM	48	69	386	36	50	399	
12-1 PM	49	79	459	47	71	406	
1-2 PM	40	53	411	49	71	485	
2-3 PM	33	52	556	38	65	463	
3-4 PM	44	70	560	46	69	557	
4-5 PM	65	99	614	33	55	608	
5-6 PM	65	102	568	31	46	540	
6-7 PM	27	37	397	23	36	377	





HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2021								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2021 Existing Condition								
Development - Belmont MA											
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	1								
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach		Eastbound			Westbound			Northbound			Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Number of Lanes, N	1	1	0	0	1	0	0	0	0	0	0	0	
Lane Usage	L	Т			TR						LR		
Vehicle Volumes Averages (veh/h)	7	406	0	0	435	4	0	0	0	4	0	8	
Pedestrian Averages (peds/h)		0			0		0		0				
Gap Averages (gaps/h)		0			0			0		0			
Delay (s/veh)		0.0			0.0			0.0		0.0			
Delay (veh-hrs)		0.0			0.0			0.0			0.0		
School Crossing and Roadway	Netwo	rk			·								
Number of Students in Highest Hour	0				wo or Mo	re Major	Routes Yes		Yes				
Number of Adequate Gaps in Period	0			\	Weekend Counts No								

Railroad Crossing

Number of Minutes in Period

rtain odd Grossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

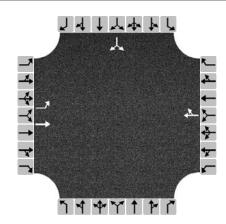
0

5-year Growth Factor (%)

0

	HCS7 Warrants Report													
Volume Si	Volume Summary													
Hour	Major	Minor	Total	Peds/h	Gaps/h	1A	1A	1B	1B	2	3A	3B	4A	4B
Hour	Volume	Volume	Volume	reus/II	Gaps/II	(100%)	(80%)	(100%)	(80%)	(100%)	(100%)	(100%)		(100%)
07 - 08	838	17	855	0	0	No	No	No	No	No	No	No	No	No
08 - 09 878 6 884 0 0 No No No No No No No												No	No	No
09 - 10													No	No
10 - 11 713 18 731 0 0 No No No No No No No													No	No
11 - 12 716 8 724 0 0 No No No No No No No													No	No
12 - 13 780 15 795 0 0 No No No No No No No													No	No
13 - 14	826	14	840	0	0	No	No	No	No	No	No	No	No	No
14 - 15	925	10	935	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1032	22	1054	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1108	10	1118	0	0	No	No	No	No	No	No	No	No	No
17 - 18	994	7	1001	0	0	No	No	No	No	No	No	No	No	No
18 - 19	691	20	711	0	0	No	No	No	No	No	No	No	No	No
Total	10246	161	10407	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1: E	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	m Vehicula	ar Volume:	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d)r					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	ularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	our-Hou	r Vehicul	ar Volun	1e										
Four-Hour	· Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3: I	Peak Hou	r												
A. Peak-Ho	our Condit	ions (Minc	r delay	and min	or volume	and to	otal volum	e)or						
B. Peak-Ho	our Vehicul	lar Volume	es (Both m	ajor appro	achesar	nd highe	r minor ap	proach)						
Warrant 4: I	Pedestria	n Volum	2											
A. Four Ho	ur Volume	esor												
B. One-Ho	ur Volume	S												
Warrant 5: S	School Cr	ossing												
Gaps Same	e Period	and												
Student Vo	olumes													
Nearest Tr	affic Contr	ol Signal (optional)										✓	
Warrant 6: 0	Coordinat	ted Signa	ıl System											
Degree of Platooning (Predominant direction or both directions)														
Warrant 7: Crash Experience														
A. Adequate trials of alternatives, observance and enforcement failedand														
B. Reported crashes susceptible to correction by signal (12-month period)and														
C. 80% Volumes for Warrants 1A, 1B,or 4 are satisfied														
Warrant 8: Roadway Network														
A. Weekday Volume (Peak hour totaland projected warrants 1, 2, or 3)or														
B. Weekend Volume (Five hours total)														
Warrant 9: Grade Crossing														
A. Grade C	Crossing wi	thin 140 ft	:and											
B. Peak-Ho	our Vehicul	lar Volume	es											

HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2028								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 No-Build Condition								
Development - Belmont MA											
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	1								
Major Street Speed (mi/h)	34	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach	Eastbound			Westbound			Northbound			Southbound			
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Number of Lanes, N	1	1 1 0		0	1	0	0	0	0	0	0	0	
Lane Usage	L	Т			TR						LR		
Vehicle Volumes Averages (veh/h)	7	452	0	0	483	4	0	0	0	4	0	8	
Pedestrian Averages (peds/h)	0			0				0			0		
Gap Averages (gaps/h)		0		0		0		0			0		
Delay (s/veh)	veh) 0.0			0.0		0.0			0.0				
Delay (veh-hrs)	rs) 0.0			0.0			0.0			0.0			
School Crossing and Roadway Network													

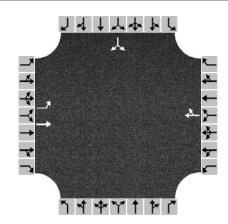
Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

Railroad Crossing

Traini Gard Grossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

	HCS7 Warrants Report													
Volume Si	Volume Summary													
Hour	Major	Minor	Total	Peds/h	Gaps/h	1A	1A	1B	1B	2	3A	3B	4A	4B
Hour	Volume	Volume	Volume	reus/II	Gaps/II	(100%)	(80%)	(100%)	(80%)	(100%)	(100%)	(100%)		(100%)
07 - 08	902	17	919	0	0	No	No	No	No	No	No	No	No	No
08 - 09	956	6	962	0	0	No	No	No	No	No	No	No	No	No
09 - 10 816 14 830 0 0 No No No No No No No													No	No
10 - 11 795 18 813 0 0 No No No No No No No													No	No
11 - 12 799 8 807 0 0 No No No No No No No													No	No
12 - 13 872 15 887 0 0 No No No No No No No													No	No
13 - 14	917	14	931	0	0	No	No	No	No	No	No	No	No	No
14 - 15	1026	10	1036	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1143	22	1165	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1231	10	1241	0	0	No	No	No	No	No	No	No	No	No
17 - 18	1116	7	1123	0	0	No	No	No	No	No	No	No	No	No
18 - 19	794	20	814	0	0	No	No	No	No	No	No	No	No	No
Total	11367	161	11528	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1: I	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	m Vehicula	ar Volume:	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	Four-Hou	r Vehicul	ar Volun	1e										
Four-Hour	r Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3: I														
	our Condit	•						•						
	our Vehicu			ajor appro	achesar	nd highe	r minor ap	proach)						
Warrant 4: I			2											
	our Volume													
	our Volume													
Warrant 5: S														
	e Period	and												
Student V		16: 17	1											
Warrant 6: 0	raffic Contr		•										✓	
					th directic	anc)								
Degree of Platooning (Predominant direction or both directions)														
Warrant 7: Crash Experience A. Adequate trials of alternatives, observance and enforcement failedand														
B. Reported crashes susceptible to correction by signal (12-month period)and														
C. 80% Volumes for Warrants 1A, 1B,or 4 are satisfied														
Warrant 8: Roadway Network														
A. Weekday Volume (Peak hour totaland projected warrants 1, 2, or 3)or														
B. Weekend Volume (Five hours total)														
			o total)											
Warrant 9: Grade Crossing A. Grade Crossing within 140 ftand														
	our Vehicu													
D. I Cak-III	Jai Veriicu	.a. voluitie	,,,											

HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2028								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 No-Build Condition								
	Development - Belmont MA										
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	0								
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach		Eastbound			Westbound			Iorthboun	ıd	Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R
Number of Lanes, N	1	1	0	0	1	0	0	0	0	0	0	0
Lane Usage	L	L T			TR						LR	
Vehicle Volumes Averages (veh/h)	18	452	0	0	483	12	0	0	0	12	0	20
Pedestrian Averages (peds/h)		0		0		0			0			
Gap Averages (gaps/h)		0			0			0		0		
Delay (s/veh)		0.0			0.0		0.0			0.0		
Delay (veh-hrs)		0.0			0.0		0.0			0.0		
School Crossing and Roadway	School Crossing and Roadway Network											

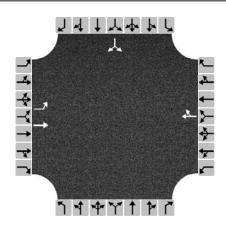
Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

Railroad Crossing

ram oud crossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

					HCS	57 Wai	rrants	Repoi	rt					
Volume S	ımmərv	,						_						
	_		T	D 1 (1	C //	1.0	1.4	10	10		2.4	20	4.4	45
Hour	Major Volume	Minor Volume	Total Volume	Peds/h	Gaps/h	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)		4B (100%)
07 - 08	910	44	954	0	0	No	No	No	No	No	No	No	No	No
08 - 09	967	30	997	0	0	No	No	No	No	No	No	No	No	No
09 - 10	828	33	861	0	0	No	No	No	No	No	No	No	No	No
10 - 11	809	36	845	0	0	No	No	No	No	No	No	No	No	No
11 - 12	816	25	841	0	0	No	No	No	No	No	No	No	No	No
12 - 13	892	35	927	0	0	No	No	No	No	No	No	No	No	No
13 - 14	935	33	968	0	0	No	No	No	No	No	No	No	No	No
14 - 15	1049	27	1076	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1166	38	1204	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1261	29	1290	0	0	No	No	No	No	No	No	No	No	No
17 - 18	1147	28	1175	0	0	No	No	No	No	No	No	No	No	No
18 - 19	818	38	856	0	0	No	No	No	No	No	No	No	No	No
Total	11598	396	11994	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1:	Eight-Hou	ır Vehicu	lar Volur	ne								$\overline{}$		
A. Minimu	m Vehicula	ar Volumes	s (Both ma	jor approa	achesan	d higher	minor ap	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand-	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	Four-Hou	r Vehicul	ar Volun	ne										
Four-Hou	r Vehicular	Volume (E	Both major	· approach	esand	higher m	inor appro	ach)						
Warrant 3: I	Peak Hou	r												
A. Peak-H	our Conditi	ions (Minc	or delay	and min	or volume	and to	otal volum	e)or						
B. Peak-H	our Vehicul	lar Volume	es (Both m	ajor appro	achesai	nd highe	r minor ap	proach)						
Warrant 4: I	Pedestria	n Volume												
A. Four Ho	our Volume	sor												
B. One-Ho	our Volume	S												
Warrant 5: S	School Cr	ossing												
Gaps Sam	e Period	and												
Student V	olumes													
Nearest Tr	affic Contr	ol Signal (optional)										✓	
Warrant 6:	Coordinat	ted Signa	l System											
Degree of	Platooning	g (Predom	inant direc	tion or bo	th directio	ons)								
Warrant 7:	Crash Exp	erience												
A. Adequa	te trials of	alternative	es, observa	ance and e	nforceme	nt failed	and							
B. Reporte	ed crashes	susceptible	e to correc	tion by sig	ınal (12-m	onth perio	od)and-							
C. 80% Vo	lumes for \	Narrants 1	A, 1B,or	r 4 are sa	ntisfied									
Warrant 8: I														
	ay Volume			nd projec	ted warra	nts 1, 2, oı	· 3)or							
	nd Volume													
Warrant 9:			<u> </u>											
	Crossing wi		:and											
	our Vehicul													

HCS7 Warrants Report										
Project Information										
Analyst	JC	Date	1/4/2021							
Agency	VAI	Analysis Year	2028							
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 Build Condition Zone 3 and							
	Development - Belmont MA		Zone 4							
Project Description	Pleasant Street at Olmsted Drive									
General										
Major Street Direction	East-West	Population < 10,000	No							
Starting Time Interval	7	Coordinated Signal System	No							
Median Type	Undivided	Crashes (crashes/year)	0							
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No							
Nearest Signal (ft)	360		_							



Approach		Eastbound	t	\	Westboun	d	N	Iorthboun	ıd	Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R
Number of Lanes, N	1	1	0	0	1	0	0	0	0	0	0	0
Lane Usage	L	Т			TR						LR	
Vehicle Volumes Averages (veh/h)	59	452	0	0	483	39	0	0	0	39	0	61
Pedestrian Averages (peds/h)		0		0		0			0			
Gap Averages (gaps/h)		0		0			0			0		
Delay (s/veh)		0.0			0.0		0.0			0.0		
Delay (veh-hrs)		0.0			0.0		0.0			0.0		
School Crossing and Roadwa	y Netwo	rk										
	_								_			

Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

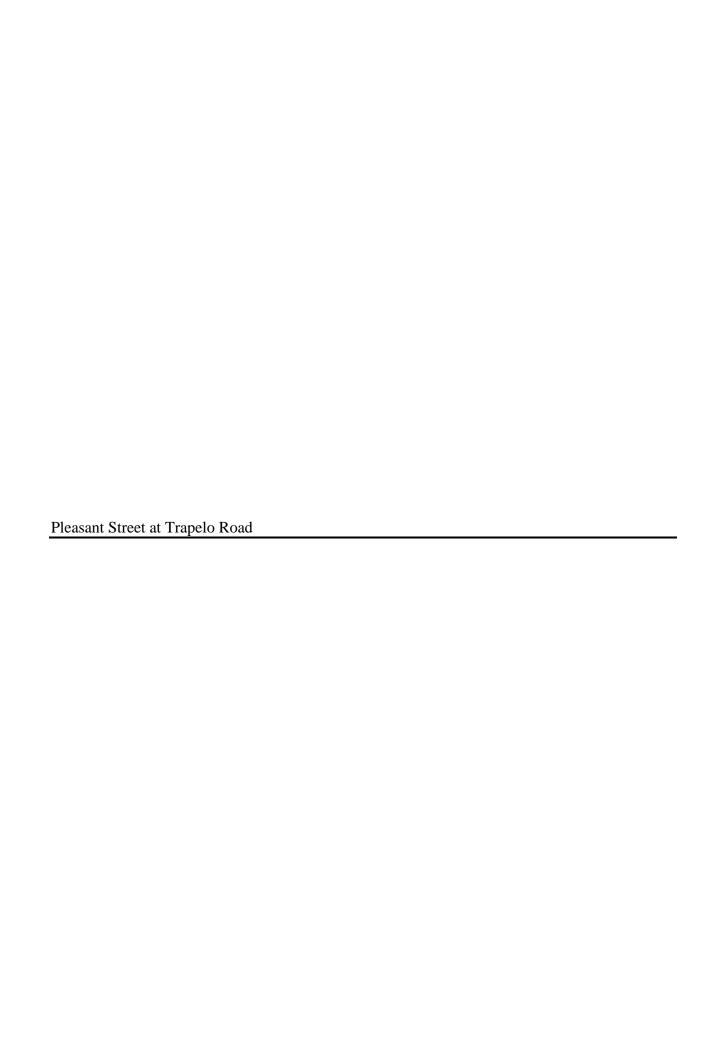
Railroad Crossing

rtain odd Grossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

					HCS	57 Wai	rran <u>ts</u>	Repo	rt					
Volume S	ummary	,						_						
	_		T	D 1 (1	C //	1.0	1.4	10	10		2.4	20	4.4	45
Hour	Major Volume	Minor Volume	Total Volume	Peds/h	Gaps/h	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)		4B (100%)
07 - 08	1027	61	1088	0	0	No	No	No	Yes	No	No	No	No	No
08 - 09	1096	61	1157	0	0	No	No	No	Yes	No	No	No	No	No
09 - 10	885	71	956	0	0	No	No	No	Yes	No	No	No	No	No
10 - 11	857	88	945	0	0	No	No	No	Yes	No	No	No	No	No
11 - 12	871	117	988	0	0	No	No	No	Yes	No	No	No	No	No
12 - 13	983	128	1111	0	0	No	Yes	Yes	Yes	No	No	No	No	No
13 - 14	1016	93	1109	0	0	No	No	Yes	Yes	No	No	No	No	No
14 - 15	1122	85	1207	0	0	No	No	Yes	Yes	No	No	No	No	No
15 - 16	1232	114	1346	0	0	No	No	Yes	Yes	Yes	No	No	No	No
16 - 17	1310	164	1474	0	0	Yes	Yes	Yes	Yes	Yes	No	No	No	No
17 - 18	1185	167	1352	0	0	Yes	Yes	Yes	Yes	Yes	No	No	No	No
18 - 19	833	64	897	0	0	No	No	No	Yes	No	No	No	No	No
Total	12417	1213	13630	0	0	2	3	6	12	3	0	0	0	0
Warrants														
Warrant 1:	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	ım Vehicula	ar Volumes	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2:	Four-Hou	r Vehicul	ar Volun	1e										
Four-Hou	r Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3:	Peak Hou	r												
A. Peak-H	our Condit	ions (Minc	or delay	and min	or volume	and to	otal volum	e)or						
B. Peak-Ho	our Vehicul	lar Volume	es (Both m	ajor appro	achesai	nd highe	r minor ap	proach)						
Warrant 4:	Pedestria	n Volume	• • • • • • • • • • • • • • • • • • •											
A. Four Ho	our Volume	esor												
B. One-Ho	our Volume	'S												
Warrant 5: S	School Cr	ossing												
Gaps Sam	e Period	and												
Student V	olumes													
Nearest Tr	raffic Contr	ol Signal (optional)										✓	
Warrant 6:	Coordinat	ted Signa	l System											
Degree of	Platooning	g (Predom	inant direc	tion or bo	th directio	ons)								
Warrant 7:	Crash Exp	erience												
A. Adequa	ite trials of	alternative	es, observa	nce and e	nforceme	nt failed	and							
B. Reporte	ed crashes	susceptible	e to correc	tion by sig	ınal (12-m	onth perio	od)and-							
C. 80% Volumes for Warrants 1A, 1B,or 4 are satisfied									✓					
Warrant 8:	Roadway	Network												
A. Weekda	ay Volume	(Peak hou	r totalar	nd projec	ted warra	nts 1, 2, or	3)or							
B. Weekend Volume (Five hours total)														
Warrant 9:	Grade Cro	ssing												
A. Grade 0	Crossing wi	thin 140 ft	:and											
B. Peak-Ho	our Vehicul	lar Volume	es											

CAPACITY ANALYSIS

Pleasant Street at Trapelo Road Pleasant Street at Olmsted Drive Olmsted Drive at Site Drive



01/11/2021

	۶	→	•	•	+	•	•	†	~	/	↓	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*				↑ Դ			4		*		7
Traffic Volume (vph)	266	892	0	0	810	186	0	0	0	240	0	263
Future Volume (vph)	266	892	0	0	810	186	0	0	0	240	0	263
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	16	16	16	15	12	12
Storage Length (ft)	0		0	0		0	0		0	300		0
Storage Lanes	1		0	0		0	0		0	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.972							0.850
Flt Protected	0.950									0.950		
Satd. Flow (prot)	1711	1801	0	0	3319	0	0	2153	0	1947	0	1599
Flt Permitted	0.119									0.757		
Satd. Flow (perm)	214	1801	0	0	3319	0	0	2153	0	1551	0	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					32							159
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		434			399			175			540	
Travel Time (s)		11.8			10.9			4.8			14.7	
Peak Hour Factor	0.93	0.93	0.93	0.91	0.91	0.91	0.92	0.92	0.92	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	0%	0%	2%	0%	1%
Adj. Flow (vph)	286	959	0	0	890	204	0	0	0	258	0	283
Shared Lane Traffic (%)	200	707		· ·	070	201		•	J	200		200
Lane Group Flow (vph)	286	959	0	0	1094	0	0	0	0	258	0	283
Number of Detectors	1	2			2		1	2	· ·	1		1
Detector Template	Left	Thru			Thru		Left	Thru		Left		Right
Leading Detector (ft)	20	100			100		20	100		20		20
Trailing Detector (ft)	0	0			0		0	0		0		0
Detector 1 Position(ft)	0	0			0		0	0		0		0
Detector 1 Size(ft)	20	6			6		20	6		20		20
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex
Detector 1 Channel	02	01. 2.1			01.12.1		027	0		51. EX		01.2%
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 2 Position(ft)	0.0	94			94		0.0	94		0.0		0.0
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel		OI LA			OI. LX			OI! EX				
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	pm+pt	NA			NA			0.0		Perm		custom
Protected Phases	5	2			6		3	8		1 Citii		4
Permitted Phases	2	_			O .		8	U		4		5
Detector Phase	5	2			6		3	8		4		4
Switch Phase	J	2			U		3	U		7		-
Minimum Initial (s)	5.0	5.0			5.0		1.0	5.0		5.0		5.0
Minimum Split (s)	25.0	25.0			25.0		6.0	25.0		25.0		25.0
Total Split (s)	22.0	56.0			34.0		8.0	26.0		26.0		26.0
Total Split (%)	24.4%	62.2%			37.8%		8.9%	28.9%		28.9%		28.9%
10(a) 3piil (70)	Z4.470	02.270			31.070		0.770	20.770		20.970		20.970

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	16.0	50.0			28.0		4.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		1.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		None	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0			7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0			11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0			0		0		0
Act Effct Green (s)	58.6	58.6			39.1					21.4		40.9
Actuated g/C Ratio	0.65	0.65			0.43					0.24		0.45
v/c Ratio	0.75	0.82			0.75					0.70		0.35
Control Delay	28.9	21.0			27.3					41.2		6.6
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	28.9	21.0			27.3					41.2		6.6
LOS	С	С			С					D		Α
Approach Delay		22.9			27.3						23.1	
Approach LOS		С			С						С	
Queue Length 50th (ft)	86	361			264					135		39
Queue Length 95th (ft)	#206	#768			#466					195		69
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	429	1172			1461					401		805
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.67	0.82			0.75					0.64		0.35

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 24.6

Intersection LOS: C

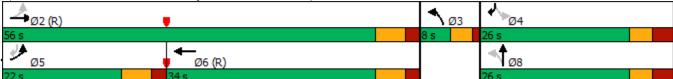
Intersection Capacity Utilization 68.0%

ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



01/19/2021

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Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL	SBT SBR
Lane Configurations 7 1 1 1 1	7
Traffic Volume (vph) 350 972 0 0 904 205 0 0 217	0 384
Future Volume (vph) 350 972 0 0 904 205 0 0 217	0 384
	1900 1900
Lane Width (ft) 11 11 11 11 11 16 16 16 15	12 12
Storage Length (ft) 0 0 0 0 0 300	0
Storage Lanes 1 0 0 0 0 0 1	1
Taper Length (ft) 25 25 25	
	1.00 1.00
Frt 0.972	0.850
Flt Protected 0.950 0.950	
Satd. Flow (prot) 1711 1801 0 0 3319 0 0 2153 0 1947	0 1599
Flt Permitted 0.099 0.757	
Satd. Flow (perm) 178 1801 0 0 3319 0 0 2153 0 1551	0 1599
Right Turn on Red Yes Yes Yes	Yes
Satd. Flow (RTOR) 32	151
Link Speed (mph) 25 25 25	25
Link Distance (ft) 434 399 175	540
	14.7
· ,	0.93 0.93
Heavy Vehicles (%) 2% 2% 2% 0% 2% 3% 0% 0% 0% 2%	0% 1%
Adj. Flow (vph) 376 1045 0 0 993 225 0 0 0 233	0 413
Shared Lane Traffic (%)	
Lane Group Flow (vph) 376 1045 0 0 1218 0 0 0 233	0 413
Number of Detectors 1 2 2 1 2 1	1
Detector Template Left Thru Thru Left Thru Left	Right
Leading Detector (ft) 20 100 100 20 100 20	20
Trailing Detector (ft) 0 0 0 0 0	0
Detector 1 Position(ft) 0 0 0 0 0	0
Detector 1 Size(ft) 20 6 20 6 20	20
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex	CI+Ex
Detector 1 Channel	
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0	0.0
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0	0.0
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0	0.0
Detector 2 Position(ft) 94 94 94	
Detector 2 Size(ft) 6 6	
Detector 2 Type CI+Ex CI+Ex CI+Ex	
Detector 2 Channel	
Detector 2 Extend (s) 0.0 0.0 0.0	
Turn Type pm+pt NA NA Perm	custom
Protected Phases 5 2 6 3 8	4
Permitted Phases 2 8 4	5
Detector Phase 5 2 6 3 8 4	4
Switch Phase	
Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 5.0	5.0
Minimum Split (s) 25.0 25.0 25.0 25.0 25.0 25.0	25.0
Total Split (s) 22.0 56.0 34.0 8.0 26.0 26.0	26.0
Total Split (%) 24.4% 62.2% 37.8% 8.9% 28.9% 28.9%	28.9%

2 - 2021 PM EX.syn Synchro 10 Report

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	16.0	50.0			28.0		4.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		1.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		None	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0			7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0			11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0			0		0		0
Act Effct Green (s)	60.3	60.3			35.2					19.7		44.8
Actuated g/C Ratio	0.67	0.67			0.39					0.22		0.50
v/c Ratio	0.82	0.87			0.92					0.69		0.47
Control Delay	36.8	23.3			40.8					42.1		9.9
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	36.8	23.3			40.8					42.1		9.9
LOS	D	С			D					D		Α
Approach Delay		26.9			40.8						21.5	
Approach LOS		С			D						С	
Queue Length 50th (ft)	144	409			~364					123		80
Queue Length 95th (ft)	#322	#848			#549					182		136
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	469	1205			1319					388		857
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.80	0.87			0.92					0.60		0.48

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 31.0

Intersection LOS: C

Intersection Capacity Utilization 74.6%

ICU Level of Service D

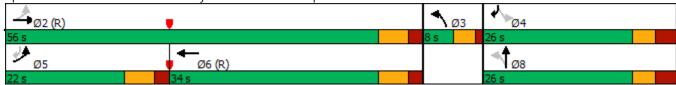
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†			† 1>			4		*		7
Traffic Volume (vph)	295	956	0	0	868	207	0	0	0	265	0	291
Future Volume (vph)	295	956	0	0	868	207	0	0	0	265	0	291
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	16	16	16	15	12	12
Storage Length (ft)	0		0	0		0	0		0	300		0
Storage Lanes	1		0	0		0	0		0	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.971							0.850
Flt Protected	0.950									0.950		
Satd. Flow (prot)	1711	1801	0	0	3316	0	0	2153	0	1947	0	1599
Flt Permitted	0.097									0.757		
Satd. Flow (perm)	175	1801	0	0	3316	0	0	2153	0	1551	0	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					34							153
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		434			399			175			540	
Travel Time (s)		11.8			10.9			4.8			14.7	
Peak Hour Factor	0.93	0.93	0.93	0.91	0.91	0.91	0.92	0.92	0.92	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	0%	0%	2%	0%	1%
Adj. Flow (vph)	317	1028	0	0	954	227	0	0	0	285	0	313
Shared Lane Traffic (%)	0.7	.020			70.					200		0.0
Lane Group Flow (vph)	317	1028	0	0	1181	0	0	0	0	285	0	313
Number of Detectors	1	2			2		1	2	_	1		1
Detector Template	Left	Thru			Thru		Left	Thru		Left		Right
Leading Detector (ft)	20	100			100		20	100		20		20
Trailing Detector (ft)	0	0			0		0	0		0		0
Detector 1 Position(ft)	0	0			0		0	0		0		0
Detector 1 Size(ft)	20	6			6		20	6		20		20
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	pm+pt	NA			NA					Perm		custom
Protected Phases	5	2			6		3	8				4
Permitted Phases	2						8			4		5
Detector Phase	5	2			6		3	8		4		4
Switch Phase	_	_			_			_		•		•
Minimum Initial (s)	5.0	5.0			5.0		1.0	5.0		5.0		5.0
Minimum Split (s)	25.0	25.0			25.0		6.0	25.0		25.0		25.0
Total Split (s)	22.0	56.0			34.0		8.0	26.0		26.0		26.0
Total Split (%)	24.4%	62.2%			37.8%		8.9%	28.9%		28.9%		28.9%
. 5.61 Opin (75)	2 170	02.270			57.070		5.770	20.770		20.770		20.770

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	16.0	50.0			28.0		4.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		1.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		None	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0			7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0			11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0			0		0		0
Act Effct Green (s)	57.0	57.0			36.3					23.0		43.7
Actuated g/C Ratio	0.63	0.63			0.40					0.26		0.49
v/c Ratio	0.83	0.90			0.87					0.72		0.37
Control Delay	40.4	28.8			35.2					40.7		7.1
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	40.4	28.8			35.2					40.7		7.1
LOS	D	С			D					D		Α
Approach Delay		31.5			35.2						23.1	
Approach LOS		С			D						С	
Queue Length 50th (ft)	120	453			326					148		47
Queue Length 95th (ft)	#268	#855			#524					216		85
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	412	1140			1356					416		842
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.77	0.90			0.87					0.69		0.37

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90

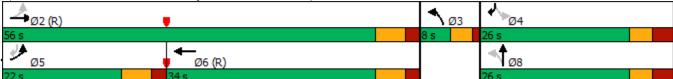
Intersection Signal Delay: 31.3

Intersection LOS: C Intersection Capacity Utilization 73.3% ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



Lanes, Volumes, Timings 3: Private Driveway/Pleasant Street & Trapelo Road

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^			↑ ↑			4		*		7
Traffic Volume (vph)	389	1042	0	0	969	232	0	0	0	245	0	428
Future Volume (vph)	389	1042	0	0	969	232	0	0	0	245	0	428
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	16	16	16	15	12	12
Storage Length (ft)	0		0	0		0	0		0	300		0
Storage Lanes	1		0	0		0	0		0	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.971							0.850
Flt Protected	0.950									0.950		
Satd. Flow (prot)	1711	1801	0	0	3316	0	0	2153	0	1947	0	1599
Flt Permitted	0.098									0.757		
Satd. Flow (perm)	176	1801	0	0	3316	0	0	2153	0	1551	0	1599
Right Turn on Red		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					38							36
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		434			399			175			540	
Travel Time (s)		11.8			10.9			4.8			14.7	
Peak Hour Factor	0.93	0.93	0.93	0.91	0.91	0.91	0.92	0.92	0.92	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	0%	0%	2%	0%	1%
Adj. Flow (vph)	418	1120	0	0	1065	255	0	0	0	263	0	460
Shared Lane Traffic (%)		0	•		.000	200			· ·	200		100
Lane Group Flow (vph)	418	1120	0	0	1320	0	0	0	0	263	0	460
Number of Detectors	1	2	•		2		1	2	· ·	1		1
Detector Template	Left	Thru			Thru		Left	Thru		Left		Right
Leading Detector (ft)	20	100			100		20	100		20		20
Trailing Detector (ft)	0	0			0		0	0		0		0
Detector 1 Position(ft)	0	0			0		0	0		0		0
Detector 1 Size(ft)	20	6			6		20	6		20		20
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 2 Position(ft)	0.0	94			94		0.0	94		0.0		0.0
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel		OITEX			OI! EX			OI! EX				
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	pm+pt	NA			NA			0.0		Perm		custom
Protected Phases	5	2			6			8		1 01111		4
Permitted Phases	2				U		8	U		4		5
Detector Phase	5	2			6		8	8		4		4
Switch Phase	3	2			U		U	U		-		-
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0		5.0		5.0
Minimum Split (s)	25.0	25.0			25.0		25.0	25.0		25.0		25.0
Total Split (s)	24.0	64.0			40.0		26.0	26.0		26.0		26.0
Total Split (%)	26.7%	71.1%			44.4%		28.9%	28.9%		28.9%		28.9%
Tutal Split (70)	20.770	11.170			44.470		20.770	20.770		20.770		20.970

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	18.0	58.0			34.0		20.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		3.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		Min	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0		0	0		0		0
Act Effct Green (s)	60.2	60.2			36.0					19.8		44.0
Actuated g/C Ratio	0.67	0.67			0.40					0.22		0.49
v/c Ratio	0.94	0.93			0.98					0.77		0.58
Control Delay	55.5	29.0			47.2					49.4		18.2
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	55.5	29.0			47.2					49.4		18.2
LOS	Е	С			D					D		В
Approach Delay		36.2			47.2						29.5	
Approach LOS		D			D						С	
Queue Length 50th (ft)	185	514			~384					139		159
Queue Length 95th (ft)	#371	#873			#543					#249		253
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	446	1204			1350					361		785
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.94	0.93			0.98					0.73		0.59

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 38.9

Intersection LOS: D

Intersection Capacity Utilization 81.0%

ICU Level of Service D

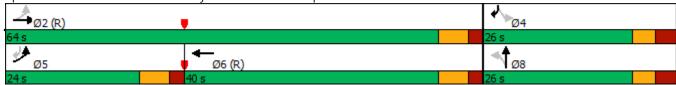
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†			∱ }			4		*		7
Traffic Volume (vph)	297	956	0	0	868	212	0	0	0	276	0	295
Future Volume (vph)	297	956	0	0	868	212	0	0	0	276	0	295
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	16	16	16	15	12	12
Storage Length (ft)	0		0	0	- ''	0	0	10	0	300	12	0
Storage Lanes	1		0	0		0	0		0	1		1
Taper Length (ft)	25		U	25		U	25		U	25		'
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	1.00	1.00	0.73	0.75	1.00	1.00	1.00	1.00	1.00	0.850
Flt Protected	0.950				0.771					0.950		0.050
Satd. Flow (prot)	1711	1801	0	0	3316	0	0	2153	0	1947	0	1599
Flt Permitted	0.099	1001	U	U	3310	U	U	2100	U	0.757	U	1399
Satd. Flow (perm)	178	1801	0	0	3316	0	0	2153	0	1551	0	1599
	1/0	1001		U	3310		U	2100		1001	U	
Right Turn on Red			Yes		٦٢	Yes			Yes			Yes
Satd. Flow (RTOR)		٦٢			35			٦٢			٦٢	153
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		434			399			175			540	
Travel Time (s)	0.00	11.8	0.00	0.01	10.9	0.04	0.00	4.8	0.00	0.00	14.7	0.00
Peak Hour Factor	0.93	0.93	0.93	0.91	0.91	0.91	0.92	0.92	0.92	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	0%	0%	2%	0%	1%
Adj. Flow (vph)	319	1028	0	0	954	233	0	0	0	297	0	317
Shared Lane Traffic (%)												
Lane Group Flow (vph)	319	1028	0	0	1187	0	0	0	0	297	0	317
Number of Detectors	1	2			2		1	2		1		1
Detector Template	Left	Thru			Thru		Left	Thru		Left		Right
Leading Detector (ft)	20	100			100		20	100		20		20
Trailing Detector (ft)	0	0			0		0	0		0		0
Detector 1 Position(ft)	0	0			0		0	0		0		0
Detector 1 Size(ft)	20	6			6		20	6		20		20
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	pm+pt	NA			NA					Perm		custom
Protected Phases	5	2			6		3	8				4
Permitted Phases	2						8			4		5
Detector Phase	5	2			6		3	8		4		4
Switch Phase												•
Minimum Initial (s)	5.0	5.0			5.0		1.0	5.0		5.0		5.0
Minimum Split (s)	25.0	25.0			25.0		6.0	25.0		25.0		25.0
Total Split (s)	22.0	56.0			34.0		8.0	26.0		26.0		26.0
Total Split (%)	24.4%	62.2%			37.8%		8.9%	28.9%		28.9%		28.9%
rotal Oplit (70)	Z-7.7/0	02.270			37.070		0.770	20.770		20.770		20.770

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	16.0	50.0			28.0		4.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		1.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		None	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0			7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0			11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0			0		0		0
Act Effct Green (s)	56.2	56.2			35.5					23.8		44.5
Actuated g/C Ratio	0.62	0.62			0.39					0.26		0.49
v/c Ratio	0.84	0.92			0.89					0.72		0.37
Control Delay	41.7	30.9			37.4					40.1		7.0
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	41.7	30.9			37.4					40.1		7.0
LOS	D	С			D					D		Α
Approach Delay		33.5			37.4						23.0	
Approach LOS		С			D						С	
Queue Length 50th (ft)	122	472			338					152		46
Queue Length 95th (ft)	#270	#855			#527					226		87
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	408	1123			1328					425		866
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.78	0.92			0.89					0.70		0.37

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.92

Intersection Signal Delay: 32.9

Intersection LOS: C

Intersection Capacity Utilization 74.2%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^			∱ ∱			4		*		7
Traffic Volume (vph)	393	1042	0	0	969	244	0	0	0	254	0	431
Future Volume (vph)	393	1042	0	0	969	244	0	0	0	254	0	431
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	11	11	11	11	11	16	16	16	15	12	12
Storage Length (ft)	0		0	0		0	0		0	300		0
Storage Lanes	1		0	0		0	0		0	1		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Frt					0.970							0.850
Flt Protected	0.950									0.950		
Satd. Flow (prot)	1711	1801	0	0	3312	0	0	2153	0	1947	0	1599
Flt Permitted	0.097									0.757		
Satd. Flow (perm)	175	1801	0	0	3312	0	0	2153	0	1551	0	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)					40							395
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		434			399			175			540	
Travel Time (s)		11.8			10.9			4.8			14.7	
Peak Hour Factor	0.93	0.93	0.93	0.91	0.91	0.91	0.92	0.92	0.92	0.93	0.93	0.93
Heavy Vehicles (%)	2%	2%	2%	0%	2%	3%	0%	0%	0%	2%	0%	1%
Adj. Flow (vph)	423	1120	0	0	1065	268	0	0	0	273	0	463
Shared Lane Traffic (%)												
Lane Group Flow (vph)	423	1120	0	0	1333	0	0	0	0	273	0	463
Number of Detectors	1	2			2		1	2		1		1
Detector Template	Left	Thru			Thru		Left	Thru		Left		Right
Leading Detector (ft)	20	100			100		20	100		20		20
Trailing Detector (ft)	0	0			0		0	0		0		0
Detector 1 Position(ft)	0	0			0		0	0		0		0
Detector 1 Size(ft)	20	6			6		20	6		20		20
Detector 1 Type	CI+Ex	CI+Ex			CI+Ex		CI+Ex	CI+Ex		CI+Ex		CI+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Queue (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 1 Delay (s)	0.0	0.0			0.0		0.0	0.0		0.0		0.0
Detector 2 Position(ft)		94			94			94				
Detector 2 Size(ft)		6			6			6				
Detector 2 Type		CI+Ex			CI+Ex			CI+Ex				
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				
Turn Type	pm+pt	NA			NA					Perm		Perm
Protected Phases	5	2			6			8				
Permitted Phases	2						8			4		4
Detector Phase	5	2			6		8	8		4		4
Switch Phase												
Minimum Initial (s)	5.0	5.0			5.0		5.0	5.0		5.0		5.0
Minimum Split (s)	25.0	25.0			25.0		25.0	25.0		25.0		25.0
Total Split (s)	24.0	64.0			40.0		26.0	26.0		26.0		26.0
Total Split (%)	26.7%	71.1%			44.4%		28.9%	28.9%		28.9%		28.9%

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Maximum Green (s)	18.0	58.0			34.0		20.0	20.0		20.0		20.0
Yellow Time (s)	4.0	4.0			4.0		3.0	3.0		3.0		3.0
All-Red Time (s)	2.0	2.0			2.0		3.0	3.0		3.0		3.0
Lost Time Adjust (s)	-1.0	-1.0			-1.0			0.0		-1.0		-1.0
Total Lost Time (s)	5.0	5.0			5.0			6.0		5.0		5.0
Lead/Lag	Lead				Lag							
Lead-Lag Optimize?	Yes				Yes							
Vehicle Extension (s)	3.0	3.0			3.0		3.0	3.0		3.0		3.0
Recall Mode	Min	C-Min			C-Max		Min	Min		Min		Min
Walk Time (s)	7.0	7.0			7.0		7.0	7.0		7.0		7.0
Flash Dont Walk (s)	11.0	11.0			11.0		11.0	11.0		11.0		11.0
Pedestrian Calls (#/hr)	0	0			0		0	0		0		0
Act Effct Green (s)	60.5	60.5			36.1					19.5		19.5
Actuated g/C Ratio	0.67	0.67			0.40					0.22		0.22
v/c Ratio	0.94	0.93			0.99					0.81		0.71
Control Delay	56.5	28.3			48.7					53.5		12.4
Queue Delay	0.0	0.0			0.0					0.0		0.0
Total Delay	56.5	28.3			48.7					53.5		12.4
LOS	Е	С			D					D		В
Approach Delay		36.0			48.7						27.6	
Approach LOS		D			D						С	
Queue Length 50th (ft)	189	514			~397					145		31
Queue Length 95th (ft)	#378	#873			#552					#263		132
Internal Link Dist (ft)		354			319			95			460	
Turn Bay Length (ft)										300		
Base Capacity (vph)	448	1210			1353					361		675
Starvation Cap Reductn	0	0			0					0		0
Spillback Cap Reductn	0	0			0					0		0
Storage Cap Reductn	0	0			0					0		0
Reduced v/c Ratio	0.94	0.93			0.99					0.76		0.69

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:EBTL and 6:WBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.99

Intersection Signal Delay: 39.0

Intersection LOS: D

Intersection Capacity Utilization 82.1%

ICU Level of Service E

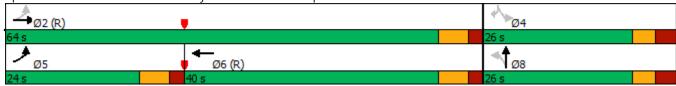
Analysis Period (min) 15

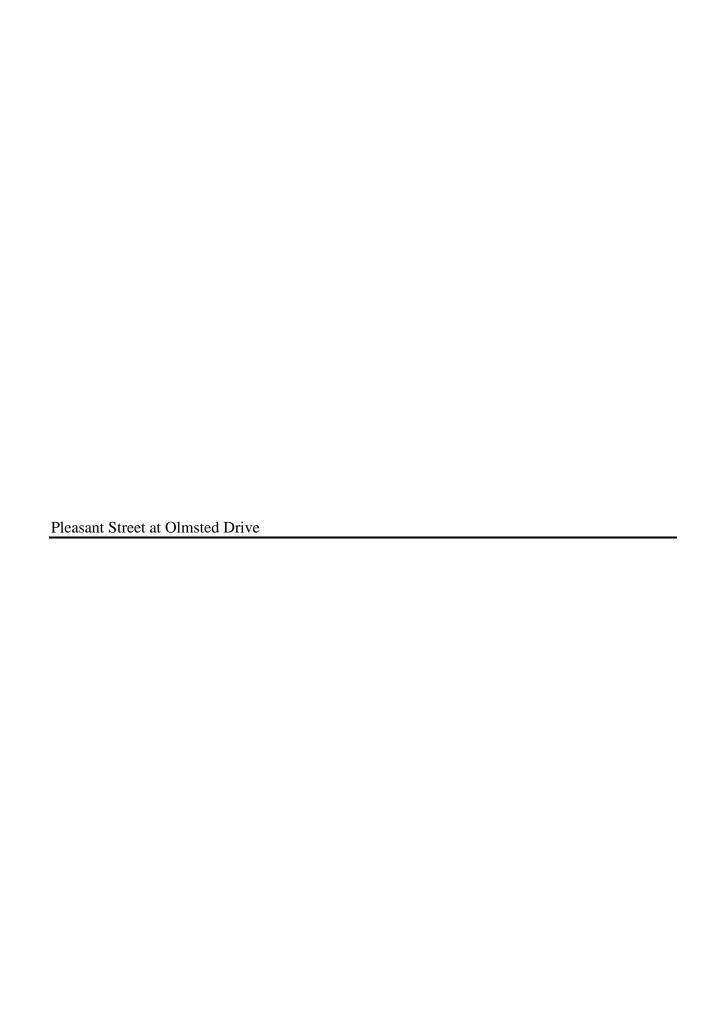
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.





Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		LDK				אמכ
Lane Configurations	, M	7	_ ች	↑	}	0
Traffic Vol, veh/h	2	7	1	451	496	0
Future Vol, veh/h	2	7	1	451	496	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	38	38	86	83	89	89
Heavy Vehicles, %	0	0	0	4	3	0
Mvmt Flow	5	18	1	543	557	0
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	1102	557	557	0	-	0
Stage 1	557	-	-	-	-	-
Stage 2	545	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	_	_	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	-	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	236	534	1024			_
•	578	334	1024	_	_	_
Stage 1	585	-	-		-	
Stage 2	202	-	-	-	-	-
Platoon blocked, %	00/	504	1001	-	-	-
Mov Cap-1 Maneuver	236	534	1024	-	-	-
Mov Cap-2 Maneuver	236	-	-	-	-	-
Stage 1	577	-	-	-	-	-
Stage 2	585	-	-	-	-	-
Annraach	- FD		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	14.2		0		0	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBL	MRTI	EBLn1	SBT	SBR
	IL				SDI	אמכ
Capacity (veh/h)		1024	-	417	-	-
HCM Lane V/C Ratio		0.001	-	0.057	-	-
HCM Control Delay (s)		8.5	-	14.2	-	-
HCM Lane LOS		Α	-	В	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

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Intersection						
Int Delay, s/veh	0.2					
		EDD	ND	NET	OPT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	, A		<u> ነ</u>	↑	f)	
Traffic Vol, veh/h	0	4	5	550	597	1
Future Vol, veh/h	0	4	5	550	597	1
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	33	33	91	91	84	84
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	12	5	604	711	1
Major/Minor N	linor2	Λ	/lajor1	Λ	/lajor2	
	1326	712	712	0	//aju/2 -	0
Stage 1	712	112		-	-	
	614	•	-	-	-	-
Stage 2		- 4 2		-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	173	436	897	-	-	-
Stage 1	490	-	-	-	-	-
Stage 2	544	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	172	436	897	-	-	-
Mov Cap-2 Maneuver	172	-	-	-	-	-
Stage 1	487	-	-	-	-	-
Stage 2	544	-	-	-	-	-
Annroach	EB		NB		SB	
Approach						
HCM Control Delay, s	13.5		0.1		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		897	-		-	-
HCM Lane V/C Ratio		0.006		0.028	-	-
HCM Control Delay (s)		9	-		-	-
HCM Lane LOS		A	-	В	-	-
HCM 95th %tile Q(veh)		0	-	0.1	-	-
2011						

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Intersection						
Int Delay, s/veh	0.3					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		EDK				SDK
Lane Configurations	Y	7	<u> </u>	†	^	٥
Traffic Vol, veh/h	2	7	1	501	549	0
Future Vol, veh/h	2	7	1	501	549	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	38	38	86	83	89	89
Heavy Vehicles, %	0	0	0	4	3	0
Mvmt Flow	5	18	1	604	617	0
Major/Minor N	/liner?	N	/laior1		Anior?	
	/linor2		Major1		Major2	^
Conflicting Flow All	1223	617	617	0	-	0
Stage 1	617	-	-	-	-	-
Stage 2	606	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	200	494	973	-	-	-
Stage 1	542	-	-	-	-	-
Stage 2	548	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	200	494	973	-	-	-
Mov Cap-2 Maneuver	200	-	-	-	-	-
Stage 1	541	_	_	_	-	-
Stage 2	548	_	_	_	_	_
Stage 2	570					
Approach	EB		NB		SB	
HCM Control Delay, s	15.3		0		0	
HCM LOS	С					
NA: 1 /04 1 N		NDI	NOT	EDL 4	ODT	000
Minor Lane/Major Mvm	t	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		973	-	· · -	-	-
HCM Lane V/C Ratio		0.001	-	0.064	-	-
HCM Control Delay (s)		8.7	-	15.3	-	-
HCM Lane LOS		Α	-	С	-	-
		^		0.2		
HCM 95th %tile Q(veh)		0	-	0.2	-	-

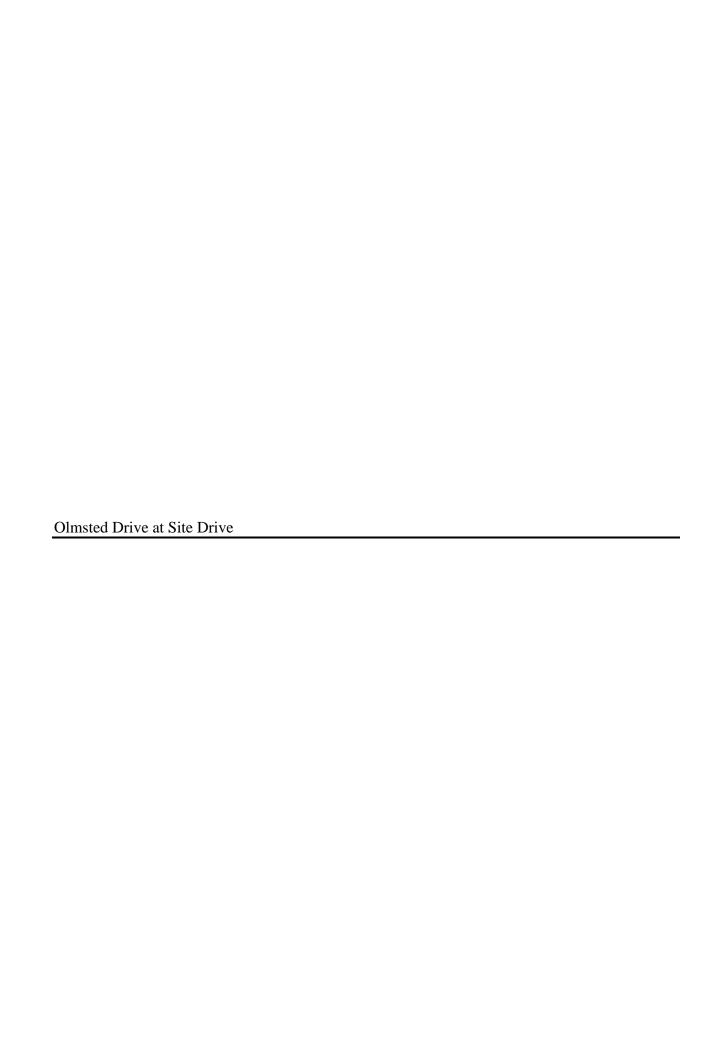
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Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
		LDK				SDK
Lane Configurations	7	4	ጟ	†	}	1
Traffic Vol, veh/h	0	4	5	616	669	1
Future Vol, veh/h	0	4	5	616	669	1
Conflicting Peds, #/hr	0	0	0	0	0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	33	33	91	91	84	84
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	12	5	677	796	1
	1inor2		/lajor1		/lajor2	
Conflicting Flow All	1484	797	797	0	-	0
Stage 1	797	-	-	-	-	-
Stage 2	687	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	_	-	-	-
Critical Hdwy Stg 2	5.4	-	_	-	_	-
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	139	390	834	_		_
	447	370	034	-	-	
Stage 1			-		-	
Stage 2	503	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	138	390	834	-	-	-
Mov Cap-2 Maneuver	138	-	-	-	-	-
Stage 1	444	-	-	-	-	-
Stage 2	503	-	-	-	-	-
Annroach	ED		ND		CD	
Approach	EB		NB		SB	
HCM Control Delay, s	14.5		0.1		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NBL	NIDT	EBLn1	SBT	SBR
					SDI	אטכ
Capacity (veh/h)		834	-	0,0	-	-
HCM Lane V/C Ratio		0.007		0.031	-	-
HCM Control Delay (s)		9.3	-		-	-
HCM Lane LOS		Α	-	В	-	-
HCM 95th %tile Q(veh)		0	-	0.1	-	-

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Intersection						
Int Delay, s/veh	1.4					
	EBL	EBR	MDI	NBT	SBT	SBR
Movement Lang Configurations		FRK	NBL			SRK
Lane Configurations	12	22	<u>ነ</u>	†	F 40	A
Traffic Vol, veh/h	12	22	8	501	549	4
Future Vol, veh/h	12	22	8	501	549	4
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	- 1F0	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	- 0/	0	0	-
Peak Hour Factor	38	38	86	83	89	89
Heavy Vehicles, %	0	0	0	4	3	0
Mvmt Flow	32	58	9	604	617	4
Major/Minor N	Minor2	N	/lajor1	N	Major2	
Conflicting Flow All	1241	619	621	0	-	0
Stage 1	619	-	-	-	-	-
Stage 2	622	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2		_	_
Pot Cap-1 Maneuver	195	492	969	-	-	-
Stage 1	541	.,_	-	_	_	_
Stage 2	539	_	_	_	_	
Platoon blocked, %	007			_	_	_
Mov Cap-1 Maneuver	193	492	969	_	_	
Mov Cap-2 Maneuver	193	- 7/2	-	_	_	_
Stage 1	536	_	_	_		_
Stage 2	539			_	_	
Slayt 2	557	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	20.7		0.1		0	
HCM LOS	С					
Minor Lane/Major Mvm	t	NBL	NDT	EBLn1	SBT	SBR
	t e				SDI	אטכ
Capacity (veh/h)		969	-	0.0	-	-
HCM Control Polov (c)		0.01		0.281	-	-
HCM Long LOS		8.8	-		-	-
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	C 1.1	-	-
			-			_

Intersection						
Int Delay, s/veh	1.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥		7		Þ	
Traffic Vol, veh/h	8	16	21	616	669	11
Future Vol, veh/h	8	16	21	616	669	11
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	_
Peak Hour Factor	33	33	91	91	84	84
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	24	48	23	677	796	13
WWW. LOW	Z -7	70	20	011	, 70	10
	Minor2		/lajor1		/lajor2	
Conflicting Flow All	1526	803	809	0	-	0
Stage 1	803	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	_	-	-
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
Pot Cap-1 Maneuver	131	387	825	_	_	_
Stage 1	444		- 025	_	_	_
Stage 2	484	-		-		_
Platoon blocked, %	404			_	_	-
	127	207	025	-	-	-
Mov Cap-1 Maneuver	127	387	825	-	-	-
Mov Cap-2 Maneuver	127	-	-	-	-	-
Stage 1	432	-	-	-	-	-
Stage 2	484	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	27.7		0.3		0	
HCM LOS	21.1 D		0.5		U	
HOW LUS	D					
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		825	-		_	_
HCM Lane V/C Ratio		0.028		0.316	_	_
HCM Control Delay (s)	9.5	_		_	_
HCM Lane LOS		Α	_	D	_	_
HCM 95th %tile Q(veh	1)	0.1	_	1.3	_	_
1101VI 73111 701116 Q(VCI	7	0.1		1.5		



Intersection						
Int Delay, s/veh	2.2					
		EDT.	MPT	MADD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		र्स	f)		Y	
Traffic Vol, veh/h	0	16	7	3	9	0
Future Vol, veh/h	0	16	7	3	9	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	17	8	3	10	0
N.A. ' /N.A'					A1 C	
-	Major1		Najor2		Vinor2	
Conflicting Flow All	11	0	-	0	27	10
Stage 1	-	-	-	-	10	-
Stage 2	-	-	-	-	17	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1608		-	-	988	1071
Stage 1	-	_	_	_	1013	-
Stage 2	_	_	_	-	1006	_
Platoon blocked, %		_	_	_	1000	
Mov Cap-1 Maneuver	1608			-	988	1071
•		-	-		988	1071
Mov Cap-2 Maneuver	-	-	-	-		
Stage 1	-	-	-	-	1013	-
Stage 2	-	-	-	-	1006	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.7	
HCM LOS	- 0		U		Α	
TIOWI LOG					٨	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1608	-	-	-	988
HCM Lane V/C Ratio		-	-	-	-	0.01
HCM Control Delay (s)		0	-	-	-	8.7
HCM Lane LOS		A	-	-	-	А
HCM 95th %tile Q(veh))	0	_	-	_	0
HOW FOUT FOUT Q(VCH)	,	U				U

Intersection						
Int Delay, s/veh	1.3					
		EDT	MDT	WDD	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	4	^	0	Y	0
Traffic Vol, veh/h	0	13	17	9	7	0
Future Vol, veh/h	0	13	17	9	7	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	14	18	10	8	0
Major/Minor	laiar1	N	//olor)		Minara	
	lajor1		Major2		Minor2	
Conflicting Flow All	28	0	-	0	37	23
Stage 1	-	-	-	-	23	-
Stage 2	-	-	-	-	14	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1585	-	-	-	975	1054
Stage 1	-	-	-	-	1000	-
Stage 2	-	-	-	-	1009	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1585	-	-	-	975	1054
Mov Cap-2 Maneuver	-	-	-	-	975	-
Stage 1	-	-	-	-	1000	-
Stage 2	-	_	-	_	1009	_
- 1						
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.7	
HCM LOS					Α	
Minor Lane/Major Mymt	+	FRI	FRT	WRT	W/RR	SRI n1
Minor Lane/Major Mymt	t	EBL	EBT	WBT	WBR:	
Capacity (veh/h)	Ì	1585	-	-	-	975
Capacity (veh/h) HCM Lane V/C Ratio	i .	1585 -	EBT -	-	-	975 0.008
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)	1	1585 - 0	- - -	- -	-	975 0.008 8.7
Capacity (veh/h) HCM Lane V/C Ratio		1585 -	-	-	-	975 0.008

Intersection						
Int Delay, s/veh	3.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	₽		W	
Traffic Vol, veh/h	0	7	4	3	9	0
Future Vol, veh/h	0	7	4	3	9	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	a.# -	0	0	-	0	_
Grade, %	- "	0	0	_	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	0	8	4	3	10	
IVIVIIII FIOW	U	Ö	4	3	10	0
Major/Minor	Major1	N	Major2	ľ	Minor2	
Conflicting Flow All	7	0		0	14	6
Stage 1	-	-	_	-	6	-
Stage 2	_	_	_	_	8	_
Critical Hdwy	4.12	_		_	6.42	6.22
Critical Hdwy Stg 1	4.12	_	_	_	5.42	0.22
	-		_		5.42	-
Critical Hdwy Stg 2		-	-	-		
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1614	-	-	-	1005	1077
Stage 1	-	-	-	-	1017	-
Stage 2	-	-	-	-	1015	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1614	-	-	-	1005	1077
Mov Cap-2 Maneuver	-	-	-	-	1005	-
Stage 1	-	-	-	-	1017	-
Stage 2	-	-	-	-	1015	-
J						
A	ED		MD		CD	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.6	
HCM LOS					Α	
Minor Lane/Major Mvm	ot	EBL	EBT	WBT	WBR :	SRI n1
	ic		LDT	VVDI	WDIX.	
Capacity (veh/h)		1614	-	-	-	1005
HCM Card D. Patro (2)		-	-	-	-	0.01
HCM Control Delay (s))	0	-	-	-	8.6
HCM Lana LOC		Α	-	_	_	Α
HCM Lane LOS HCM 95th %tile Q(veh		0				0

Intersection						
Int Delay, s/veh	2					
		EST	MOT	MES	051	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		4	f)	_	Y	
Traffic Vol, veh/h	0	6	8	9	7	0
Future Vol, veh/h	0	6	8	9	7	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	2, # -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	7	9	10	8	0
Major/Minor N	Major1	N	Major2	ı	Minor2	
Conflicting Flow All	19	0	-	0	21	14
Stage 1	- 17	Ū	-	-	14	-
	-	-	-	-	7	-
Stage 2	4.12	-	-		6.42	6.22
Critical Hdwy Stg 1	4.12	-		-	5.42	0.22
Critical Hdwy Stg 1		-	-		5.42	
Critical Hdwy Stg 2	2 210	-	-	-	3.518	2 210
Follow-up Hdwy	2.218	-	-	-		
Pot Cap-1 Maneuver	1597	-	-	-	996	1066
Stage 1	-	-	-	-	1009	-
Stage 2	-	-	-	-	1016	-
Platoon blocked, %	4507	-	-	-	607	1011
Mov Cap-1 Maneuver	1597	-	-	-	996	1066
Mov Cap-2 Maneuver	-	-	-	-	996	-
Stage 1	-	-	-	-	1009	-
Stage 2	-	-	-	-	1016	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.6	
HCM LOS	U		- 0		Α	
TIGIVI LOG					А	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1597	-	-	-	996
HCM Lane V/C Ratio		-	-	-	-	800.0
HCM Control Delay (s)		0	-	-	-	8.6
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh))	0	-	-	-	0

Intersection						
Int Delay, s/veh	2.3					
		EDT	WDT	WDD	CDI	CDD
Movement Lanc Configurations	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	0	ન	1	2	7	0
Traffic Vol, veh/h	0	4			3	0
Future Vol, veh/h	0	4	2	2	3	0
Conflicting Peds, #/hr	0	0	0	0	0	O Cton
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	4	2	2	3	0
Major/Minor N	Major1	N	Major2		Minor2	
Conflicting Flow All	4	0	najorz -	0	7	3
Stage 1	_	-	_	-	3	-
Stage 2	_	_		_	4	_
Critical Hdwy	4.12	-	-	_	6.42	6.22
Critical Hdwy Stg 1	4.12	-	-	-	5.42	0.22
			-		5.42	-
Critical Hdwy Stg 2		-	-	-		3.318
Follow-up Hdwy	2.218	-	-	-		
Pot Cap-1 Maneuver	1618	-	-	-	1014	1081
Stage 1	-	-	-	-	1020	-
Stage 2	-	-	-	-	1019	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1618	-	-	-	1014	1081
Mov Cap-2 Maneuver	-	-	-	-	1014	-
Stage 1	-	-	-	-	1020	-
Stage 2	-	-	-	-	1019	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.6	
HCM LOS	U		U		Α	
FICIVI LOS					A	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1618	-	-	-	1014
HCM Lane V/C Ratio		-	-	-	-	0.003
HCM Control Delay (s)		0	-	-	-	8.6
HCM Lane LOS		Α	-	-	-	Α
HCM 95th %tile Q(veh))	0	-	-	-	0

Intersection						
Int Delay, s/veh	1.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	LDL	4	1	WDIC	₩	ODIC
Traffic Vol, veh/h	0	3	4	4	3	0
Future Vol, veh/h	0	3	4	4	3	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	- -	None
Storage Length		-	_	-	0	-
Veh in Median Storage	. # -	0	0	_	0	_
Grade, %	- π	0	0	_	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	3	4	4	3	0
IVIVIIIL FIUW	U	3	4	4	3	U
Major/Minor 1	Major1	N	Major2		Minor2	
Conflicting Flow All	8	0	-	0	9	6
Stage 1	-	-	-	-	6	-
Stage 2	-	-	-	-	3	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1612	-	-	-	1011	1077
Stage 1	-	-	-	-	1017	-
Stage 2	-	-	-	-	1020	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1612	-	-	_	1011	1077
Mov Cap-2 Maneuver	-	_	_	-	1011	-
Stage 1	-	_	-	-	1017	-
Stage 2	_	_	_	_	1020	_
Jugo Z					1020	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.6	
HCM LOS					Α	
Minor Lane/Major Mvm	nt	EBL	EBT	WBT	WBR :	SBLn1
Capacity (veh/h)		1612	-			1011
HCM Lane V/C Ratio		-	_	_		0.003
HCM Control Delay (s)		0	_	-	_	8.6
HCM Lane LOS		A	_	_	_	Α
HCM 95th %tile Q(veh))	0	_	_	_	0
110W 75W 76W 2(VCH)		U				U

Intersection						
Int Delay, s/veh	5.7					
		CDT	MOT	MES	CDI	CDD
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	•	र्न	1		Y	
Traffic Vol, veh/h	0	0	0	2	4	0
Future Vol, veh/h	0	0	0	2	4	0
Conflicting Peds, #/hr	_ 0	_ 0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	:,# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	2	4	0
Major/Minor N	Major1	ı	Major2		Minor2	
						1
Conflicting Flow All	2	0	-	0	1	1
Stage 1	-	-	-	-	1	-
Stage 2	-	-	-	-	0	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2		-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	
Pot Cap-1 Maneuver	1620	-	-	-	1022	1084
Stage 1	-	-	-	-	1022	-
Stage 2	-	-	-	-	-	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1620	-	-	-	1022	1084
Mov Cap-2 Maneuver	-	-	-	-	1022	-
Stage 1	-	-	-	-	1022	-
Stage 2	-	-	-	_	-	-
g · -						
	F.D.		16.5		0.5	
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvm	ıt	EBL	EBT	WBT	WBR:	SBLn1
Capacity (veh/h)		1620				1022
HCM Lane V/C Ratio		1020	-	-		0.004
HCM Control Delay (s)		0		_	_	8.5
HCM Lane LOS		A	-	-	-	8.5 A
HCM 95th %tile Q(veh)		0	-	-	-	0
HOW YOU WILL Q(VEN)		U	-	-	-	U

Intersection						
Int Delay, s/veh	3.6					
		CDT	MOT	MES	CDI	000
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		ની	₽		¥	
Traffic Vol, veh/h	0	0	0	4	3	0
Future Vol, veh/h	0	0	0	4	3	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage		0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	0	4	3	0
Major/Minor I	Major1	N	/lajor2		Minor2	
Conflicting Flow All	4	0	-	0	2	2
Stage 1	-	-	-	-	2	-
Stage 2		_	_	_	0	_
Critical Hdwy	4.12	_	_	-	6.42	6.22
Critical Hdwy Stg 1	-	_	_	_	5.42	-
Critical Hdwy Stg 2	_	_	_	-	5.42	_
Follow-up Hdwy	2.218	_	_	_	3.518	
Pot Cap-1 Maneuver	1618	_	_	_	1021	1082
Stage 1	-	_	_	_	1021	-
Stage 2	_		_	_	-	_
Platoon blocked, %		_	_	_		
Mov Cap-1 Maneuver	1618	_		-	1021	1082
Mov Cap-1 Maneuver	1010		-		1021	1002
Stage 1	-	-	-	-	1021	-
Stage 2	-	-	-	-	1021	-
Staye 2	-	-	-	-	-	-
Approach	EB		WB		SB	
HCM Control Delay, s	0		0		8.5	
HCM LOS					Α	
Minor Lane/Major Mvm	nt .	EBL	EBT	WBT	WBR:	CRI n1
	It		LDI	VVDI		
Capacity (veh/h)		1618	-	-		1021
		-	-	-	-	0.003
HCM Captrol Dalay (c)		Λ				
HCM Control Delay (s)		0	-	-	-	8.5
		0 A 0	-	-	-	8.5 A 0

MEMORANDUM

TO: Mr. John C. Dawley FROM: Scott W. Thornton, P.E. and

> Northland Residential Corporation Jennifer Conners

80 Beharrell Street, Suite E Vanasse & Associates, Inc. Concord, MA 01742

35 New England Business Center Drive

Suite 140

Andover, MA 01810 (978) 474-8800

DATE: April 16, 2021 RE: 8058

SUBJECT: Traffic Signal Warrants Analysis

McLean Hospital Residential Development - Olmsted Drive

Waltham, Massachusetts

Vanasse & Associates, Inc. (VAI) has performed a detailed Traffic Signal Warrants Analysis (TSWA) for the intersection of Olmsted Drive at Pleasant Street in Belmont, Massachusetts, as part of the Mclean Hospital Zone 3 Residential Development to assess the warrants required to justify the installation of a traffic control signal at the subject intersection. This analysis was completed in accordance with the methodology and procedures outlined in the Manual on Uniform Traffic Control Devices (MUTCD)¹ and based on traffic counts and field measurements conducted in November 2020.

The traffic Signal Warrant Analysis (TSWA) has been conducted for the intersection of Pleasant Street at Olmsted Drive as required in the Traffic Monitoring and Mitigation Agreement (TMMA). The TMMA states that a traffic signal should be evaluated at this intersection along with traffic projections of the future McLean Zone 3 (Senior Housing subdistrict) and Zone 4 (Research & Development subdistrict). In order to project the impacts of the future development within the McLean Zones 3 and 4 District, the maximum level of permitted traffic for this District were reviewed, as stipulated in the TMMA. The TSWA is summarized below and provided in the Appendix.

EXISTING CONDITIONS

Geometry

Olmsted Drive at Pleasant Street - Olmsted Drive intersects Pleasant Street from the north to form a three-way intersection that operates under STOP-sign control. The Pleasant Street eastbound approach provides an approximate 10-foot wide exclusive left-turn lane and an approximate 11-foot wide through/right-turn lane with an approximate 4-foot wide marked shoulder provided. The Pleasant Street westbound approach provides one approximate 11-foot wide general-purpose travel lane with an approximate 4-foot wide marked shoulder provided. The Olmsted Drive southbound approach provides an approximate 20-foot wide general-purpose travel lane. A sidewalk is provided along the eastern side of Pleasant Street at this location. Crosswalks are not provided for this intersection as there is no sidewalk along the north side of Pleasant Street or on Olmstead Drive. Land use in the vicinity of this intersection consists primarily of commercial uses and open space.

¹Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, DC; 2009.



1

EXISTING TRAFFIC VOLUMES

In order to perform an analysis a 12-hour TMC was conducted at Pleasant Street at Olmsted Drive in November 2020. In order to account for the reduction in traffic volumes caused by COVID-19 travel restrictions, historic traffic count data conducted in April 2018 and November 2019 in the same study area was reviewed. It is important to note that the 2018 data were obtained from the earlier town wide traffic study conducted by the Town of Belmont. Based upon this comparison, the November 2020 weekday morning and evening peak-hour volumes were found to be approximately 40 percent lower. The traffic counts that form the basis of this assessment were adjusted upward by 40 percent in order to provide an appropriate and conservative estimate of roadway operating conditions. It is important to note that in order to establish a 2021 Baseline condition, the November 2020 existing traffic volumes were grown by 1.0 percent per year.

FUTURE TRAFFIC VOLUMES

General Background Traffic Growth

Traffic-volume data compiled by MassDOT from permanent count stations and historic traffic counts in the area were reviewed in order to determine general background traffic growth trends. Based on a review of this data, it was determined that traffic volumes within the study area have fluctuated over the past several years. In order to be consistent with previous traffic studies in the area, a 1.0 percent per year compounded annual growth rate was used to account for general background traffic growth for weekday morning and evening peak hour.

No-Build Traffic Volumes

The 2028 No-Build traffic-volume networks were developed by applying the 1 percent per year compounded annual background traffic growth rate to the 2021 baseline condition peak-hour traffic volumes plus the trips from identified background developments (1010 Pleasant Street - Proposed Marijuana Facility).

Project-Generated Traffic – Zone 3

The development proposal entails construction of 40 for sale, age-restricted townhouse condominiums and 110 multi-family residences comprised of 53 age-restricted units and 57 non-age restricted units. In order to estimate the trip-generation characteristics of the proposed development, the Institute of Transportation Engineers (ITE) *Trip Generation* manual² for ITE Land Use Code (LUC) 221, *Multifamily Housing (Mid-Rise)* and LUC 252 *Senior Adult Housing* were used to project traffic volume of the Project. Adjustments were applied to account for transit usage.

The proposed 150 housing units are expected to generate approximately 610 vehicle trips on an average weekday (two-way, 24-hour volume), with 36 vehicle trips (11 vehicles entering and 25 exiting) expected during the weekday morning peak hour and 46 vehicle trips (26 vehicles entering and 20 exiting) expected during the weekday evening peak hour.

Build Traffic Volumes – Zone 3

The proposed development trips were then added to develop volumes for the 2028 No-Build conditions in order to obtain the 2028 Build traffic signal warrant analysis.

²Trip Generation, 10th Edition; Institute of Transportation Engineers; Washington, DC; 2017.





Build Traffic Volumes-Zone 4

An additional condition was analyzed to include trips associated with the Zone 4 subdistrict. Based on the TMMA, Zone 4 entails construction of a research and development building and is expected to generate approximately 1,784 vehicle trips on an average weekday, with 206 vehicle trips expected during the weekday morning peak hour and 180 vehicle trips expected during the weekday evening peak hour. These Zone 4 trips were combined with the Zone 3 trips to obtain the 2028 Build with Zone 3 and Zone 4 volume scenario for the analysis.

TRAFFIC SIGNAL WARRANTS ANALYSIS

The MUTCD³ establishes nine (9) warrants or criteria to evaluate a location for the installation or retention of a traffic signal. At least one of the nine warrants should be satisfied in order to justify the installation of a traffic signal; however, satisfaction of a warrant in and of itself does not justify traffic signal control. An engineering evaluation of the location in question should indicate that the establishment of traffic signal control will improve the overall safety and/or operation of the intersection. Table 1 identifies the nine traffic signal warrants. Table 2 identifies the results of the TSWA for the study intersection under existing and Future conditions.

Table 1 TRAFFIC SIGNAL WARRANTS^a

Warrant No.	Description
1	Eight-Hour Vehicular Volume
1	Condition A – Min. Vehicular Volume ^b
	Condition B – Interruption of Continuous Traffic ^c
2	Four-Hour Vehicular Volume
3	Peak Hour
4	Pedestrian Volume
5	School Crossing
6	Coordinated Signal System
7	Crash Experience
8	Roadway Network
9	Intersection near a Grade Crossing

^aMUTCD; Federal Highway Administration; Washington, DC; 2009.

³Manual on Uniform Traffic Control Devices (MUTCD); Federal Highway Administration; Washington, DC; 2009.



3

^bA large number of intersecting traffic is the principal reason to consider installing a traffic control signal.

^cTraffic volume on a major street is so heavy that traffic on a minor intersecting street suffer excessive delay in entering or crossing the major street.

Table 2
TRAFFIC SIGNAL WARRANTS ANALYSIS RESULTS

Warrant No.		Satisfied for 2021 Existing Conditions	Satisfied for 2028 No-Build Conditions	Satisfied for 2028 Build Conditions w/Zone 3	Satisfied for 2028 Build Conditions w/Zones 3 and 4
1	Eight-Hour Vehicular Volume				
	Condition A – Min. Vehicular Volume	No	No	No	No
	Condition B – Interruption of Continuous Traffic	No	No	No	No
2	Four-Hour Vehicular Volume	No	No	No	No
3	Peak Hour	No	No	No	No
4	Pedestrian Volume	No	No	No	No
5	School Crossing	No	No	No	No
6	Coordinated Signal System	No	No	No	No
7	Crash Experience	No	No	No	No
8	Roadway Network	No	No	No	No
9	Grade Crossing	No	No	No	No

As indicated in Table 2, under all conditions analyzed the intersection of Pleasant Street at Olmsted Drive does not meet any of the 9 warrant criteria. Accordingly, the installation of a traffic signal at this intersection is not recommended. The detailed TSWA worksheets are provided in the Appendix.

Of note, a review of motor vehicle crash data available from the Massachusetts Department of Transportation (MassDOT) for the subject intersection indicates that only one (1) motor vehicle crash was reported to have occurred at the subject intersection between 2013 and 2017 (the most recent 5-year period for which data is available).

CONCLUSION

VAI has completed a detailed Traffic Signal Warrants Analysis for the intersection of Pleasant Street at Olmsted Drive in Belmont, Massachusetts, as part of the Mclean Hospital Zone 3 Residential Development to assess the warrants required to justify the installation of a traffic control signal at the subject intersection. This analysis was performed in accordance with the criteria established in the MUTCD for conducting such analyses and has determined that the installation of a traffic control signal at the intersection is not warranted under any of the traffic volume conditions. In the event that the warrant criteria is not met and given the low incidence of motor vehicle crashes occurring at the intersection during the five-year review period, the MUTCD and MassDOT guidelines clearly note that a traffic signal shall not be installed.



APPENDIX

AUTOMATIC TRAFFIC RECORDER SEASONAL ADJUSTMENTS MOTOR VEHICLE CRASH DATA GENERAL BACKGROUND TRAFFIC GROWTH TRAFFIC SIGNAL WARRANT ANALYSIS (TSWA)



Location: Pleasant Street
Location: East of Olmsted Drive

Percent

36.3%

63.7%

City/State: Belmont, MA 8058VL01

Start	11/10/202	E	В	Hour	Totals	V	VB	Hour	Totals	Combin	ed Totals
Time	Tue	Morning	Afternoon								
12:00		5	69			1	64				
12:15		4	66			4	88				
12:30		1	52			2	76				
12:45		3	72	13	259	1	64	8	292	21	551
01:00		2	66	.0	200	1	74	J			
01:15		2	90			4	61				
01:30		1	83			1	66				
01:45		1	77	6	316	1	70	7	271	13	587
02:00		4	67	-		0	72	-			
02:15		1	72			0	75				
02:30		2	78			1	99				
02:45		2	80	9	297	2	103	3	349	12	646
03:00		1	88	-		2	95	-	0.10		
03:15		2	88			3	75				
03:30		0	91			0	99				
03:45		4	94	7	361	4	97	9	366	16	727
04:00		2	113		33.	1	90	J			
04:15		2 7	77			2	81				
04:30		3	113			5	97				
04:45		3	88	15	391	15	116	23	384	38	775
05:00		6	95	.0	00.	8	99	_0	00.	33	
05:15		5	92			9	106				
05:30		13	73			19	88				
05:45		22	77	46	337	26	65	62	358	108	695
06:00		17	76	10	00.	41	66	02	000	100	000
06:15		35	54			41	73				
06:30		54	56			57	56				
06:45		58	47	164	233	70	58	209	253	373	486
07:00		53	75		200	76	45		200	0.0	.00
07:15		62	49			71	37				
07:30		93	43			85	36				
07:45		81	35	289	202	84	52	316	170	605	372
08:00		79	45			86	31	0.0		000	0.2
08:15		68	33			100	29				
08:30		72	25			72	30				
08:45		57	30	276	133	86	23	344	113	620	246
09:00		59	25	0		91	23			5.30	
09:15		54	33			62	22				
09:30		58	24			63	17				
09:45		62	18	233	100	74	22	290	84	523	184
10:00		57	21			69	16				
10:15		65	12			49	20				
10:30		61	18			68	17				
10:45		57	8	240	59	72	16	258	69	498	128
11:00		67	13	0		61	9	_30			.20
11:15		65	13			57	11				
11:30		56	15			64	8				
11:45		70	5	258	46	68	4	250	32	508	78
Total		1556	2734		.,,	1779	2741			3335	5475
		.000								5000	5.70

39.4%

60.6%

37.9%

62.1%

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA 8058VL01

Start	11/11/202	F	В	Hour	Totals	W	/B	Hour	Totals	Combin	ed Totals
Time	Wed	Morning	Afternoon								
12:00		3	87			2	76				
12:15		3	67			5	88				
12:30		0	82			5	82				
12:45		0	84	6	320	3	79	15	325	21	645
01:00		2	66			0	68				
01:15		1	64			4	82				
01:30		2	86			5	80				
01:45		0	73	5	289	1	92	10	322	15	611
02:00			66			1	90				
02:15		2	71			4	95				
02:30		3	100			0	86				
02:45		1	85	9	322	4	79	9	350	18	672
03:00		2	84			2	93				
03:15		1	90			0	101				
03:30		0	103			2	92				
03:45		0	77	3	354	1	106	5	392	8	746
04:00		3	89			1	71				
04:15		3	102			3	81				
04:30		0	80			3	84				
04:45		2	98	8	369	11	100	18	336	26	705
05:00		3	85			6	78				
05:15		1	93			11	78				
05:30		11	75			11	69				
05:45		15	66	30	319	23	73	51	298	81	617
06:00		21	73			28	75				
06:15		28	43			36	65				
06:30		39	56			50	49				
06:45		44	42	132	214	48	54	162	243	294	457
07:00		53	46			59	36				
07:15		46	29			63	36				
07:30		64	33			59	29				
07:45		60	44	223	152	53	33	234	134	457	286
08:00		60	40			64	36				
08:15		51	28			67	22				
08:30		51	27			61	28				
08:45		52	29	214	124	90	23	282	109	496	233
09:00		48	25			74	15				
09:15		44	28			63	23				
09:30		56	22			63	24				
09:45		59	16	207	91	77	20	277	82	484	173
10:00		69	17			71	23				
10:15		69	14			70	14				
10:30		75	14			60	19				
10:45		70	13	283	58	88	13	289	69	572	127
11:00		52	10			71	7				
11:15		80	12			76	7				
11:30		66	15			71	12				
11:45		91	8	289	45	86	2	304	28	593	73
Total		1409	2657			1656	2688			3065	5345
Percent		34.7%	65.3%			38.1%	61.9%			36.4%	63.6%
Grand		2965	5391			3435	5429			6400	10820
Total											
Percent		35.5%	64.5%			38.8%	61.2%			37.2%	62.8%

ADT ADT 8,610 AADT 8,610

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA

Start	11/9/2	020	Tı		W	ed	Th	u	Fi	ri	Sa	ıt	Su	n	Week A	verage
Time	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	WB	EB	ŴВ
12:00 AM	*	*	13	8	6	15	*	*	*	*	*	*	*	*	10	12
01:00	*	*	6	7	5	10	*	*	*	*	*	*	*	*	6	8
02:00	*	*	9	3	9	9	*	*	*	*	*	*	*	*	9	6
03:00	*	*	7	9	3	5	*	*	*	*	*	*	*	*	5	7
04:00	*	*	15	23	8	18	*	*	*	*	*	*	*	*	12	20
05:00	*	*	46	62	30	51	*	*	*	*	*	*	*	*	38	56
06:00	*	*	164	209	132	162	*	*	*	*	*	*	*	*	148	186
07:00	*	*	289	316	223	234	*	*	*	*	*	*	*	*	256	275
08:00	*	*	276	344	214	282	*	*	*	*	*	*	*	*	245	313
09:00	*	*	233	290	207	277	*	*	*	*	*	*	*	*	220	284
10:00	*	*	240	258	283	289	*	*	*	*	*	*	*	*	262	274
11:00	*	*	258	250	289	304	*	*	*	*	*	*	*	*	274	277
12:00 PM	*	*	259	292	320	325	*	*	*	*	*	*	*	*	290	308
01:00	*	*	316	271	289	322	*	*	*	*	*	*	*	*	302	296
02:00	*	*	297	349	322	350	*	*	*	*	*	*	*	*	310	350
03:00	*	*	361	366	354	392	*	*	*	*	*	*	*	*	358	379
04:00	*	*	391	384	369	336	*	*	*	*	*	*	*	*	380	360
05:00	*	*	337	358	319	298	*	*	*	*	*	*	*	*	328	328
06:00	*	*	233	253	214	243	*	*	*	*	*	*	*	*	224	248
07:00	*	*	202	170	152	134	*	*	*	*	*	*	*	*	177	152
08:00	*	*	133	113	124	109	*	*	*	*	*	*	*	*	128	111
09:00	*	*	100	84	91	82	*	*	*	*	*	*	*	*	96	83
10:00	*	*	59	69	58	69	*	*	*	*	*	*	*	*	58	69
11:00	*	*	46	32	45	28	*	*	*	*	*	*	*	*	46	30
Lane	0	0	4290	4520	4066	4344	0	0	0	0	0	0	0	0	4182	4432
Day	0		881	0	841	10	0		0		0		0		861	4
AM Peak	-	-	07:00	08:00	11:00	11:00	-	-	-	-	-	-	-	-	11:00	08:00
Vol.		-	289	344	289	304	-	-	- ,		-				274	313
PM Peak	-	-	16:00	16:00	16:00	15:00	-	-	-	-	-	-	-	-	16:00	15:00
Vol.	-	-	391	384	369	392	-	-	-	-	-	-	-	-	380	379
Comb. Total	0		8	8810	8	3410		0		0		0		0	8	614
ADT	AD	T 8,610	AAI	OT 8,610												

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA 8058SP01

EΒ

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	4	5	4	0	0	0	0	0	0	0	0	13
01:00	0	0	0	0	5	1	0	0	0	0	0	0	0	0	6
02:00	0	0	0	3	4	2	0	0	0	0	0	0	0	0	9
03:00	0	0	1	2	0	4	0	0	0	0	0	0	0	0	7
04:00	0	0	1	4	8	0	2	0	0	0	0	0	0	0	15
05:00	0	0	1	19	18	8	0	0	0	0	0	0	0	0	46
06:00	0	2	12	40	85	24	1	0	0	0	0	0	0	0	164
07:00	0	2	17	67	144	56	2	1	0	0	0	0	0	0	289
08:00	0	0	16	83	122	49	6	0	0	0	0	0	0	0	276
09:00	0	2	10	69	112	39	1	0	0	0	0	0	0	0	233
10:00	0	2	17	83	117	16	5	0	0	0	0	0	0	0	240
11:00	0	1	20	100	109	28	0	0	0	0	0	0	0	0	258
12 PM	0	1	18	103	111	25	1	0	0	0	0	0	0	0	259
13:00	0	0	16	115	148	36	1	0	0	0	0	0	0	0	316
14:00	0	0	21	84	148	40	4	0	0	0	0	0	0	0	297
15:00	1	0	24	111	183	40	2	0	0	0	0	0	0	0	361
16:00	1	3	33	153	164	34	3	0	0	0	0	0	0	0	391
17:00	0	0	38	143	130	26	0	0	0	0	0	0	0	0	337
18:00	0	0	16	113	84	19	1	0	0	0	0	0	0	0	233
19:00	0	1	13	92	84	11	1	0	0	0	0	0	0	0	202
20:00	0	3	11	50	57	8	4	0	0	0	0	0	0	0	133
21:00	0	0	1	40	43	13	3	0	0	0	0	0	0	0	100
22:00	0	0	3	17	24	13	1	0	1	0	0	0	0	0	59
23:00	0	0	2	16	17	8	3	0	0	0	0	0	0	0	46
Total	2	17	291	1511	1922	504	41	1	1	0	0	0	0	0	4290

Daily

 15th Percentile:
 26 MPH

 50th Percentile:
 30 MPH

 85th Percentile:
 34 MPH

 95th Percentile:
 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3433

 Percent in Pace:
 80.0%

 Number of Vehicles > 30 MPH:
 2469

 Percent of Vehicles > 30 MPH:
 57.6%

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA

$\overline{}$

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	0	0	3	2	1	0	0	0	0	0	0	0	0	6
01:00	0	0	0	2	2	1	0	0	0	0	0	0	0	0	5
02:00	0	0	2	3	3	1	0	0	0	0	0	0	0	0	9
03:00	0	0	1	0	1	1	0	0	0	0	0	0	0	0	3
04:00	0	0	0	2	5	1	0	0	0	0	0	0	0	0	8
05:00	0	0	5	9	11	3	2	0	0	0	0	0	0	0	30
06:00	0	3	7	35	60	24	3	0	0	0	0	0	0	0	132
07:00	0	1	13	62	108	35	3	1	0	0	0	0	0	0	223
08:00	0	0	12	62	102	31	5	2	0	0	0	0	0	0	214
09:00	0	0	9	59	111	24	4	0	0	0	0	0	0	0	207
10:00	1	2	15	76	143	44	2	0	0	0	0	0	0	0	283
11:00	0	1	12	119	115	36	6	0	0	0	0	0	0	0	289
12 PM	1	4	17	109	138	46	4	1	0	0	0	0	0	0	320
13:00	0	1	9	91	153	33	2	0	0	0	0	0	0	0	289
14:00	0	3	30	113	138	36	2	0	0	0	0	0	0	0	322
15:00	0	1	41	133	133	42	4	0	0	0	0	0	0	0	354
16:00	0	5	38	122	167	34	3	0	0	0	0	0	0	0	369
17:00	0	0	30	122	135	32	0	0	0	0	0	0	0	0	319
18:00	0	1	18	81	96	17	0	1	0	0	0	0	0	0	214
19:00	0	1	6	48	79	18	0	0	0	0	0	0	0	0	152
20:00	0	0	3	47	64	10	0	0	0	0	0	0	0	0	124
21:00	0	1	2	12	61	13	2	0	0	0	0	0	0	0	91
22:00	0	1	1	23	25	8	0	0	0	0	0	0	0	0	58
23:00	0	1	3	15	17	9	0	0	0	0	0	0	0	0	45
Total	2	26	274	1348	1869	500	42	5	0	0	0	0	0	0	4066

Daily 15th Percentile: 26 MPH

50th Percentile: 31 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

59.4%

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3217

 Percent in Pace:
 79.1%

 Number of Vehicles > 30 MPH:
 2416

Grand Total 4 43 565 2859 3791 1004 83 6 1 0 0 0 0 0 0 8356

Overall 15th Percentile: 26 MPH

Percent of Vehicles > 30 MPH:

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6650

 Percent in Pace:
 79.6%

 Number of Vehicles > 30 MPH:
 4885

 Percent of Vehicles > 30 MPH:
 58.5%

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA

WB

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	3	3	0	2	0	0	0	0	0	0	0	8
01:00	0	0	0	0	4	2	1	0	0	0	0	0	0	0	7
02:00	0	0	0	0	2	1	0	0	0	0	0	0	0	0	3
03:00	0	2	1	2	2	2	0	0	0	0	0	0	0	0	9
04:00	0	1	3	4	8	6	1	0	0	0	0	0	0	0	23
05:00	0	0	2	11	27	20	1	1	0	0	0	0	0	0	62
06:00	3	3	29	68	85	19	1	1	0	0	0	0	0	0	209
07:00	1	2	30	100	130	45	6	0	1	0	0	0	0	1	316
08:00	2	1	34	95	147	62	1	0	0	0	0	0	0	2	344
09:00	2	0	15	79	124	65	4	0	0	0	0	0	1	0	290
10:00	2	4	26	70	113	36	5	1	0	0	0	0	0	1	258
11:00	1	2	25	73	111	34	4	0	0	0	0	0	0	0	250
12 PM	3	1	28	85	122	50	2	0	0	0	0	0	0	1	292
13:00	1	7	21	84	120	31	7	0	0	0	0	0	0	0	271
14:00	1	2	26	106	159	49	4	1	1	0	0	0	0	0	349
15:00	1	2	40	110	154	54	5	0	0	0	0	0	0	0	366
16:00	1	4	62	138	139	31	7	2	0	0	0	0	0	0	384
17:00	0	4	40	172	115	26	1	0	0	0	0	0	0	0	358
18:00	1	3	30	113	87	16	2	1	0	0	0	0	0	0	253
19:00	0	1	22	71	58	17	1	0	0	0	0	0	0	0	170
20:00	1	3	21	40	34	12	2	0	0	0	0	0	0	0	113
21:00	1	0	9	30	33	10	1	0	0	0	0	0	0	0	84
22:00	0	0	4	30	27	7	0	0	1	0	0	0	0	0	69
23:00	0	0	0	12	12	4	4	0	0	0	0	0	0	0	32
Total	21	42	468	1496	1816	599	62	7	3	0	0	0	1	5	4520

Daily

 15th Percentile:
 25 MPH

 50th Percentile:
 30 MPH

 85th Percentile:
 34 MPH

 95th Percentile:
 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 3312

 Percent in Pace:
 73.3%

 Number of Vehicles > 30 MPH:
 2493

 Percent of Vehicles > 30 MPH:
 55.2%

8058SP01

Accurate Counts

978-664-2565

Location: Pleasant Street
Location: East of Olmsted Drive

City/State: Belmont, MA 8058SP01

WR

Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	1	1	3	9	1	0	0	0	0	0	0	0	0	15
01:00	0	0	3	0	3	3	1	0	0	0	0	0	0	0	10
02:00	0	0	0	4	3	2	0	0	0	0	0	0	0	0	9
03:00	0	1	1	2	0	1	0	0	0	0	0	0	0	0	5
04:00	0	0	0	2	8	8	0	0	0	0	0	0	0	0	18
05:00	1	0	2	16	19	9	1	0	1	0	0	0	0	2	51
06:00	0	3	19	46	62	26	5	0	0	0	0	0	0	1	162
07:00	4	2	30	70	86	32	8	0	0	0	0	0	0	2	234
08:00	4	0	14	99	128	34	1	1	0	0	0	0	0	1	282
09:00	0	1	12	84	127	48	5	0	0	0	0	0	0	0	277
10:00	3	2	23	104	123	33	1	0	0	0	0	0	0	0	289
11:00	3	5	26	92	133	39	5	0	1	0	0	0	0	0	304
12 PM	2	1	27	97	140	54	4	0	0	0	0	0	0	0	325
13:00	3	3	27	107	142	36	4	0	0	0	0	0	0	0	322
14:00	3	9	43	134	129	28	3	1	0	0	0	0	0	0	350
15:00	1	3	43	152	145	45	2	1	0	0	0	0	0	0	392
16:00	3	2	46	155	106	23	1	0	0	0	0	0	0	0	336
17:00	0	3	42	139	89	24	1	0	0	0	0	0	0	0	298
18:00	0	1	31	88	107	13	3	0	0	0	0	0	0	0	243
19:00	1	4	23	42	52	10	2	0	0	0	0	0	0	0	134
20:00	2	4	6	47	38	12	0	0	0	0	0	0	0	0	109
21:00	0	1	8	30	31	10	2	0	0	0	0	0	0	0	82
22:00	0	2	4	36	22	4	1	0	0	0	0	0	0	0	69
23:00	0	0	3	10	12	3	0	0	0	0	0	0	0	0	28
Total	30	48	434	1559	1714	498	50	3	2	0	0	0	0	6	4344

Daily

15th Percentile: 25 MPH 50th Percentile: 30 MPH 85th Percentile: 34 MPH

85th Percentile: 34 MPH 95th Percentile: 38 MPH

Mean Speed(Average): 31 MPH 10 MPH Pace Speed: 26-35 MPH Number in Pace: 3273

Percent in Pace: 75.3%
Number of Vehicles > 30 MPH: 2273
Percent of Vehicles > 30 MPH: 52.3%

Grand Total	51	٩n	902	3055	3530	1097	112	10	5	Ω	Λ	Ο	1	11	8864
Gianu i olai	31	90	902	3033	3330	1091	112	10	3	U	U	U		1.1	0004

Overall 15th Percentile : 25 MPH 50th Percentile : 30 MPH

85th Percentile: 34 MPH 95th Percentile: 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6585

 Percent in Pace:
 74.3%

Number of Vehicles > 30 MPH: 4766
Percent of Vehicles > 30 MPH: 53.8%

Location: Pleasant Street Location: East of Olmsted Drive

City/State: Belmont, MA

EB, WB

<u> </u>															
Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/10/20	0	0	0	7	8	4	2	0	0	0	0	0	0	0	21
01:00	0	0	0	0	9	3	1	0	0	0	0	0	0	0	13
02:00	0	0	0	3	6	3	0	0	0	0	0	0	0	0	12
03:00	0	2	2	4	2	6	0	0	0	0	0	0	0	0	16
04:00	0	1	4	8	16	6	3	0	0	0	0	0	0	0	38
05:00	0	0	3	30	45	28	1	1	0	0	0	0	0	0	108
06:00	3	5	41	108	170	43	2	1	0	0	0	0	0	0	373
07:00	1	4	47	167	274	101	8	1	1	0	0	0	0	1	605
08:00	2	1	50	178	269	111	7	0	0	0	0	0	0	2	620
09:00	2	2	25	148	236	104	5	0	0	0	0	0	1	0	523
10:00	2	6	43	153	230	52	10	1	0	0	0	0	0	1	498
11:00	1	3	45	173	220	62	4	0	0	0	0	0	0	0	508
12 PM	3	2	46	188	233	75	3	0	0	0	0	0	0	1	551
13:00	1	7	37	199	268	67	8	0	0	0	0	0	0	0	587
14:00	1	2	47	190	307	89	8	1	1	0	0	0	0	0	646
15:00	2	2	64	221	337	94	7	0	0	0	0	0	0	0	727
16:00	2	7	95	291	303	65	10	2	0	0	0	0	0	0	775
17:00	0	4	78	315	245	52	1	0	0	0	0	0	0	0	695
18:00	1	3	46	226	171	35	3	1	0	0	0	0	0	0	486
19:00	0	2	35	163	142	28	2	0	0	0	0	0	0	0	372
20:00	1	6	32	90	91	20	6	0	0	0	0	0	0	0	246
21:00	1	0	10	70	76	23	4	0	0	0	0	0	0	0	184
22:00	0	0	7	47	51	20	1	0	2	0	0	0	0	0	128
23:00	0	0	2	28	29	12	7	0	0	0	0	0	0	0	78
Total	23	59	759	3007	3738	1103	103	8	4	0	0	0	1	5	8810

Daily 15th Percentile :

 15th Percentile:
 25 MPH

 50th Percentile:
 30 MPH

 85th Percentile:
 34 MPH

 95th Percentile:
 38 MPH

 Mean Speed(Average):
 31 MPH

 10 MPH Pace Speed:
 26-35 MPH

 Number in Pace:
 6745

 Percent in Pace:
 76.6%

 Number of Vehicles > 30 MPH:
 4962

 Percent of Vehicles > 30 MPH:
 56.3%

Location: Pleasant Street

Location: East of Olmsted Drive City/State: Belmont, MA

FB	WB

LD, **D															
Start	1	16	21	26	31	36	41	46	51	56	61	66	71	76	
Time	15	20	25	30	35	40	45	50	55	60	65	70	75	999	Total
11/11/20	0	1	1	6	11	2	0	0	0	0	0	0	0	0	21
01:00	0	0	3	2	5	4	1	0	0	0	0	0	0	0	15
02:00	0	0	2	7	6	3	0	0	0	0	0	0	0	0	18
03:00	0	1	2	2	1	2	0	0	0	0	0	0	0	0	8
04:00	0	0	0	4	13	9	0	0	0	0	0	0	0	0	26
05:00	1	0	7	25	30	12	3	0	1	0	0	0	0	2	81
06:00	0	6	26	81	122	50	8	0	0	0	0	0	0	1	294
07:00	4	3	43	132	194	67	11	1	0	0	0	0	0	2	457
08:00	4	0	26	161	230	65	6	3	0	0	0	0	0	1	496
09:00	0	1	21	143	238	72	9	0	0	0	0	0	0	0	484
10:00	4	4	38	180	266	77	3	0	0	0	0	0	0	0	572
11:00	3	6	38	211	248	75	11	0	1	0	0	0	0	0	593
12 PM	3	5	44	206	278	100	8	1	0	0	0	0	0	0	645
13:00	3	4	36	198	295	69	6	0	0	0	0	0	0	0	611
14:00	3	12	73	247	267	64	5	1	0	0	0	0	0	0	672
15:00	1	4	84	285	278	87	6	1	0	0	0	0	0	0	746
16:00	3	7	84	277	273	57	4	0	0	0	0	0	0	0	705
17:00	0	3	72	261	224	56	1	0	0	0	0	0	0	0	617
18:00	0	2	49	169	203	30	3	1	0	0	0	0	0	0	457
19:00	1	5	29	90	131	28	2	0	0	0	0	0	0	0	286
20:00	2	4	9	94	102	22	0	0	0	0	0	0	0	0	233
21:00	0	2	10	42	92	23	4	0	0	0	0	0	0	0	173
22:00	0	3	5	59	47	12	1	0	0	0	0	0	0	0	127
23:00	0	1	6	25	29	12	0	0	0	0	0	0	0	0	73
Total	32	74	708	2907	3583	998	92	8	2	0	0	0	0	6	8410

25 MPH Daily 15th Percentile:

50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

Mean Speed(Average): 31 MPH 10 MPH Pace Speed: 26-35 MPH Number in Pace : 6490 Percent in Pace : 77.2% Number of Vehicles > 30 MPH: 4689

133 1467 5914 7321 2101 195 17220 Grand Total

25 MPH Overall 15th Percentile:

Percent of Vehicles > 30 MPH:

Percent of Vehicles > 30 MPH:

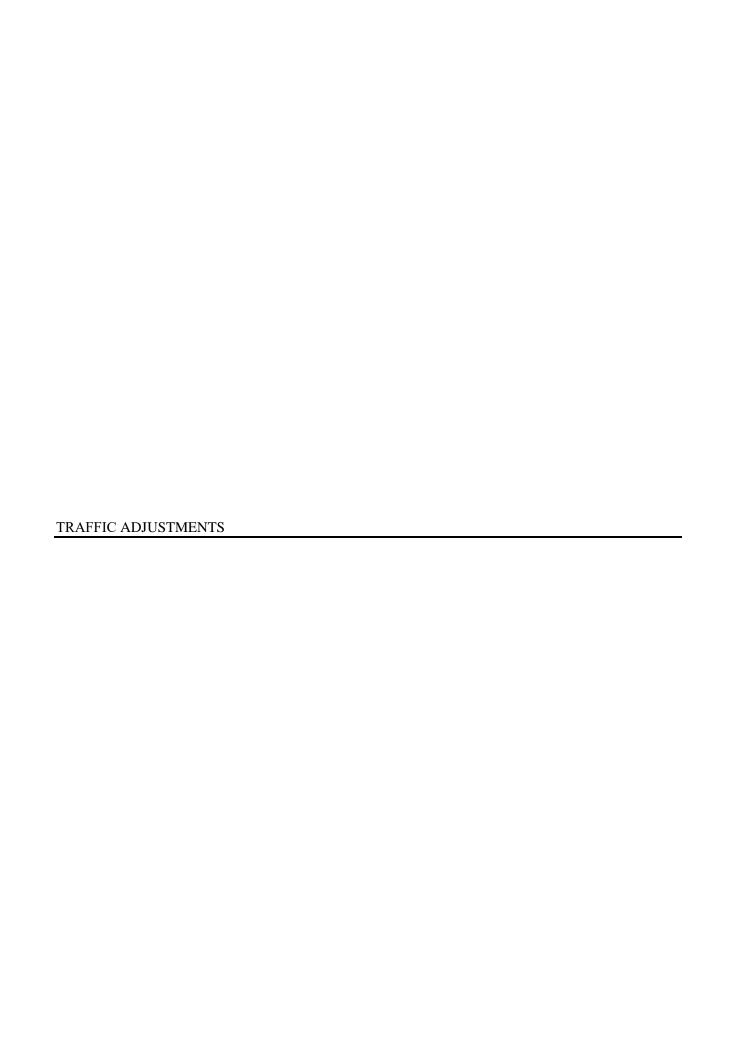
50th Percentile: 30 MPH 85th Percentile: 34 MPH 95th Percentile: 38 MPH

Mean Speed(Average): 31 MPH 10 MPH Pace Speed : Number in Pace : 26-35 MPH 13235 Percent in Pace : 76.9% Number of Vehicles > 30 MPH:

9651 56.0%

55.8%

8058SP01



Comment 1: N/S Street : Trapelo Road
Comment 2: E/W Street : Mill Street
Comment 3: City/State : Belmont, MA
Comment 4: Weather : Clear

	Comment 4.	veaulei . C		Wednesday, A	pril 11, 2018 (6:00 AM										Tuesday,	November, 10	2020					
		Mill St		1	rapelo Rd		1	Trapelo Rd						Mill St			Trapelo Rd		T	rapelo Rd			
	F	rom North			From East		F	From West					F	rom North			From East		F	rom West			
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Total	Peak	Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds	Total	
7:00:00 AM	30	124	0	108	76	0	149	34	0	521		7:00:00 AM	56	19	0	75	72	0	22	89	0	333	
7:15:00 AM	31	125	0	136	131	0	150	35	0	608		7:15:00 AM	93	24	0	70	77	0	30	103	0	397	
7:30:00 AM	43	105	0	131	141	0	170	29	0	619		7:30:00 AM	89	27	0	81	81	0	27	131	0	436	
7:45:00 AM	25	136	0	175	146	0	163	25	0	670	2418	7:45:00 AM	91	23	0	89	97	0	27	104	0	431	1597
8:00:00 AM	50	150	0	162	147	0	150	34	0	693	2590	8:00:00 AM	89	22	0	105	103	0	18	114	0	451	1715
8:15:00 AM	53	149	0	129	169	0	132	36	0	668	2650	8:15:00 AM	94	36	0	87	110	0	21	109	0	457	1775
8:30:00 AM	69	142	0	152	150	0	138	19	0	670	2701	8:30:00 AM	91	26	0	91	113	0	15	102	0	438	1777
8:45:00 AM	69	145	0	142	154	0	134	29	0	673	2704	8:45:00 AM	74	24	0	92	91	0	30	100	0	411	1757
	370	1076	0	1135	1114	0	1186	241	0	5122	2704		677	201	0	690	744	0	190	852	0	3354	1777
								Seasona	al Adj 1 %	5173	2731								Co	omparisson		1.57	1.57
							202	20 Adjuste	d 1% year	5277	2786								Say	COVID A	dj į	1.5	7
4:00:00 PM	52	169	0	121	166	0	123	47	0	678		4:00:00 PM	88	29	0	113	90	0	44	152	0	516	
4:15:00 PM	41	136	0	121	177	0	134	23	0	632		4:15:00 PM	109	39	0	123	92	0	24	113	0	500	
4:30:00 PM	37	148	0	93	177	0	166	35	0	656		4:30:00 PM	114	33	0	123	89	0	27	132	0	518	
4:45:00 PM	40	151	0	91	183	0	150	32	0	647	2613		101	39	0	148	76	0	33	123	0	520	2054
5:00:00 PM	55	128	0	91	170	0	156	36	0	636	2571	5:00:00 PM	95	21	0	143	70	0	22	132	0	483	2034
5:15:00 PM	46	137	0	130	175	0	145	43	0	676	2615		102	34	0	168	77	0	22	107	0	510	2021
5:30:00 PM	33	140	0	100	173	0	163	35	0	661	2620		85	20	0	97	71	0	26	99	0	398	1911
5:45:00 PM	44	179	0	77	206	0	147	28	0	681	2654	5:45:00 PM	57	32	0	103	52	0	28	103	0	375	1766
3.43.00 i Wi	348	1188	0	824	1444	0	1184	279	0	5267	2654	3.43.00 T W	751	247	0		617	0	226	961	0	3820	2054
	340	1100	U	024	1444	U	1104		al Adj 1 %	5320	2681		731	241	U	1010	017	U I		mparisson	U	1.42	1.33
							201	20 Adjuste	•		2735							4		/ COVID A		1.42	
							202												Jaj		<u> </u>		
								TOTA	AL Peaks	10704	5521										L Peaks	7174	3831
																		Į.		omparisson		1.49	1.44
																			Say	/ COVID Ad	dj .	1.5	J

Comment 1: N/S Street : Pleasant Street
Comment 2: E/W Street : Trapelo Road
Comment 3: City/State : Belmont, MA
Comment 4: Weather : Clear

`	Johnnent 4.	would .	Cicai																			
_				Thrusday	, November 14											November, 10						
		Pleasant S	t		Trapelo Ro	1		Trapelo Rd					Pleasant St		1	Trapelo Rd			Frapelo Rd			
		From North			From East			From West					From North			From East		F	rom West			
Start Time	Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds		Start T	ime Left	Right	Peds	Thru	Right	Peds	Left	Thru	Peds		
7:00:00 AM											7:00:0) AM 3	2 37	0	116	14	0	37	111	0		
7:15:00 AM											7:15:0) AM 3	7 36	0	101	17	0	46	146	0		
7:30:00 AM											7:30:0) AM 4	3 41	0	126	33	0	62	157	0		
7:45:00 AM											7:45:0) AM 4	3 41	0	145	34	0	40	160	0		
8:00:00 AM											8:00:0) AM 3	5 54	0	157	37	0	44	145	0		
8:15:00 AM											8:15:0) AM 4	5 50	0	145	27	0	41	169	0		
8:30:00 AM											8:30:0) AM 2	7 45	0	157	26	0	41	151	0		
8:45:00 AM											8:45:0) AM 3	1 54	0	128	19	0	38	155	0		
4:00:00 PM	52	. 71	0) 228	3 42	0	68	212	0	673	4:00:0) PM 3	5 63	0	158	32	0	73	168	0	529	
4:15:00 PM	50		0	218			69	236	0	699	4:15:0			0	157	35	0	48	168	0	492	
4:30:00 PM	58			193			78	233	0	697	4:30:0			0	166	45	0	67	186	0	560	
4:45:00 PM	46			222			78	234	0	698	2767 4:45:0			0	152	33	0	55	167	0	524	2105
5:00:00 PM	52			180			79	242	0	663	2757 5:00:0) PM 2		0	161	26	0	71	161	0	513	2089
5:15:00 PM	59	67	0	166	78	0	79	227	0	676	2734 5:15:0) PM 4	4 66	0	160	41	0	54	173	0	538	2135
5:30:00 PM	73			172		0	71	194	0	628	2665 5:30:0			0	131	26	0	51	138	0	431	2006
5:45:00 PM	57) 182			65	221	0	630	2597 5:45:0			0	103	25	0	50	109	0	355	1837
										5364	2767										3942	2135
								Seasona	al Adj 1 %	5418	2795						Ī	С	omparisson		1.40	1.34
							20	20 Adjuste		5527	2851								y COVID A		1.40	



Crash Number	City Town Name	Crash Date	Weekday	Crash Severity		Number of Vehicles	Age of Driver - Youngest Known	Driver Contributing Circumstances (All Drivers)	First Harmful Event	Light Conditions	Manner of Collision		Vehicle Actions Prior to Crash (All Vehicles)	Vehicle Travel Directions (All Vehicles)		Latitude	Longitude	Street Numb er	Roadway	Near Intersection Roadway
									PLEASANT ST / O	LMSTED	DRIVE									
4380932	BELMONT	02/24/2017	Friday	Non-fatal injury	3:47 PM	1	35-44	D1: (Glare)	Collision with pedalcycle (bicycle, tricycle, unicycle, pedal car)	Dusk	Angle	Dry \	V1: Turning left	V1: W	Clear	42.38902647	-71.19046006			OLMSTED DRIVE



CRASH RATE WORKSHEET

CITY/TOWN : Belmon	ı <u>t</u>			COUNT DA	NTE :	2020	MHD USE ONLY			
DISTRICT: 4	UNSIGN	ALIZED :	х	SIGNA	LIZED :		Source #			
		~ IN	TERSECTION	N DATA ~						
			ILICEOTIC							
MAJOR STREET :	Pleasant St	reet					ST#			
MINOR STREET(S):	Olmsted Dr	ive					ST#			
							ST#			
INTERRECTION	Alarth.			I						
INTERSECTION DIAGRAM	North		6 2	4	592		REF #			
(Label Approaches)			<u> </u>	١						
		552	3							
			Peak Hou	r Volumes			<u> </u>			
APPROACH:	1	2	3	4	5	Total Entering				
DIRECTION:	NB	SB	EB	WB		Vehicles				
VOLUMES (PM):	0	6	552	592		1,150				
"K" FACTOR:	0.080	APPROA	CH ADT :	14,375	ADT = TOTA	L VOL/"K" FAC	т.			
TOTAL # OF ACCIDENTS :	1	# OF YEARS :	5		GE#OF NTS(A):	0.20				
CRASH RATE CALC	ULATION :	0.04	RATE =	(A * 1,0 (ADT	000,000) * 365)					
	nt Rate for Di						-			

Statewide Accident Rate for Signalized Inteserction = 0.78 and Unsignalized/Inteserction = 0.57





Start Time: 12:00:00 AM Site Code: 82890001 Location: Boston Post Road Location: West of Village Drive City/State: Marlborough, MA

Start Date: 6/13/2019

2020 Raw

		ted Dr North		ant St East	Pleasant St From West		
Start Time	Left	Right	Thru	Right	Left	Thru	
7-8 AM	3	9	307	1	2	283	
8-9 AM	2	2	344	2	2	272	
9-10 AM	3	7	293	2	3	229	
10-11 AM	3	10	252	6	6	241	
11-12 PM	3	3	244	5	5	252	
12-1 PM	3	8	291	2	3	256	
1-2 PM	6	4	260	7	8	309	
2-3 PM	2	5	355	0	5	294	
3-4 PM	6	10	357	8	11	354	
4-5 PM	2	5	391	1	6	387	
5-6 PM	1	4	358	3	3	339	
6-7 PM	7	7	243	5	9	232	

Covid Adj - 2020 Baseline Condition COVID adj 1.4

1% above no seasonal adjustment Pleasant St Pleasant St Olmsted Dr From East From North From West Time Left Right Thru Right Left Thru 7-8 AM 8-9 AM 9-10 AM 10-11 AM 11-12 PM 12-1 PM 1-2 PM

2-3 PM

3-4 PM

4-5 PM

5-6 PM

6-7 PM

2021 Adjustment1 year 1.01

	1 percent per year compounded annual background													
		ted Dr North		ant St East		ant St West								
Time	Left	Right	Thru	Right	Left	Thru								
7-8 AM	4	13	434	1	3	400								
8-9 AM	3	3	487	3	3	385								
9-10 AM	4	10	414	3	4	324								
10-11 AM	4	14	357	8	8	340								
11-12 PM	4	4	345	7	7	357								
12-1 PM	4	11	411	3	4	362								
1-2 PM	8	6	368	10	11	437								
2-3 PM	3	7	502	0	7	416								
3-4 PM	8	14	505	11	15	501								
4-5 PM	4-5 PM 3		552 1		8	547								
5-6 PM	1 6		506	4	4	480								
6-7 PM	10	10	343	7	13	328								



Source: ITE Trip Generation Manual, 10th Edition

Land Use Code 882

Land Use Marijuana Dispensary Setting General Urban/Suburban

Time Period Weekday

Time Period	wee	ekuay				
Trip Type	Vel	nicle	Daily		Daily	
# Data Sites		4	303	303	303	303
	% of 24-H	lour Traffic	Entering	Exiting	Entering	Exiting
Time	Entering	Exiting				
12-1 AM	0	0	0.0	0.0	0	0
1-2 AM	0	0	0.0	0.0	0	0
2-3 AM	0	0	0.0	0.0	0	0
3-4 AM	0	0	0.0	0.0	0	0
4-5 AM	0	0	0.0	0.0	0	0
5-6 AM	0.1	0.1	0.3	0.3	0	0
6-7 AM	0.4	0.1	1.2	0.3	1	0
7-8 AM	1.4	0.2	4.2	0.6	4	1
8-9 AM	4.1	3.6	12.4	10.9	12	11
9-10 AM	5.3	4.3	16.1	13.0	16	13
10-11 AM	8.2	7.5	24.8	22.7	25	23
11-12 PM	8.1	7.9	24.5	23.9	25	24
12-1 PM	9.1	8.9	27.6	27.0	28	27
1-2 PM	8.3	8.0	25.1	24.2	25	24
2-3 PM	8.4	9.3	25.5	28.2	26	28
3-4 PM	9.9	9.6	30.0	29.1	30	29
4-5 PM	11.3	11.4	34.2	34.5	34	34
5-6 PM	12.4	12.4	37.6	37.6	38	38
6-7 PM	12.5	14.7	37.9	44.5	38	45
7-8 PM	0.2	1.6	0.6	4.8	1	5
8-9 PM	0.1	0.2	0.3	0.6	0	1
9-10 PM	0	0.1	0.0	0.3	0	0
10-11 PM	0	0	0.0	0.0	0	0
11-12 AM	0	0	0.0	0.0	0	0
					303	303
Data obtaine	d from the IT	F Canaral I	Irhan/Suhu	rhan - T	rine by tim	of day

Background Development												
			out			in						
	From	North	From	East	From	West						
Start Time	Left	Right	Thru	Right	Left	Thru						
%	0	0	0.65	0	0	0.65						
7-8 AM	0	0	1	0	0	3						
8-9 AM	0	0	7	0	0	8						
9-10 AM	0	0	8	0	0	10						
10-11 AM	0	0	15	0	0	16						
11-12 PM	0	0	16	0	0	16						
12-1 PM	0	0	18	0	0	18						
1-2 PM	0	0	16	0	0	16						
2-3 PM	0	0	18	0	0	17						
3-4 PM	0	0	19	0	0	20						
4-5 PM	0	0	22	0	0	22						
5-6 PM	0	0	25	0	0	25						
6-7 PM	0	0	29	0	0	25						

2028 No- Build													
7 year 1.01													
1 percent pe	er year	compo	unded	annua	l backç	ground							
	Olms	ted Dr	Pleas	ant St	Pleas	ant St							
	From North From East From Wes												
Start Time Left Right Thru Right Left Thr													
7-8 AM	4	13	466	1	3	432							
8-9 AM	3	3	529	3	3	421							
9-10 AM	4	10	452	3	4	357							
10-11 AM	4	14	398	8	8	381							
11-12 PM	4	4	386	7	7	399							
12-1 PM	4	11	459	3	4	406							
1-2 PM	8	6	411	10	11	485							
2-3 PM	3	7	556	0	7	463							
3-4 PM	8	14	560	11	15	557							
4-5 PM	3	7	614	1	8	608							
5-6 PM	1	6	568	4	4	540							
6-7 PM	10	10	397	7	13	377							

Data obtained from the ITE General Urban/Suburban - Trips by time of day



La

Hourly Distribution of Entering and Exiting Vehicle

Source: ITE Trip Generation Manual, 10th Edition

	221 Multifamily Ho General Urba Wee	an/Suburban	ise) 332				Land Use Code Land Use t Setting U Time Period /	Housin Jrban/S	Suburba		278				610
# Data Sites	Veh	iolo					Trip Type '	Vohiolo						305	305
# Data Sites	Ven		Da	ilv	Da	ilv	# Data Sites	verlicie	1	D	aily	Da	ilv		otal
	% of 24-H		166	166	166	166		4-Hour	ı Traffic	139	139	139	139	10	itai
	70 OI 24-II	oui manic	100	100	100	100	70 OI 2-	+-1 10ui	Trailic	100	100	100	100		
Time	Entering	Exiting	Entering	Exitina	Entering	Exiting	Time F	nterino	Exiting	Enterino	Exiting	Entering	Exiting	Entering	Exiting
11110	Lintoning	Exiting	Lintolling	Extung	Lintorning	LAiting	11110 2	-111011119	Landing	Lincolnie	LXIIII	Lintoning	Exiting	Lintorning	Exiting
12-1 AM	0.7	0.3	1.2	0.5	1	0	12-1 AM	0.3	0.4	0.4	0.6	0	1	1	1
1-2 AM	0.3	0.2	0.5	0.3	0	0	1-2 AM	0.2	0.5	0.3	0.7	0	1	0	1
2-3 AM	0.2	0.2	0.3	0.3	0	0	2-3 AM	0	0.1	0.0	0.1	0	0	0	0
3-4 AM	0.4	0.3	0.7	0.5	1	0	3-4 AM	0.1	0.2	0.1	0.3	0	0	1	0
4-5 AM	0.3	8.0	0.5	1.3	0	1	4-5 AM	0.2	0.5	0.3	0.7	0	1	0	2
5-6 AM	0.6	2.7	1.0	4.5	1	4	5-6 AM	0.9	2.2	1.3	3.1	1	3	2	7
6-7 AM	1.5	6.5	2.5	10.8	2	11	6-7 AM	1.3	3.0	1.8	4.2	2	4	4	15
7-8 AM	2.8	12.1	4.6	20.1	5	20	7-8 AM	2.1	5.1	2.9	7.1	3	7	8	27
8-9 AM	3.5	8.8	5.8	14.6	6	15	8-9 AM	3.9	6.3	5.4	8.8	5	9	11	24
9-10 AM	2.9	5.7	4.8	9.5	5	10	9-10 AM	4.7	6.7	6.5	9.3	7	9	12	19
10-11 AM	2.7	4.7	4.5	7.8	5	8	10-11 AM	6.4	7.5	8.9	10.4	9	10	14	18
11-12 PM	4.5	4.5	7.5	7.5	7	8	11-12 PM	6.8	6.5	9.5	9.0	10	9	17	17
12-1 PM	4.8	4.6	8.0	7.6	8	8	12-1 PM	8.5	9.0	11.8	12.5	12	12	20	20
1-2 PM	4.1	4.8	6.8	8.0	7	8	1-2 PM	7.7	8.0	10.7	11.1	11	11	18	19
2-3 PM	5.8	5.0	9.6	8.3	10	8	2-3 PM	9.1	6.7	12.6	9.3	13	9	23	17
3-4 PM	6.7	4.9	11.1	8.1	11	8	3-4 PM	8.7	5.7	12.1	7.9	12	8	23	16
4-5 PM	10.6	6.2	17.6	10.3	18	10	4-5 PM	8.3	6.3	11.5	8.8	12	9	30	19
5-6 PM	12.6	7.7	20.9	12.8	21	13	5-6 PM	7.3	5.7	10.1	7.9	10	8	31	21
6-7 PM	9.3	6.6	15.4	11.0	15	11	6-7 PM	6.3	5.2	8.8	7.2	9	7	24	18
7-8 PM	7.8	4.8	12.9	8.0	13	8	7-8 PM	5.8	5.2	8.1	7.2	8	7	21	15
8-9 PM	7.0	3.3	11.6	5.5	12	6	8-9 PM	4.8	4.1	6.7	5.7	7	6	19	12
9-10 PM	5.5	2.2	9.1	3.7	9	4	9-10 PM	3.1	2.2	4.3	3.1	4	3	13	7
10-11 PM	3.6	1.9	6.0	3.2	6	3	10-11 PM	2.5	1.8	3.5	2.5	3	3	9	6
11-12 AM	2.0	1.1	3.3	1.8	3	2	11-12 AM	8.0	1.1	1.1	1.5	1	2	4	4
					166	166						139	139	305	305

Data obtained from the ITE General Urban/Suburban - Trips by time of day

e Trips by Land Use

Trip Generation

	out	out		ın	ın	
	From	North	From	East	From	West
Time	Left	Right	Thru	Right	Left	Thru
%	0.4	0.6	0	0.4	0.6	0
7-8 AM	11	16	0	3	5	0
8-9 AM	10	14	0	4	7	0
9-10 AM	8	11	0	5	7	0
10-11 AM	7	11	0	6	8	0
11-12 PM	7	10	0	7	10	0
12-1 PM	8	12	0	8	12	0
1-2 PM	8	11	0	7	11	0
2-3 PM	7	10	0	9	14	0
3-4 PM	6	10	0	9	14	0
4-5 PM	8	11	0	12	18	0
5-6 PM	8	13	0	12	19	0
6-7 PM	7	11	0	10	14	0

2028 Build Zone 3

		ted Dr North		ant St East	Pleasant St From West		
Start Time	Left Right		Thru	Right	Left	Thru	
7-8 AM	15	29	466	4	8	432	
8-9 AM	13	17	529	7	10	421	
9-10 AM	12	21	452	8	11	357	
10-11 AM	11	25	398	14	16	381	
11-12 PM	11	14	386	14	17	399	
12-1 PM	12	23	459	11	16	406	
1-2 PM	16	17	411	17	22	485	
2-3 PM	10	17	556	9	21	463	
3-4 PM	14	24	560	20	29	557	
4-5 PM	11	18	614	13	26	608	
5-6 PM	9	19	568	16	23	540	
6-7 PM	17	21	397	17	27	377	



Hourly Distribution of Entering and Exiting Vehicle Trips by Land Use

Source: ITE Trip Generation Manual, 10th Edition

Land Use Code 710

Time Period

8-9 PM

9-10 PM

10-11 PM

11-12 AM

0.7

0.5

0.3

0.4

1.3

1.5

2.0

0.2

Land Use General Office Building

Weekday

Setting General Urban/Suburbar Daily number obtaneid from TMA

Trip Type Vehicle # Data Sites 16 Daily Daily % of 24-Hour Traffic 892 892 892 892 Entering Exiting Entering Exiting Entering Exiting 12-1 AM 0.9 0.2 0.1 1.8 2 1-2 AM 0 0.1 0.0 0.9 0 1 2-3 AM 0 0.0 0.0 0 0 0 0 0 3-4 AM 0.1 0.0 0.9 4-5 AM 0.2 1 2 0.1 0.9 1.8 4 5-6 AM 0.4 0.1 3.6 0.9 41 6-7 AM 4.6 0.5 41.0 4.5 4 17 7-8 AM 13.1 1.9 116.9 16.9 117 8-9 AM 14.4 3.5 128.4 31.2 129 31 9-10 AM 6.4 4.3 38.4 57 38 5.9 48.2 52.6 52 10-11 AM 5.4 48 11-12 PM 10.3 91.9 55 92 6.2 93 12-1 PM 10.2 10.4 91.0 92.8 91 1-2 PM 9.0 6.7 80.3 59.8 81 60 58 2-3 PM 8.2 6.5 73 76 3-4 PM 7.4 8.5 75.8 4-5 PM 5.5 15.2 49 135 49.1 5-6 PM 4.2 15.6 37.5 38 139 6-7 PM 1.7 2.9 25.9 15 26 7-8 PM 0.9 2.2 8 20 8.0 19.6

	Trip Generation											
	out	out		in	in							
	From	North	From	East	From West							
Time	Left	Right	Thru	Right	Left	Thru						
%	0.4	0.6	0	0.4	0.6	0						
7-8 AM	7	10	0	47	70	0						
8-9 AM	12	19	0	52	77	0						
9-10 AM	15	23	0	23	34	0						
10-11 AM	21	31	0	19	29	0						
11-12 PM	37	55	0	22	33	0						
12-1 PM	37	56	0	36	55	0						
1-2 PM	24	36	0	32	49	0						
2-3 PM	23	35	0	29	44	0						
3-4 PM	30	46	0	26	40	0						
4-5 PM	54	81	0	20	29	0						
5-6 PM	56	83	0	15	23	0						

Note: Same distribution of the zone 3 was assumed for zone 4 Developments

0

6

9

16

6-7 PM

12

13

18

2

892

4

3

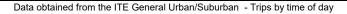
4

892

10

2028 Build - w/ Zone 3 and 4

		ted Dr North		ant St East	Pleasant St From West		
Start Time	Left	Right	Thru	Right	Left	Thru	
7-8 AM	22	39	466	51	78	432	
8-9 AM	25	36	529	59	87	421	
9-10 AM	27	44	452	31	45	357	
10-11 AM	32	56	398	33	45	381	
11-12 PM	48	69	386	36	50	399	
12-1 PM	49	79	459	47	71	406	
1-2 PM	40	53	411	49	71	485	
2-3 PM	33	52	556	38	65	463	
3-4 PM	44	70	560	46	69	557	
4-5 PM	65	99	614	33	55	608	
5-6 PM	65	102	568	31	46	540	
6-7 PM	27	37	397	23	36	377	



6.2

4.5

2.7

3.6

11.6

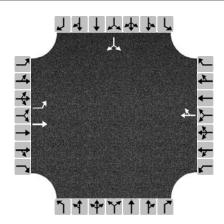
13.4

17.8

1.8



HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2021								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2021 Existing Condition								
	Development - Belmont MA										
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	1								
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach		Eastbound			Westbound			Northbound			Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Number of Lanes, N	1	1	0	0	1	0	0	0	0	0	0	0	
Lane Usage	L	Т			TR						LR		
Vehicle Volumes Averages (veh/h)	7	406	0	0	435	4	0	0	0	4	0	8	
Pedestrian Averages (peds/h)		0			0		0		0				
Gap Averages (gaps/h)		0			0			0		0			
Delay (s/veh)		0.0			0.0			0.0		0.0			
Delay (veh-hrs)		0.0			0.0			0.0		0.0			
School Crossing and Roadway	Netwo	rk											
Number of Students in Highest Hour	0			1	wo or Mo	re Major	Routes		Yes				
Number of Adequate Gaps in Period	0			\	Veekend (end Counts No							

Railroad Crossing

Number of Minutes in Period

rtain odd Grossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

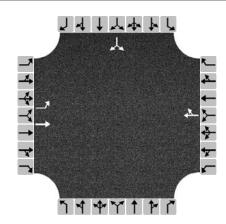
0

5-year Growth Factor (%)

0

	HCS7 Warrants Report													
Volume Si	Volume Summary													
Hour	Major	Minor	Total	Peds/h	Gaps/h	1A	1A	1B	1B	2	3A	3B	4A	4B
Hour	Volume	Volume	Volume	reus/II	Gaps/11	(100%)	(80%)	(100%)	(80%)	(100%)	(100%)	(100%)		(100%)
07 - 08	838	17	855	0	0	No	No	No	No	No	No	No	No	No
08 - 09 878 6 884 0 0 No No No No No No											No	No	No	
09 - 10													No	No
10 - 11 713 18 731 0 0 No No No No No No No													No	No
11 - 12 716 8 724 0 0 No No No No No No No													No	No
12 - 13 780 15 795 0 0 No No No No No No No													No	No
13 - 14	826	14	840	0	0	No	No	No	No	No	No	No	No	No
14 - 15	925	10	935	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1032	22	1054	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1108	10	1118	0	0	No	No	No	No	No	No	No	No	No
17 - 18	994	7	1001	0	0	No	No	No	No	No	No	No	No	No
18 - 19	691	20	711	0	0	No	No	No	No	No	No	No	No	No
Total	10246	161	10407	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1: E	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	m Vehicula	ar Volume:	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d)r					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	ularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	our-Hou	r Vehicul	ar Volun	1e										
Four-Hour	· Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3: I	Peak Hou	r												
A. Peak-Ho	our Condit	ions (Minc	r delay	and min	or volume	and to	otal volum	e)or						
B. Peak-Ho	our Vehicul	lar Volume	es (Both m	ajor appro	achesar	nd highe	r minor ap	proach)						
Warrant 4: I	Pedestria	n Volum	2											
A. Four Ho	ur Volume	esor												
B. One-Ho	ur Volume	S												
Warrant 5: S	School Cr	ossing												
Gaps Same	e Period	and												
Student Vo	olumes													
Nearest Tr	affic Contr	ol Signal (optional)										✓	
Warrant 6: 0	Coordinat	ted Signa	ıl System											
Degree of Platooning (Predominant direction or both directions)														
Warrant 7: Crash Experience														
A. Adequate trials of alternatives, observance and enforcement failedand														
B. Reported crashes susceptible to correction by signal (12-month period)and														
C. 80% Volumes for Warrants 1A, 1B,or 4 are satisfied														
Warrant 8: Roadway Network														
A. Weekday Volume (Peak hour totaland projected warrants 1, 2, or 3)or														
B. Weekend Volume (Five hours total)														
Warrant 9: Grade Crossing														
A. Grade C	Crossing wi	thin 140 ft	:and											
B. Peak-Ho	our Vehicul	lar Volume	es											

HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2028								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 No-Build Condition								
Development - Belmont MA											
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	1								
Major Street Speed (mi/h)	34	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach		Eastbound		Westbound			Northbound			Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R
Number of Lanes, N	1	1 1 0		0	1	0	0	0	0	0	0	0
Lane Usage	L	Т			TR						LR	
Vehicle Volumes Averages (veh/h)	7	452	0	0	483	4	0	0	0	4	0	8
Pedestrian Averages (peds/h)	0				0			0			0	
Gap Averages (gaps/h)		0			0			0		0		
Delay (s/veh)	0.0			0.0			0.0			0.0		
Delay (veh-hrs)	0.0			0.0		0.0			0.0			
School Crossing and Roadway Network												

Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

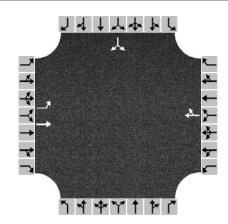
Railroad Crossing

Traini Gard Grossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

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	HCS7 Warrants Report													
Volume Si	Volume Summary													
Hour	Major	Minor	Total	Peds/h	Gaps/h	1A	1A	1B	1B	2	3A	3B	4A	4B
Hour	Volume	Volume	Volume	reus/II	Gaps/II	(100%)	(80%)	(100%)	(80%)	(100%)	(100%)	(100%)		(100%)
07 - 08	902	17	919	0	0	No	No	No	No	No	No	No	No	No
08 - 09 956 6 962 0 0 No No No No No No												No	No	No
09 - 10 816 14 830 0 0 No No No No No No No													No	No
10 - 11 795 18 813 0 0 No No No No No No No													No	No
11 - 12 799 8 807 0 0 No No No No No No No													No	No
12 - 13 872 15 887 0 0 No No No No No No No													No	No
13 - 14	917	14	931	0	0	No	No	No	No	No	No	No	No	No
14 - 15	1026	10	1036	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1143	22	1165	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1231	10	1241	0	0	No	No	No	No	No	No	No	No	No
17 - 18	1116	7	1123	0	0	No	No	No	No	No	No	No	No	No
18 - 19	794	20	814	0	0	No	No	No	No	No	No	No	No	No
Total	11367	161	11528	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1: I	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	m Vehicula	ar Volume:	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	Four-Hou	r Vehicul	ar Volun	1e										
Four-Hour	r Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3: I														
	our Condit	•						•						
	our Vehicu			ajor appro	achesar	nd highe	r minor ap	proach)						
Warrant 4: I			2											
	our Volume													
	our Volume													
Warrant 5: S														
	e Period	and												
Student V		16: 17	1											
Warrant 6: 0	raffic Contr		•										✓	
					th directic	anc)								
Degree of Platooning (Predominant direction or both directions)														
A. Adequate trials of alternatives, observance and enforcement failedand														
B. Reported crashes susceptible to correction by signal (12-month period)and														
C. 80% Volumes for Warrants 1A, 1B,or 4 are satisfied														
Warrant 8: Roadway Network														
A. Weekday Volume (Peak hour totaland projected warrants 1, 2, or 3)or														
B. Weekend Volume (Five hours total)														
			o total)											
Warrant 9: Grade Crossing A. Grade Crossing within 140 ftand														
	our Vehicu													
D. I Cak-III	Jai Veriicu	.a. voluitie	,,,											

HCS7 Warrants Report											
Project Information											
Analyst	JC	Date	1/4/2021								
Agency	VAI	Analysis Year	2028								
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 No-Build Condition								
	Development - Belmont MA										
Project Description	Pleasant Street at Olmsted Drive										
General											
Major Street Direction	East-West	Population < 10,000	No								
Starting Time Interval	7	Coordinated Signal System	No								
Median Type	Undivided	Crashes (crashes/year)	0								
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No								
Nearest Signal (ft)	360										



Approach		Eastbound	k	Westbound		Northbound			Southbound			
Movement	L	L T R		L	Т	R	L	Т	R	L	Т	R
Number of Lanes, N	1	1	0	0	1	0	0	0	0	0	0	0
Lane Usage	L	L T			TR						LR	
Vehicle Volumes Averages (veh/h)	18	452	0	0	483	12	0	0	0	12	0	20
Pedestrian Averages (peds/h)		0		0		0			0			
Gap Averages (gaps/h)		0			0		0			0		
Delay (s/veh)		0.0			0.0		0.0			0.0		
Delay (veh-hrs)		0.0		0.0		0.0			0.0			
School Crossing and Roadway Network												

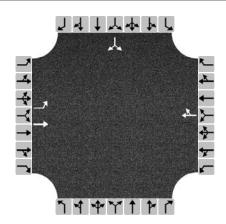
Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

Railroad Crossing

name and crossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

					HCS	57 Wai	rrants	Repoi	rt					
Volume S	ımmərv	,						_						
	_		T	D 1 (1	C //	1.0	1.4	10	10		2.4	20	4.4	45
Hour	Major Volume	Minor Volume	Total Volume	Peds/h	Gaps/h	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)		4B (100%)
07 - 08	910	44	954	0	0	No	No	No	No	No	No	No	No	No
08 - 09	967	30	997	0	0	No	No	No	No	No	No	No	No	No
09 - 10	828	33	861	0	0	No	No	No	No	No	No	No	No	No
10 - 11	809	36	845	0	0	No	No	No	No	No	No	No	No	No
11 - 12	816	25	841	0	0	No	No	No	No	No	No	No	No	No
12 - 13	892	35	927	0	0	No	No	No	No	No	No	No	No	No
13 - 14	935	33	968	0	0	No	No	No	No	No	No	No	No	No
14 - 15	1049	27	1076	0	0	No	No	No	No	No	No	No	No	No
15 - 16	1166	38	1204	0	0	No	No	No	No	No	No	No	No	No
16 - 17	1261	29	1290	0	0	No	No	No	No	No	No	No	No	No
17 - 18	1147	28	1175	0	0	No	No	No	No	No	No	No	No	No
18 - 19	818	38	856	0	0	No	No	No	No	No	No	No	No	No
Total	11598	396	11994	0	0	0	0	0	0	0	0	0	0	0
Warrants														
Warrant 1:	Eight-Hou	ır Vehicu	lar Volur	ne								$\overline{}$		
A. Minimu	m Vehicula	ar Volumes	s (Both ma	jor approa	achesan	d higher	minor ap	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand-	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2: I	Four-Hou	r Vehicul	ar Volun	ne										
Four-Hou	r Vehicular	Volume (E	Both major	· approach	esand	higher m	inor appro	ach)						
Warrant 3: I	Peak Hou	r												
A. Peak-H	our Conditi	ions (Minc	or delay	and min	or volume	and to	otal volum	e)or						
B. Peak-H	our Vehicul	lar Volume	es (Both m	ajor appro	achesai	nd highe	r minor ap	proach)						
Warrant 4: I	Pedestria	n Volume												
A. Four Ho	our Volume	sor												
B. One-Ho	our Volume	S												
Warrant 5: S	School Cr	ossing												
Gaps Sam	e Period	and												
Student V	olumes													
Nearest Tr	affic Contr	ol Signal (optional)										✓	
Warrant 6:	Coordinat	ted Signa	l System											
Degree of	Platooning	g (Predom	inant direc	tion or bo	th directio	ons)								
Warrant 7:	Crash Exp	erience												
A. Adequa	te trials of	alternative	es, observa	ance and e	nforceme	nt failed	and							
B. Reporte	ed crashes s	susceptible	e to correc	tion by sig	ınal (12-m	onth perio	od)and-							
C. 80% Vo	lumes for \	Narrants 1	A, 1B,or	r 4 are sa	ntisfied									
Warrant 8: I														
	ay Volume			nd projec	ted warra	nts 1, 2, oı	· 3)or							
	nd Volume													
Warrant 9:			<u> </u>											
	Crossing wi		:and											
	our Vehicul													

HCS7 Warrants Report										
Project Information										
Analyst	JC	Date	1/4/2021							
Agency	VAI	Analysis Year	2028							
Jurisdiction	McLean Hospital Residential	Time Period Analyzed	2028 Build Condition Zone 3 and							
	Development - Belmont MA		Zone 4							
Project Description	Pleasant Street at Olmsted Drive									
General										
Major Street Direction	East-West	Population < 10,000	No							
Starting Time Interval	7	Coordinated Signal System	No							
Median Type	Undivided	Crashes (crashes/year)	0							
Major Street Speed (mi/h)	25	Adequate Trials of Crash Exp. Alt.	No							
Nearest Signal (ft)	360		_							



Approach		Eastbound		\	Westbound			Northbound			Southbound		
Movement	L	Т	R	L	Т	R	L	Т	R	L	Т	R	
Number of Lanes, N	1	1 1 0		0	1	0	0	0	0	0	0	0	
Lane Usage	L	L T			TR						LR		
Vehicle Volumes Averages (veh/h)	59	452	0	0	483	39	0	0	0	39	0	61	
Pedestrian Averages (peds/h)		0		0		0			0				
Gap Averages (gaps/h)		0			0			0			0		
Delay (s/veh)		0.0			0.0			0.0			0.0		
Delay (veh-hrs)		0.0		0.0		0.0			0.0				
School Crossing and Roadway	Netwo	rk											

Number of Students in Highest Hour	0	Two or More Major Routes	Yes
Number of Adequate Gaps in Period	0	Weekend Counts	No
Number of Minutes in Period	0	5-year Growth Factor (%)	0

Railroad Crossing

ram oud crossing			
Grade Crossing Approach	None	Rail Traffic (trains/day)	0
Highest Volume Hour with Trains	Unknown	High Occupancy Buses (%)	0
Distance to Stop Line (ft)		Tractor-Trailer Trucks (%)	0

					HCS	57 Wai	rran <u>ts</u>	Repo	rt					
Volumo Si	ummary	,						_						
Volume S	_		T	D 1 (1	C //	1.0	1.4	10	10		2.4	20	4.4	45
Hour	Major Volume	Minor Volume	Total Volume	Peds/h	Gaps/h	1A (100%)	1A (80%)	1B (100%)	1B (80%)	2 (100%)	3A (100%)	3B (100%)		4B (100%)
07 - 08	1027	61	1088	0	0	No	No	No	Yes	No	No	No	No	No
08 - 09	1096	61	1157	0	0	No	No	No	Yes	No	No	No	No	No
09 - 10	885	71	956	0	0	No	No	No	Yes	No	No	No	No	No
10 - 11	857	88	945	0	0	No	No	No	Yes	No	No	No	No	No
11 - 12	871	117	988	0	0	No	No	No	Yes	No	No	No	No	No
12 - 13	983	128	1111	0	0	No	Yes	Yes	Yes	No	No	No	No	No
13 - 14	1016	93	1109	0	0	No	No	Yes	Yes	No	No	No	No	No
14 - 15	1122	85	1207	0	0	No	No	Yes	Yes	No	No	No	No	No
15 - 16	1232	114	1346	0	0	No	No	Yes	Yes	Yes	No	No	No	No
16 - 17	1310	164	1474	0	0	Yes	Yes	Yes	Yes	Yes	No	No	No	No
17 - 18	1185	167	1352	0	0	Yes	Yes	Yes	Yes	Yes	No	No	No	No
18 - 19	833	64	897	0	0	No	No	No	Yes	No	No	No	No	No
Total	12417	1213	13630	0	0	2	3	6	12	3	0	0	0	0
Warrants														
Warrant 1:	Eight-Hou	ır Vehicu	lar Volui	ne										
A. Minimu	ım Vehicula	ar Volumes	s (Both ma	jor approa	chesan	d higher	minor app	oroach)d	or					
B. Interrup	tion of Co	ntinuous T	raffic (Bot	h major ap	proaches	and hi	gher minc	r approacl	n)or					
80% Vehic	cularand	Interrup	tion Volur	nes (Both	major app	roaches	and higl	ner minor a	approach)					
Warrant 2:	Four-Hou	r Vehicul	ar Volun	1e										
Four-Hou	r Vehicular	Volume (E	Both major	approach	esand	higher m	inor appro	ach)						
Warrant 3:	Peak Hou	r												
A. Peak-H	our Condit	ions (Minc	or delay	and min	or volume	and to	otal volum	e)or						
B. Peak-Ho	our Vehicul	lar Volume	es (Both m	ajor appro	achesai	nd highe	r minor ap	proach)						
Warrant 4:	Pedestria	n Volume	• • • • • • • • • • • • • • • • • • •											
A. Four Ho	our Volume	esor												
B. One-Ho	our Volume	'S												
Warrant 5: S	School Cr	ossing												
Gaps Sam	e Period	and												
Student V	olumes													
Nearest Tr	raffic Contr	ol Signal (optional)										✓	
Warrant 6:	Coordinat	ted Signa	l System											
Degree of	Platooning	g (Predom	inant direc	tion or bo	th directio	ons)								
Warrant 7:	Crash Exp	erience												
A. Adequa	ite trials of	alternative	es, observa	nce and e	nforceme	nt failed	and							
B. Reporte	ed crashes	susceptible	e to correc	tion by sig	ınal (12-m	onth perio	od)and-							
C. 80% Vo	lumes for \	Warrants 1	A, 1B,oı	4 are sa	ntisfied								✓	
Warrant 8:	Roadway	Network												
A. Weekda	ay Volume	(Peak hou	r totalar	nd projec	ted warra	nts 1, 2, or	3)or							
B. Weeker	nd Volume	(Five hour	s total)											
Warrant 9:	Grade Cro	ssing												
A. Grade 0	Crossing wi	thin 140 ft	:and											
B. Peak-Ho	our Vehicul	lar Volume	es											